

# CONSULTING ENGINEERS • PLANNERS • SURVEYORS PROVIDING QUALITY PROFESSIONAL SERVICES SINCE 1960

October 16, 1998 W.O.#5244

Federal Emergency Management Agency Federal Insurance Administration Office Risk Assessment Technical Operations Division 500 C Street SW Washington, D.C. 20472

Attention:

Michael Buckley

SUBJECT:

CONDITIONAL LETTER OF MAP REVISION BASED ON PHYSICAL CHANGES FOR THE EAST C-1 DETENTION

**BASIN** 

A complete analysis of the East C-1 Detention Basin and Levee including all design calculations is included herein, in support of this request for the Conditional Letter of Map Revision (CLOMR) for the East C-1 Detention Basin, in the City of Henderson, Nevada. This request for revision is based on physical changes to the watershed and floodplain due to the proposed construction of the East C-1 Detention Basin and Levee by the Southern Nevada Water Authority. The construction of this Regional Flood Control Project will reduce the peak runoff within the floodplain downstream of the East C-1 Detention Basin site. SDN5 was used for the C-1 Channel System and SDN3 was used for the East C-1 Detention Basin and Levee. Accompanying are the HEC-1 output both on floppy disk and hardcopy. This CLOMR is requested by the City of Henderson to eliminate the Zone "A" Northwest of the East C-1 Detention Basin.

Enclosed with this request are the following items:

- 1. Revision Requestor and Community Official Form (FORM 1)
- 2. Certification by Registered Professional Engineer And/Or Land Surveyor Form (FORM 2)
- 3. Hydrological Analysis Form (FORM 3)
- 4. Riverine Hydraulic Analysis Form (FORM 4)
- 5. Riverine/Costal Mapping Form (FORM 5)
- 6. Channelization Form (FORM 6)
- 7. Levee/Floodwall System Analyses Form (FORM 8)

- 8. Dam Form (FORM 11)
- 9. Proposed Firm Map
- 10. Clark County Regional Flood Control District Maintenance of Structures Manual
- 11. East C-1 Detention Basin Calculation Notebook
- 12. Floppy disk containing SDN3 and SDN5 HEC-1 outputs
- 13. Construction Plans for the East C-1 Detention Basin and Levee

An annotated FIRM for the East C-1 Detention Basin CLOMR showing the proposed revisions has been included with this request, demonstrating that the Zone A floodplain should no longer exist Northwest of the East C-1 Detention Basin and Levee.

Because this is a public project and is funded by the Southern Nevada Water Authority, it is our understanding that the fee for reviewing this CLOMR application is waived.

If there are any questions or if additional information is necessary please contact this office at (702) 247-4020.

Respectfully Submitted,

VTN NEVADA

Lora Vennettilli, E.I.

### FEDERAL EMERGENCY MANAGEMENT AGENCY REVISION REQUESTOR AND COMMUNITY OFFICIAL FORM

CONT

FEMA USE ONLY

O.M.B. No. 3057-0148 Expires July 31, 1997

PUBLIC BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 2.13 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing and reviewing the form. Send comments regarding the accuracy of the burden estimate and any

	to respond to this right corner of this	s torm.	1. OVERVIEW				
Improved of Floodway: Other	nange sting powed methodology data	: (check a	ill that apply	)			
. Flooding Source:		sh					
. Project Name/Ident			ention 1	Basin	,		
FEMA zone designa		A		5 21			<u> </u>
(example: A, AH, The NFIP map pane	AO, A1-A30, A99, Al	-					
Community No.	Community Name	Coun		State	Map No.	Panel No.	Effective Date
X: 480301	Katy, City Harris County	Harris, Harris	Fort Bend	TX TX	480301 48201C	0005D 0220G	02/08/83
<b>480287</b> 2005 City	of Hendersor	_	rk_	<u>NV</u>	32003C	2620D	8 <u>/16/9</u>
							•
			<del></del>			<del></del>	
. The area of revision that apply)	n encompasses the fo	llowing t	ypes of flood	ing, structu	res, and associa	Led discipline	s: (check all
Types of Flood	ing		Structu	res	_	Disciplin	<del>63*</del>
<ul><li>☑ Riverine</li><li>☐ Coastal</li></ul>				elization Ploodwall		ater Resource	
Alluvial Fan	_		☐ Bridge	Culvert		☐ Hydrauli	CS .
☐ Shallow Flood ☐ Lakes	ing (e.g. Zones A() an	d AII)	Dam Coasta	1		☐ Sediment☐ Interior I	Transport Drainage
	3 L .		O Fill			ructural	
Affected wind/wi	d by ave action		D None	outuon	= ~	eotechnical and Surveying	<b>.</b>
☐ Yes			_	el Relocation		ther (describe	
□ No			= DAGE	ition (describe)			
U	_ \$				<del></del>		
Other(describ	e)		ad Dualass	ional Engin	eer and/or La	nd Surveyor'	Form for
Other(describ	d "Certification by	Register	ed Lioiess			·	
Other(describ	d "Certification by		OODWAY MF			agra <b>k</b> ts	
Other(describe Attach completed each discipline cl	d "Certification by hecked. (Form 2)  Rooding source have	2.FL	OODWAY INF	DRMATION ed on the eff	ective FIRM or	PBFM? D 1	/es ₽ No
Other(describ Attach completes each discipline cl	d "Certification by hecked. (Form 2)  Rouding source have floodway delineation	2.FL	OODWAY INF	DRMATION ed on the eff	ective FIRM or	PBFM? D 1	

Noodw	a copy of either a public notice distributed by the community stating the community's intent to revise the cay or a statement by the community that it has notified all affected property owners and affected adjacent ictions.
9. Doe	es the State have jurisdiction over the floodway or its adoption by communities participating in the NFIP?  ☐ Yes     No
	attach a copy of a letter notifying the appropriate State agency of the floodway revision and documentation of the val of the revised floodway by the appropriate State agency.
	3. PROPOSED ENCROACHMENTS
10. \	With floodways:
	a. Does the revision request involve fill, new construction, substantial improvement, or other development in the floodway?   Yes  No
	b. If yes, does the development cause the 100-year water surface elevation to increase at any location by more than 0.000 feet?   Yes No
11.	Without floodways:
	a. Does the revision request involve fill, new construction, substantial improvement, or other development in the 100-year floodplain?
	b. If yes, does the cumulative effect of all development that has occurred since the effective SPHA was originally identified cause the 100-year water surface elevation to increase at any location by more than one foot (or other surcharge limit if community or state has adopted more stringent criteria)?
the N	answer to either Items 10b or 11b is yes, please provide documentation that all requirements of Section 65.12 of IFIP regulations have been mot, regarding evaluation of alternatives, notice to individual legal property owners, arrence of CEO, and certification that no insurable structures are impacted.
	4. REVISION REQUESTOR ACKNOWLEDGMENT
	Having read NFIP Regulations, 44 CFR Ch. I, parts 59, 60, 61, and 72, I believe that the proposed revision is not in compliance with the requirements of the aforementioned NFIP Regulations.
	5. COMMUNITY OFFICIAL ACKNOWLEDGMENT
13.	Was this revision request reviewed by the community for compliance with the community's adopted floodplain management ordinances?
14.	Does this revision request have the endorsement of the community?   Yes  No
If no	to either of the above questions, please explain:
	se note that community acknowledgment and /or notification is required for all requests as outlined in Paragrap (b) of the NFIP Regulations.
	6. OPERATION AND MAINTENANCE
15.	Does the physical change involve a flood control structure (e.g., levees, floodwalls, channelization, basins, dams \textit{\textit{Q}} Yes \textit{\textit{Q}} No
1	If yes, please provide the following information for each of the new flood control structures:
	A. Inspection of the flood control project will be conducted periodically by City of Henderson entity
	with a maximum interval of months between inspections.
	B. Based on the results of scheduled periodic inspections, appropriate maintenance of the flood control facilities
1	will be conducted by
1	
1	to ensure the integrity and degree of flood protection of the structure.
	C. A formal plan of operation, including documentation of the flood warning system, specific actions and assignments of responsibility by individual name or title, and provisions for testing the plan at intervals not less than 1 year.   has been prepared for the flood control structure.

	flood control (	structure. If not performed promptly by an own	ar arnor then the earn minute. The samminity
	asu broatos c	he necessary services without cost to the Feders	al Government.
ach ope	ration and m	aintenance plans	
		7. REQUESTED RESPONSE FROM F	
Ame	examining the ndments to N , this request	he pertinent NFIP regulations and reviewing the lational Flood Insurance Program Maps, A Guid is for a:	e document entitled "Appeals, Revisions, and de for Community Officials," dated December
XX a.	CLOMR		r a proposed project, if built as proposed, would roposed hydrology changes (see 44 CFR Ch. I,
b.	LOMR		arrent NPIP map to show changes to floodplains, ally depict decreased flood hazards. (See 44 CFR
c	PMR	Because of the time and cost involved to char	es to floodplains, floodways, or flood elevations.  nge, reprint, and redistribute an NFIP map, a  effects increased flood hazards or large-scope  )
d.	Other:	Describe	
	. 0	8. FORMS INCLUDED	
•	-	'Certification By Registered Professional Engir rould be included with this request if (check the	•
Hye	•	sis for flooding source differs from that	Mydrologic Analysis Form (Form 3)
	iraulic analys d to develop F	sis for riverine flooding differs from that IRM	Riverine Hydraulic Analysis Form (Form 4)
infe		sed on updated topographic revised floodplain or floodway quested	Riverine /Coastal Mapping Form (Form 5)
The	request invo	lves any type of channel modification	☐ Channelization Form (Form 6)
		lves new bridge or culvert or revised isting bridge or culvert	☐ Bridge/Culvert Form (Form 7)
	e request invo tem	lves a new revised levee/floodwall	Levee/Floodwall System Analysis Form (Form 8)
Th	e request invo	lves analysis of coastal flooding	Coastal Analysis Form (Form 9)
	•	lves coastal structures credited as tion from the 100-year flood	Coastal Structures (Form 10)
P		lves an existing, proposed, or modified	Dam Form (Form 11)
•	•	rives all existing, proposed, or mounted	

#### 9. MNTIAL REVIEW FEE

3. BHT BAL REVI	IW PEC
18. Has the minimum initial review fee for the appropriate rec	quest category been included?   Yes No
Check or money order only. Make check or money order paying by Visa or Mastercard please refer to the credit can	
or  19. Is this request for a project that is for public benefit and is structures in identified flood hazard areas which were in the flood control project?	
or 20. Is this request to correct map errors, to include the effect hazard, or solely to provide more detailed data?	s of natural changes within the areas of special flood  Yes No
Note: I understand that my signature indicates that all information submitted in support of this request is correct.	Note: Signature indicates that the community understands, from the revision requester, the impacts of the revision on flooding conditions
INUM	in the community.  Lut Oul
Signazure of Revision Requester	Signature of Community Official
Ken D. Gilbreth, Principal Printed Name and Title of Revision Requester	Curt Chandler, Land Development M.  Printed Name and Title of Community Official
VTN Nevada  Company Name	City of Henderson Community Name
(702) 247-4020 10-16-9E	10-5-58
Telephone No. Date	Date
	ر من المنظم المن المنظم المن المنظم المن المنظم المن المنظم المن المنظم المن المنظم المنظم المنظم المنظم المنظم
Does this request impact any other communities?	⊠ No
If yes, attach letters from all affected jurisdictions acknowle the floodway, if applicable.	edging the revision request and approving the changes to
Note: Although a photograph of physical changes is not req	uired, it may be helpful for FEMA's review.

#### 9. MITTAL REVIEW FEE

Initial fee amount: \$	•
TIL sale on manage and a policy black about AP MANAY APPAP	payable to: National Flood Insurance Program. 1
paying by Visa or Mastercard please refer to the credit care	d information form which follows this form.
or  Is this request for a project that is for public benefit and is a structures in identified flood hazard areas which were in the flood control project?	
or  Is this request to correct map errors, to include the effects hazard, or solely to provide more detailed data?	of natural changes within the areas of special flood Yes No
ote: I understand that my signature indicates that all formation submitted in support of this request is errect.	Note: Signature indicates that the community understands, from the revision requester, the impacts of the revision on flooding conditions in the community.
Signature of Revision Requester	Signature of Community Official
Ken D. Gilbreth, Principal Printed Name and Trile of Revision Requester	Kevin Eubanks, Assistant Genera.  Printed Name and Title of Community Official
VTN Nevada Company Name	Clark County Regional Flood Cont: Community Name
	10/1/02
(702) 247-4020	10/1/98 Date

# FEDERAL EMERGENCY MANAGEMENT AGENCY CERTIFICATION BY REGISTERED PROFESSIONAL ENGINEER AND/OR LAND SURVEYOR FORM

FEMA USE ONLY

O.M.B. No. 3067-0148 Expires July 31, 1997

### PUBLIC BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average . 23 hour per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing and reviewing the form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden, to: Information Collections Management, Federal Emergency Management Agency, 500 C Street, S.W., Washington, DC 20472.

You are not required to respond to this collection of information unless a valid OMB Control Number is displayed in the upper right corner of this form.

019	played in the upper right corner of this form.
1.	This certification is in accordance with 44 CFR Ch. I, Section 65.2
2.	I am licensed with expertise in Hydrology and Hydraulics
	[example: water resources (hydrology, hydraulics, sediment transport, interior drainage)* structural, geotechnical, land surveying.]
3.	I have years experience in the expertise listed above.
4.	I have prepared I reviewed the attached supporting data and analyses related to my expertise.
<b>5</b> .	I A have have not visited and physically viewed the project.
6.	In my opinion, the following analyses and /or designs, is/are being certified:
	To remove the ZOne A based on a physical change.
7.	Based on the following review, the modifications in place have been constructed in general accordance with plans and specifications.
	Basis for above statement: (check all that apply)  a. Viewed all phases of actual construction.  b. Compared plans and specifications with as-built survey information.  c. Examined plans and specifications and compared with completed projects.  d. Other (Specify)
100	All information submitted in support of this request is correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 01.  Ken D. Gilbreth
`` <b>"</b>	(please print or type)
T:	le: Project Manager
l ' ''	(please print or type)
Re	gistration No. 9177 Expiration Date: 6-30-99
Su	nte Nevada
Ту	pe of License Civil
	1/2/14
1-	Signature
	10-16-99
	Date
I	
	Seal (Optional)
•s	pecify Subdiscipline
N	ote: Insert not applicable (N/A) if statement does not apply.

# FEDERAL EMERGENCY MANAGEMENT AGENCY HYDROLOGIC ANALYSIS FORM

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O.M.B. No. 3067-0148 Expires July 31, 1997

# PUBLIC BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 3.67 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing and reviewing the form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden, to: Information Collections Management, Pederal Emergency Management Agency, 500 C Street, S.W., Washington, DC 20472.

You are not required to respond to this collection of information unless a valid OMB Control Number is displayed in the upper right corner of this form.

•	Name: City of Henderson
Flooding Sou	rce: Unnamed Wash rm for each flooding source)
-	e/Identifier: East C-1 Detention Basin
Project Main	1. HYDROLOGIC AMALYSIS IN FIS
☐ App	roximate study stream (Zone A) ailed study stream (briefly explain methodology)
	2. REASON FOR NEW HYDROLOGIC ANALYSIS
	1 Improved data (see data revision on page 3)
2	Changed physical conditions of watershed (explain) Construction of Flood Control Facility in the watershed, the East C-1 Detention Basin.
	Alternative methodology (justify why the revised model is better than model used in the effective FIS)
,	Evaluation of proposed conditions (CLOMRs only) (explain) Currently under construct project should be complete by September, 1999
	Other
files for t	uter program/model was used in revising the hydrologic analysis, please provide a diskette with the input he 10-, 50-, 100 - and 500-year recurrence intervals.
Only the	100-year recurrence interval need be included for SPHAs designated as Zone A.
	3. APPROVAL OF ANALYSIS
ar	pproval of hydrologic analysis, including the resulting peak discharge value (s) has been provided by the opropriate local, state, or Federal Agency. (i.e., City of Henderson, Clark County Region Clark County Region Control District, Division of Water Resources, Dam Safety

Page 1 of 7

## 4. REVIEW OF RESULTS

		Compa	rison of 100-	year Flood	Discharges			
Loca	Lion		Drainuge (8g mi.		PIS (efi	ı) :	Revised	(cfs) :
East C-1	Detention	Basin	_	16	268	8	93	
				<del></del>		<del></del>		
Note: When I	revised dischar		t significant	tly different	than FIS diach	orane PEWA	may paguin	
					ate to complete		may requir	
ften the case w fore, transition								
ite that such a rges? Please e:	transition mu	st be assu	red. What	is the transi	tion from the	proposed disc		
	•		•		-1 Detent		n, the	Zone A
floodp	olain has	been 1	reduced	and is	confined	to the e	xisting	washes
downst	ream of	the Det	tention	Basin.				
								<del></del>
		<del></del>					<u> </u>	·
			<del></del>		<del> </del>			<del> </del>
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	····		<del></del>					
h a completed '	review of resu	alts" page	for each flo	oding source	₽.	a sasone de		
				14.5			•	
			• • • •		w values pres	ented in the F	IS (i.e. no ch	unged
			ed solely to r	revise the flo	· · · · · · · · · · · · · · · · · · ·			•
new hydrologi zulic conditions	)? 🗆 Yes 🖄 i	No		• :	·	Yes D No		

÷**€** or

### 5. HISTORICAL FLOODING INFORMATION

	Is historical data available for th If yes, provide the following	e flooding source? LJ Y ag:	es 12º No	
	Location along flooding source:			·
	Maximum peak discharge:			cfs
	Second highest peak discharge:		<del></del>	cfs
	Source of information:			
		6. GAGE RECORD IN	FORMATION .	
<u> </u>	Location of nearest gage to proj			watershed; specify)
	Gaging Station:			
	Drainage area at gage:			
	Number of years of data:			
		7. DATA RE	VISION	
	Please use the following table to new data (New) or as revising e			ted by this request and identify them as h a separate sheet.)
	Data Parameter	New	Revised	Data Source
		3 1	П	Clark County Regional
	<pre>Hydrology/Hydraulic</pre>	Analysis A	u	
	Hydrology/Hydraulic	Analysis 🖪		Flood Control
	Hydrology/Hydraulic		_	
	Hydrology/Hydraulic			
	Hydrology/Hydraulic		0	
•	The data source can be a Fede local governments may have be	ral, State, or local gove	Crnment agency, or ments than Federa	
•	The data source can be a Fede local governments may have I data may not be accepted by F discharge.  Attach documentation corrobo	ral, State, or local gove less strict data requirer PEMA unless it is demonstrating each data source case of a published doc	rnment agency, or ments than Federa nstrated that the december of the contract	Flood Control  a private source. Some State and l agencies, in which case the hydrologic lata give a better estimate of the flood
•	The data source can be a Fede local governments may have I data may not be accepted by F discharge.  Attach documentation corrobe a published document). In the	ral, State, or local gove less strict data requirer PEMA unless it is demonstrating each data source case of a published doc	rnment agency, or ments than Federa nstrated that the december of a government or a government	Flood Control  a private source. Some State and l agencies, in which case the hydrologic lata give a better estimate of the flood
•	The data source can be a Fede local governments may have I data may not be accepted by F discharge.  Attach documentation corrobe a published document). In the	ral, State, or local gove less strict data requirer EMA unless it is demonstrating each data source case of a published document of the state of the	ment agency, or ments than Federa nstrated that the dece (i.e., certified atacument or a government or a gover	a private source. Some State and I agencies, in which case the hydrologic
	The data source can be a Fede local governments may have a data may not be accepted by F discharge.  Attach documentation corrobe a published document). In the and pertinent pages may be here.	ral, State, or local gove less strict data requirer PEMA unless it is demonstrating each data source case of a published document in the state of th	ment agency, or ments than Federa nstrated that the dece (i.e., certified atacument or a government or a gover	Flood Control  a private source. Some State and l agencies, in which case the hydrologic lata give a better estimate of the flood
	The data source can be a Fede local governments may have a data may not be accepted by Fedischarge.  Attach documentation corrobe a published document). In the and pertinent pages may be here.	ral, State, or local gove less strict data requirer EMA unless it is demonstrating each data source case of a published doce leful.  8. METHODOLOGY Feecords (use Attachment B)	ment agency, or ments than Federa nstrated that the dece (i.e., certified atacument or a government or a gover	Flood Control  a private source. Some State and l agencies, in which case the hydrologic lata give a better estimate of the flood  stement, report, bibliographical reference

### ATTACHMENT A: STATISTICAL ANALYSIS OF GAGE RECORDS

	FIS:	•			Revised	 l:	
1. Number of years of data							
Systematic							<del></del>
Historical							
2. Homogeneous data?		Yes		•	☐ Yes	3	□ No
3. Data adjustments?	0	Yes		<b>6</b> ~ ·	☐ Yes	В	O No
4. Number of high outliers							
Low outliers							
Zero events							
5. Generalized skew							
6. Station skew					-		
7. Adopted skew			**				
8. Probability distribution used (justify			•				
if log-Pearson III was not used)							
9. Transfer equations to ungaged sites							Пм
If yes, specify method	••••	• • • • •	•••••	• • • • • •	16		— M
			•				
			•	<del>- 1</del>	<del></del>		
	···········						<del></del>
10. Expected probability*			• • • • • •		. 🗆 Ye	<b>:8</b>	□ N
11. Comparison of results with other analyses			· • • • • •		. 🗆 Ye	35	
If yes, describe comparison					10 10		
	sgar.		. `4.*		£2		187 - 3
***				·			
-5:				. ****			
-5:							

Attach analysis including plot of flood frequency curve.

#### ATTACHMENT B: REGIONAL REGRESSION EQUATIONS

١.	Bibliographical Reference:				
	(Attack a copy of title page, table of contents, and pertinent pages is	ncluding eq	uations.)		
2.	Gaged or ungaged stream:			·	
<b>3.</b> ક્ર,	Hydrologic region(s):  Attach backup map.				
<b>4.</b> 7	Provide parameters, values, and source of data used to define par	ameters.			
		<u> </u>			<del></del>
		FIS	8	Rev	ised
5.	Urbanized conditions calculations?	☐ Yes	□ No	☐ Yes	□ No
6	Percent of watershed urbanization	<del></del>			<del></del>
7.	Is the watershed controlled?	☐ Yes	□ No	☐ Yes	□ No
8.	Comparison with other analyses?	□ Yes	□ No	□ Yes	□ No
	If the answer to questions 5, 7, or 8 is yes, explain methodology in	n Commen	ts.	4.	
	If data are not available, indicate with "N/A".				
9.	Comments				
t					

Attach computation and supporting maps delineating the watershed boundary and drainage area divides.

### ATTACHMENT C: PRECIPITATION/RUMOPF MODEL

		FIS:	_	Revised Hec-1		
M		Unknow	1	410		
	Version: Date:	11		3/23/93	₹	
۹,	Date:			NOAA At		
	ource of rainfall distribution:	11	<del></del>	NSACE		
				6 Hr		
	ainfall duration:			Varies		
	real adjustment to precipitation (%):					
	laximum overland flow length			7000	<del></del>	
	ydrograph development method:	**	<del></del>	SCS		
L	oss rate method:			SCS	1	- Ga
	Source of soils information:			USDA S		
_	Source of land use information			Curren		
	channel routing method:			Kinema		
	Reservoir routing:	☐ Yes	DK No	Yes		
B	Baseflow considerations:	☐ Yes	K No	☐ Yes	亞	No
_	If yes, explain how baseflow was determined:		· · · · · · · · · · · · · · · · · · ·			<del></del>
	If yes, explain how baseflow was determined:  Snowmelt considerations?	. 🗆 Yes	□ No	☐ Yes	. 2	
			_	☐ Yes		N
	Snowmelt considerations?		_			
	Snowmelt considerations?		_			
-	Snowmelt considerations?		_			3 N
-	Snowmelt considerations?  Model calibration?  If yes, explain how calibration was performed  Future land use conditions?	🛘 Yes	□ N <sub>0</sub>	Yea	. 0	3 N
-	Snowmelt considerations?  Model calibration?  If yes, explain how calibration was performed  Future land use conditions?  If yes, explain why	Yes	□ No	. Ø Yer	ood	] N

Attach precipitation/runoff model, hydrologic model schematic, curve number calculations, time of concentration calculations, and supporting maps, delineating the watershed boundary and drainage area divides.

### ATTACHMENT D: CONFIDENCE LIMITS EVALUATION

charges f	charges for selected location:					
eedance l	Probability	FIS	Revised			
10%	(10-year)	cfs	cfs			
2%	-	cfs	cfs			
1%		cfs	cfs			
0.2%	(500-year)	cfs	cfs			
(100-yea	r) Flood Confidence Intervals					
	90% Confidence Interval:	5% limit	cfs			
		95% limit	cli			
	50% Confidence Interval:	25% limit	cfi			
		75% limit				
FIS is withi	e value of the 100-year frequency flood in s beyond the 50% confidence interval but in the 90% confidence interval, does the 1 r surface elevation change by 1.0 foot or	l 100-year				
Note	: An example of confidence limits analys	sis can be found in Appendix 9 of	Bulletin 17B.			

Attach Confidence Limits Analysis.

# FEDERAL EMERGENCY MANAGEMENT AGENCY RIVERINE HYDRAULIC ANALYSIS FORM

FEMA USE ONLY

O.M.B. No. 3067-0148 Expires July 31, 1997

### PUBLIC BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 2.25 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing and reviewing the form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden, to: Information Collections Management, Federal Emergency Management Agency, 500 C Street, S.W., Washington, DC 20472.

You are not required to respond to this collection of information unless a valid OMB Control Number is displayed in the upper right corner of this form. City of Henderson Community Name: \_\_\_ Unnamed Wash Flooding Source: Unnam Project Nume/Identifier: East C-1 Detention Basin 1. REACH TO BE REVISED Downstream limit: \_\_\_C1/Drake Channel\_\_ Upstream limit: \_\_East\_C-1 Detention Basin\_\_ 2. EFFECTIVE PIS Not studied Studied by approximate methods C1/Drake Channel Downstream limit of study\_\_\_ East C-1 Detention Basin Upstream limit of study\_\_\_\_ ☐ Studied by detailed methods Downstream limit of detailed study\_\_\_\_\_\_ Upstream limit of detailed study\_\_\_\_ ☐ Floodway delineated Downstream limit of Floodway\_\_\_ Upstream limit of Floodway\_\_\_ 3. HYDRAULIC ANALYSIS Why is the hydraulic analysis different from that used to develop the FIRM. (Check all that apply) Mot studied in FIS Improved hydrologic data/analysis. Explain: grand grade at 1000 to Improved hydraulic analysis. Explain:\_\_\_\_ Flood control structure. Explain: Construction of the East C-1 Detention Basin and Appurtenant Other. Explain:

PLEASE REFER TO THE INSTRUCTIONS FOR THE APPROPRIATE MAILING ADDRESS

#### 2. Models Submitted

### For areas which have detailed flooding:

Full input and output listings along with files on diskette (if available) for each of the models listed below (items 1, 2, 3, 4, and 5) and summary of the source of input parameters used in the models must be provided. The summary must include a <u>complete</u> description of any changes made from model to model (e.g. duplicate effective model to corrected effective model) At a minimum, the Duplicate Effective (item 1) and the Revised or Post-Project Conditions (item 4) models must be submitted. See instructions for directions on when other models may be required.

### For areas which do not have detailed flooding:

Only the 190-year flood profile is required. A hydraulic model is not required for areas which do not have detailed flooding; however, BFEs may not be added to the revised FIRM. If a hydraulic model is developed for the area, items 3 and 4 described below must be submitted.

If hydraulic models are not developed, hydraulic analyses for existing or pre-project conditions and revised or postproject conditions must be submitted. All calculations must be submitted for these analyses. (See Item 6 below.)

	<b>Y</b>	and the second of the second o
1. Duplicate Effective Model	Natural	Floodway
Copies of the hydraulic analysis used in the effective PIS, referred to as the effective models (10-, 50-, 100-, and 500-year multi-profile runs and the floodway run) must be obtained and then reproduced on the requestor's equipment to produce the <u>duplicate effective model</u> . This is required to assure that the effective model input data has been transferred correctly to the requestor's equipment and to assure that the revised data will be integrated into the effective data to provide a continuous FIS model upstream and downstream of the revised reach.		
2. Corrected Effective Model	Natural	Ploodway
The corrected effective model is the model that corrects any errors that occur in the duplicate effective model, adds any additional cross sections to the duplicate effective model, or incorporates more detailed topographic information than that used in the currently effective model. The corrected effective model must not reflect any man-made physical changes since the date of the effective model. An error could be a technical error in the modeling procedures, or any construction in the floodplain that occurred prior to the date of the effective model but was not incorporated into the effective model.		
3. Existing or Pre-Project Conditions Model	Natural	Floodway
The <u>duplicate effective model</u> or <u>corrected effective model</u> is modified to produce the <u>existing or pre-project conditions model</u> to reflect any modifications that have occurred within the floodplain since the date of the <u>effective model</u> but prior to the construction of the project for which the revision is being requested. If no modification has occurred since the date of the effective model, then this model would be identical to the <u>corrected</u> effective model or <u>duplicate</u> effective model.	<b>2</b>	• • • • • • • • • • • • • • • • • • •
4. Revised or Post-Project Conditions Model	Natural	Floodway
The existing or pre-project conditions model (or duplicate effective model or corrected effective model, as appropriate) is revised to reflect revised or post-project conditions. This model must incorporate any physical changes to the floodplain since the effective model was produced as well as the effects of the project. When the request is for proposed project this model must reflect proposed conditions.	_	
5. Other: Please attach a sheet describing all other models submitted.	Natural	Floodway
6. Hydraulic Analyses (Only if Hydraulic Models are not developed)	_ <b>Z</b>	
Attach all calculations for the existing or pre-project conditions and the revised or post-project conditions. Proceed to Form 5, "Riverine/Coastal Mapping Form".	· · · · · · · · · · · · · · · · · · ·	
	and the second second	

5. MODEL PARAMETERS (from model used to revise 180-year water surface elevation)

	Upstream Limit	1)OW1	nstream Limit
D-year			
-year			C-1 Deter
00-year	waterened houndary	East	Basin
00-year		<del>-</del>	
ttach diagram showing changes in 100-ye	ar discharge		
xplain how the starting water surface ele	vations were determined the	e water su	face ele
were determined using th	e revised HEC-1 flow	rates.	
			·
Give range of friction loss coefficients (Ma	nning's "N") Channel	.015	.030
•	Overbanks		
were determined.		Revise	
a transfer of the control of the con			
Explain:			
Explain:  Describe how the cross section geometry previous study) and list cross sections that  From Design drawing for	data were determined (e.g., field s	urvey, topograph	ic map, taken
Describe how the cross section geometry previous study) and list cross sections that	data were determined (e.g., field s	urvey, topograph	ic map, taken
Describe how the cross section geometry previous study) and list cross sections that	data were determined (e.g., field s	urvey, topograph	ic map, laken
Describe how the cross section geometry previous study) and list cross sections that From Design drawing for	data were determined (e.g., field at were added.  the construction.		
Describe how the cross section geometry previous study) and list cross sections that  From Design drawing for  Were natural channel banks selected as	data were determined (e.g., field and the construction.	nannel banks in t	he model?
Describe how the cross section geometry previous study) and list cross sections that From Design drawing for	data were determined (e.g., field and the construction.	nannel banks in t	he model?

## S. MODEL PARAMETERS (Cont'd)

	From construction plans for the Channel see Design Calculation
	Notebook.
_	6. RESULTS (from model used to revise 180-year water surface elevations)
	Do the results indicate:
	a. Water surface elevations higher than end points of cross sections?
	b. Supercritical depth? 🖸 Yes 🔲 No
	c. Critical depth? 🖸 Yes 🖾 No
	d. Other unique situations
	If yes to any of the above, attach an explanation that discusses the situation and how it is presented on the profiles, tables, and maps.
	What is the maximum change in energy gradient between cross-sections?
	Specify location
	What is the distance between the cross-sections in 2 above?
	What is the maximum distance between cross-sections?
	Specify location
	Ploodway determination
	a. What is the maximum surcharge allowed by the community or State?
	b. What is the maximum surcharge for the revised conditions?
	Specify location
	c. What is the maximum velocity?
	Specify location
	d. Are there any negeative surcharge values at any cross-section?  If yes, the floodway may need to be widened. If it is not widened, please explain and indicate the maximum negative surcharge.
	lain:
κþ	
хp	

### 6. RESULTS (Cont'd)

	If Yes, explain:		_	_			_	
			t this	locati	on for 1	100-year flood	elevations	new
*	constructi	lon.			<del>,</del>			
<b>你</b> , (4),								
'n								
	Do 100-year wat	er surfa	ce elevatio	ns increa	se at any loca	tion?	🗅 Ye	s 🗆 No
	on the requestor	s prope	rty, and pr	ovide an e	explanation of	ases occur, state whet I the reason for the inc placed within the curr	reases. (For examp	ole: State if t
¥.				-				
			<del></del>					
ģc	h a completed cor	nperiso	n table ent	itled: Wa	ter Surface E	levation Check (see p	age 6)	W
				7. REVISED	FRAFESA AND	FLOOD PROFILES	· ·	
				7. REVISED	FIRMTBFM AND	FLOOD PROFILES		
	The revised wat	er surfa	ce elevatio		<del></del>	FLOOD PROFILES uted by the effective F	IS Model (10-, 50-,	100-, and 50
	year), downstre	am of th	e project a	ons tie inte	o those compu	uted by the effective F	feet (vertical)	•
	year), downstre	am of th	e project a	ons tie inte	o those compu	uted by the effective F	feet (vertical)	•
	year), downstre	am of th	e project a	ons tie inte t cross-see	o those compu ction within	uted by the effective Fwithinfeet (vertical).	feet (vertical): N/A	and upstream
	year), downstreethe project at cr	am of th oas secti	e project a ion evations t	ons tie into	o those computationwithin	uted by the effective F	feet (vertical): N/A odel, dowstream of	and upstream
	year), downstreethe project at cr	am of th	e project a ion evations t	ons tie into	o those computationwithin	uted by the effective Ffeet (vertical). by the effective FIS m	feet (vertical): N/A odel, dowstream of	and upstream
	year), downstre, the project at cr The revised floo cross section	am of th	e project a ion evations t within	ons tie into	o those computationwithin ese computedfeet (vert	uted by the effective Ffeet (vertical). by the effective FIS m	feet (vertical): N/A odel, dowstream of	and upstream
	year), downstre, the project at cr The revised floo cross section	am of th	e project a ion evations t within	ons tie into	o those computationwithin ese computedfeet (vert	uted by the effective Ffeet (vertical). by the effective FIS m	feet (vertical): N/A odel, dowstream of	and upstream
	year), downstree the project at cr The revised floo cross section within Attach profiles, stream bed and (including low or	am of those sections are sections as sections as sections are sections	e project a ionevations to withinfeet (vertical flood of all flood dop-of-reference)	t cross-section in into the cal).	o those computed within N/A  rizontal scale (without encounters, trib	uted by the effective Ffeet (vertical). by the effective FIS m	N/A  nodel, dowstream of the project at cross effective FIS reported all cross sections	and upstream the project a section t, showing a, road cross
	year), downstree the project at cr The revised floo cross section within Attach profiles, stream bed and (including low o distance has ch	am of those sections at the section anged, to	e project a ion evations to within feet (vertical ame vertical of all flood d top-of-ro he station	ons tie into t cross-sec ie into tho cal).  cal and ho ds studied oud data), ing should	o those computed within N/A  orizontal scale (without enculverts, trib d be revised for	withinfeet (vertical).  by the effective FIS mical) and upstream of eas the profiles in the croachment). Also, laboratories, corporate limited.	N/A  nodel, dowstream of the project at cross effective FIS reported all cross sections at and study limit	the project a sectiont, showing s, road cross is. If channe

								MANAGMENT LEVATION								
COMMUN	ITY NAME						FLOODIND SOURCE					PROJECT NAME IIDENTIFIER				
		EFFECTIVE		DUPLIC	CATE EFFE	TIVE	CORRE	CORRECTED EFFECTIVE EXISTING/PRE-				-PROJECT REVISED/PROJECT				
			<del>r</del> -		ı —			<b>.</b>	r		<del></del>	<del></del>			<del></del>	
SECNO	NCWSEL1	FCWSEL <sup>2</sup>	SURC.3	NCWSEL1	FCWSEL <sup>2</sup>	SURC.	NCWSEL1	FCWSEL?	SURC.3	NCM2EF,	FCWSEL?	SURC.	NCWSEL'	FCWSEL <sup>2</sup>	SURC.3	
															<b> </b>	
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	-1															
					:									_		
OMMENTS	L :	L		L	1			L			L					
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						<i>:</i>										
										·						
-100-year (r	natural) Wate	r Surface Ele	vation	<del></del>	2-81	croachmer	nt (floodway)	Water Surfa	ce Elevatio	n	3.501	charge Val	ue .		•	
		desde ell conce	anallana la	the models he	41.1.		1-4		445 - 1-41-				145 3	Form A Pag		

. MT-2 Form 4 Page 6 of

# FEDERAL EMERGENCY MANAGEMENT AGENCY RIVERINE/COASTAL MAPPING FORM

FEMA USE ONLY

O.M.B. No. 3067-0148 Expires July 31, 1997

## PUBLIC BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 1.5 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing and reviewing the form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden, to: Information Collections Management, Pederal Emergency Management Agency, 500 C Street, S.W., Washington, DC 20472.

You are not required to respond to this collection of information unless a valid OMB Control Number is displayed in the upper right corner of this form.

	City of W							
Community Name:	City of He	enderson						-
looding Source:_	Unnamed Wa	sh						•
roject Name/Iden	tifier: East C	-1 Detention	n Basin					•
		1. MAPPH	NG CHANGES					
. A topogruphic w	ork map of suitable s	cale, contour inter	val, and planime	tric definition mu	st be sub	mitte	d sho	wing
(indicate N/A w	hen not applicable):				In	clude	d	
A. Revise	d approximate 100-ye	ear floodplain boun	daries (Zone A)?	<b>.</b>	Yes 🗆	No		N/A
	d detailed 100- and 50						<u> </u>	N/A
	d 100-year floodway i						ō	N/A
	on and alignment of a							
hydrai	ulic model with statio	ning control indica	ted?	<b>.</b>	Yes 🔲	No		N/A
	n alignments, roud an				Yes 🔲	No		N/A
F. Curren	nt community bounda	ries?			Yes 🗆	No	X	N/A
_	ive 100- and 500-year	- ·	-					
bound	aries from the FIRM/	FBFM reduced or e	enlarged to the	<b>34</b> 7			_	
scale (	of the topographic wor	rk map?		ന	Yes U	No	U	N/A
II. <u>Tie-in</u>	s between the effective	e and revised 100-	and 500-year	_			80	
Nood	plains and 100-year fl	oudway boundaries	s?		Yes L	No		N/A
l. The re	equestor's property bo	oundaries and com	munity easemen	ts?	Yes L	No		N/A
J. The si	gned certification of	a registered profes	sional engineer?		Yes L	No	_	N/A
K. Locat	ion and description of	reference marks?		යව	Yes 🗀	No		N/A
L. Verti	cal datum (example:	NGVD; NAVDetc.	)?		Yes 🗆	No		N/A N/A
	tal zone designations tion and alignment of				ies []	140	- 63	MM
	tel analyses?				Yes 🗆	No	Ω	N/A
	ne items ahove are ma uded on the f							
	ged. H. N/A M				nave.	1100		112
	ne source and date of t ay 1979, beach profile			n (example: ortho	oholo maj —	s, Ju	ly 19	85; fiel
= -	ne scale and contour i		-					
				··	_Contou			
b. R	evision Request		scale		Contou	r inte	rval	*
NOTE: R	levised topographic in	nformation must be	of equal or grea	ter detail.				
and 500-y FIRM and	annotated FIRM and ear floodplain and the i FBFM downstream ditional pages if need	e 100-year floodwa and upstream of th	y boundaries and	d how they tie into	those she	own o	n the	effecti
PLEA	SE REFER TO THE	INSTRUCTION	FOR THE API	PROPRIATEM	ULING A	A DIDI	RES	

## 1. MAPPENS CHANGES (Cont'd)

۵.	Has the 100-year floodplain been shifted or increased or the 100-year water surface elevation increased at any location on property other than the requestor's or community's?   Yes S No
	If yes, please give the location of shift or increase and an explanation for the increase.
_	
b.	Have the affected property owners been notified of this shift or increase and the effect it will have on their property?
	If yes, please attach letters from these property owners stating they have no objections to the revised floo boundaries if a LOMR is being requested.
c.	What is the number of insurable structures that will be impacted by this shift or increase?
	yes, explain:
if:	
ir:	yes, explain:
if;	yes, explain:  a V- zone has been designated, has it been delineated to extend landward to the heel of the primary frontal
if:	yes, explain:  a V-zone has been designated, has it been delineated to extend landward to the heel of the primary frontal ine?  Yes No  N/A
if:	a V-zone has been designated, has it been delineated to extend landward to the heel of the primary frontal ine?
if:	yes, explain:  a V-zone has been designated, has it been delineated to extend landward to the heel of the primary frontal inc?  N/A  anual or digital map submission:
if;	a V-zone has been designated, has it been delineated to extend landward to the heel of the primary frontal ine?  No  No  No  Manual

1

### 2. EARTH FILL PLACEMENT

1.	The	fill is:		Existing		☐ Propo	beed	N/A					een				to
2.	U	fili been p	المحمدات	/will be el	and in		.1	51aadu					e fr			A	
۵.		nu been p s, please s										•	163		110		ļ
3.		fill been/v		-							-	t	⊃ Yes	Ø	No		
	lf ye	es, then co	mple	le A, B, C,	and D	below.											
	<b>a.</b>	Are fill sl		ior granula e-half hori								1	☐ Yes		No		
		lf yes, ju	stify s	iteeper slo	pes												
	b.	protected greater ti	th vel by a c han 5	psion prote ocities of u cover of gr fps during	p to 5   ass, vi the 10	feet per se nes, weed )0-year fl	cond ls, or s	(fps) du similar : sust, at c	ring the vegetati n minim	: 100-; ion; sl .um, l	year fl lopes es be prot	ood m zposed ected l	ust, at d l to flou by stone	s min e wit	imum h velo ck rip	, be cities	•
		If no, des	ecribe	erosion p	rolecti	on provid	ied										
	E.	Has all fi obtainat		ced in the th the Sta											aximu Yes	_	•
	đ.	Can stru	cture	s conceiva	bly be	construc	ted or	the fill	at any	time i	in the	future	?		Yes		No
	_	ves, provid ofessional				•			ove) by t	the co	mmun	ity's N	IFIP pe	rmit (	officia	l, a r	egistered
4.	Ha	ıs fill been	place	ed/will be p	placed	in u V-zo	ne?								Yes	Ø	No
		yes, is the awail?	fill p	rotected fr	om eri	usion by a	a flood	l contro	l structi	ure su	ich as a	a revel	tment o		Yes		No
	If	yes, attacl	h Lhe	coastal str	ructure	es form.											

Riverine/Coastal Mapping Form

MT-2 Form 5

Page 3 of 3

# FEDERAL EMERGENCY MANAGEMENT AGENCY CHANNELIZATION FORM

FEMA USE ONLY

O.M.B. No. 3067-0148 Expires July 31, 1997

### PUBLIC BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 1.75 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing and reviewing the form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden, to: Information Collections Management, Federal Emergency Management Agency, 500 C Street, S.W., Washington, DC 20472.

You are not required to respond to this collection of information unless a valid OMB Control Number is displayed in the upper right corner of this form.

Community P	Name: City of Henderson
Flooding Sou	rce: Unnamed Wash
•	#Identifier: East C-1 Detention Basin
	1. EXTENT OF CHANNELIZATION
Downstream	limit:
Upstream lin	
	2. CHANNEL DESCRIPTION
1. Descri	be the inlet to the channel
2. Briefl	y describe the shape of the channel (both cross sectional and planimetric configuration) and its lining om and sides) Trapezoidal dirt lined channel with a 50 ft bottom widt
	H:4:1:V side slope and H:7:1:V side slope.
3. Descr	ibe the outlet from the channel
J. Desc.	
4. The c	nannelization includes:
ß	Levees (Attach Levee /Floodwall system analysis Form)
ŏ	Drop structures
0	Superelevated sections
(2)	Transitions in cross sectional geometry
Ø	Debris basin/detention basin
0	Energy dissipater
	Other
5. Attac	h the following:
a. C	ertified engineering drawings showing channel alignment and locations of inlet, outlet, and items
	hecked in item 4
b. 1	ypical cross sections and profiles of channel banks and invert
1	

PLEASE REFER TO THE INSTRUCTIONS FOR THE APPROPRIATE MAILING ADDRESS

## 3. HYDRAULIC CONSIDERATIONS

Deale and another final by the state of the	229	& 59	9	cfs
Do the cross sections in the hydraulic model match the typical cross sections in	the plans	? 🔯	Yes 🔲	No
Are the channel banks higher than the 100-year flood elevations everywhere?		<u> </u>	Yes 🔲	No
Are the channel banks higher than the 100-year flood energy grade lines ever	ywhere? .	. 0	Yes 🛚	No
Is the land on both sides of the channel above the adjacent 100-year flood eleve at all points along the channel?		🖾	Yes 🗆	l No
What is the range of freeboard?	·	4.2-	- 5.	2 feet
What is the range of the 100-year flood velocities?		9 -	_ 13	ft/se
What is the lining type (both bottom and sides)? Soil cement side	and bo	ttom	to S	
channel with native Explain how the channel lining prevents erosion and maintains channel stabi See Design Calculation Notebook	mater	lal. Midocum	nenlati	on)
What is the design elevation in the channel based on?			•	
☐ Subcritical flow				
Critical flow				
Supercritical New				
Energy grade line				
is the 100-year flood profile based on the above type of flow?			Yes C	3 No
If no, explain:  Is there the potential for a hydraulic jump at the following locations:				
Inlet to channel?				
Outlet of channel?				
At Drop Structures?				
At Transitions? Other locations? Explain:		لا	Yes L	ă No
If the answer to any of the above is yes, please explain how the hydraulic jump on the stability of the channel.	p is contro	iled an	d the e	ffects of th
Explain:				
•				

### 4. SEDIMENT TRANSPORT CONSIDERATIONS

lf	the answer to either 1A or 1B is yes:  a. What is the estimated sediment (bed) load?	•
	cfs (attach gradation curve)  Explain method used to estimate load See Calculation Noteb	ook.
	b. Is the 100-year flood velocity anywhere within the channel less than the 100-year flood velocity of the inlet?	☐ Yes ☒ N
	c. Will sediment accumulate anywhere within the channel?	Yes D
	d. Will deposition or scour occur at or near the inlet?	☐ Yes 🖾 1
	e. Will deposition or scour occur at or near the outlet?	☐ Yes 🖾 ì
	Attach documentation showing affects on the hydrologic and hydrau	

Channelization Form

# FEDERAL EMERGENCY MANAGEMENT AGENCY LEVEE/FLOODWALL SYSTEM ANALYSES FORM

FEMA USE ONLY

O.M.B. No. 3067-0148 Expires July 31, 1997

### PUBLIC BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 3.0. hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing and reviewing the form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden, to: Information Collections Management, Pederal Emergency Management Agency, 500 C Street, S.W., Washington, DC 20472.

You are not required to respond to this collection of information unless a valid OMB Control Number is

auspusyed i	n the upper right corner of this form.			
Community	Name: City of Henderson			
Project Nam	me/Identifier: East C-1 Detention Ba	sin	(	
	1. REACH TO SE RE	VISED		
Dow	nstream limit: C-1/Drake Channel			
Upsti	ream limit: East C-1 Detention Bas	sin		
This I	Levee/Floodwall analysis is based on:  upgrading of an existing levee/floodwall sys  a newly constructed levee/floodwall system reanalysis of an existing levee/floodwall syst			
	2. LEVEL/FLOODWALL SYS	TEM ELEMENTS		
1. Leve	e elements and locations:			
	earthen embankment, dike, berm etc.	<b>Station</b> 11+00		
<b>.</b>	structural floodwall	Station		
	other (describe)	Station	to	
2.	Structural Type:			
	monolithic cast-in place reinforced concrete	•		
	reinforced concrete masonry block			
	sheet piling  other (describe) Compact dirt w	ith soil cemen	t protection	
•	this levee/floodwall system been certified by a Feder devent?			ear
	☐ Yes	⊠ No		
If ye	s, by which agency?	•		
	rs, complete only the interior drainage section on page ntenance section of Revision Requestor and Commu		and the operation and	
	•			
<u></u>				

PLEASE REFER TO THE INSTRUCTIONS FOR THE APPROPRIATE MAILING ADDRESS

# 2. LEVEE/FLOODWALL SYSTEM ELEMENTS (Cont'd)

•			cate drawing sheet nur		
ā.	Plan of the levee embankm	ent and floodwall structures.	Sheet Numbers		
b.	water surface elevations, le	wall system showing the 100-year vee and/or wall crest and ations for the total levee system.	Sheet Numbers	·	
<b>C</b> .	opening outlet and inlet in	ater surface elevation, closure vert elevations, type and size of	Chara Nambara		
	opening, and kind of closur		Sheet Numbers		
đ.	A layout detail for the emb	ankment protection measures.	Sheet Numbers		
<b>e.</b>	Location, layout, and size a embankment features, fou structure, closure structure	ndation treatment, floodwall	Sheet Numbers		
		3. FREEBOARD			
The	e minimum freeboard provide	ed above the 100-year water surface	elevation is:		
Rive	<u>erine</u>				
	3.5 feet or more at the ups	vinstream end and throughout tream end eam and downstream of all structure	es and constrictions	☐ Yes ☐ Yes ☑ Yes	□ No □ No
Coa	<u>estal</u>				
	stillwater surge elevation of	of the one percent wave for the 100- or maximum wave runup (whichever		□ No	
	greater).		<del>-</del> ::		
	greater).  2.0 feet above 100-year sti	lwater surge elevation	□ Yes	□ No	
<b>Led</b>	2.0 feet above 100-year still ase note, occasionally excelluested, attach documentations.	ptions are made to the minimum to addressing Part 65.10 (b) (1) (iii	freeboard requirement i) of the National Floo	it If an o	
req reg	2.0 feet above 100-year stinase note, occasionally exceptuested, attach documentational pulations.  To is answered to any of the answered an indication from history is No. If yes, provide	otions are made to the minimum	freeboard requirement i) of the National Floor y: effect the 100-year wate	it. If an ood insurar	elevatio
req reg If n	2.0 feet above 100-year still ase note, occasionally exceptuested, attach documentationally exceptuations.  To is answered to any of the answered an indication from historyes No If yes, provide ove still exists.	ptions are made to the minimum ton addressing Part 65.10 (b) (1) (ii) bove, please explain where and why prical records that ice-jamming can e	freeboard requirement i) of the National Floor y: effect the 100-year wate e that the minimum free	er surface	elevatio
req reg	2.0 feet above 100-year still ase note, occasionally excelluested, attach documentations. The is answered to any of the analysis and indication from historyes. No If yes, provide ove still exists.	ptions are made to the minimum ion addressing Part 65.10 (b) (1) (ii) bove, please explain where and why prical records that ice-jamming can elice-jam analysis profile and evidence al locations (tabulate values at each 100-Year Water	freeboard requirement i) of the National Floor y: effect the 100-year wate e that the minimum free	er surface	elevatio
req reg if n is ti l i	2.0 feet above 100-year still ase note, occasionally exceptuested, attach documentationally exceptuations.  The is answered to any of the action from history is No If yes, provide ove still exists.  The is a location in the indication is a critical exists.	ptions are made to the minimum ion addressing Part 65.10 (b) (1) (ii) bove, please explain where and why prical records that ice-jamming can elice-jam analysis profile and evidence al locations (tabulate values at each 100-Year Water Surface Elevation	freeboard requirement i) of the National Floor y: effect the 100-year wate that the minimum free alevee crest grade chan	er surface reboard di	elevations scussed
req reg if n is ti	2.0 feet above 100-year still ase note, occasionally exceptuested, attach documentationally exceptuations.  The is answered to any of the action from history is an indication from history is No If yes, provide ove still exists.  The indication from history is the elevations at critical contains at critical contains and indication is a contained by the elevation is a critical contained by the elevation i	ptions are made to the minimum from addressing Part 65.10 (b) (1) (ii) bove, please explain where and why prical records that ice-jamming can elice-jam analysis profile and evidence al locations (tabulate values at each 100-Year Water Surface Elevation 2344.74	freeboard requirement i) of the National Floor y: effect the 100-year wate that the minimum free levee crest grade chan Levee Crest 2349	er surface teboard di	elevations scussed
req reg If n is ti still about Tat	2.0 feet above 100-year still ase note, occasionally excelluested, attach documentations.  The is answered to any of the answered to any of the answered and indication from history is a location of the elevations at critical to the elevations at critical action are considered to the elevation of the elevation o	potions are made to the minimum from addressing Part 65.10 (b) (1) (ii) bove, please explain where and why prical records that ice-jamming can explain analysis profile and evidence al locations (tabulate values at each 100-Year Water Surface Elevation 2344.74 2336.23	freeboard requirement i) of the National Floory:  effect the 100-year wate that the minimum free levee crest grade channels between the control of the contr	er surface reboard di	elevations scussed
req reg tf n ls ti l l l l l l l l l l l l l l l l l l	2.0 feet above 100-year still ase note, occasionally exceptuested, attach documentations.  The is answered to any of the answered to any of the answered an indication from history (a) No If yes, provide ove still exists.  The isolate the elevations at critical contains and the elevation at critical contains and critical contains and critical contains and critical con	botions are made to the minimum from addressing Part 65.10 (b) (1) (ii) bove, please explain where and why brical records that ice-jamming can elice-jam analysis profile and evidence al locations (tabulate values at each 100-Year Water Surface Elevation 2344.74 2336.23 2325.72	freeboard requirement i) of the National Floory:  effect the 100-year wate that the minimum free levee crest grade channels between the control of the contr	er surface reboard di	elevations scussed
req reg if n is ti i abo	2.0 feet above 100-year stinase note, occasionally exceptuested, attach documentationally exceptuations.  The is answered to any of the answered an indication from history (and the elevations at critical content of the elevations at critical content of the elevations at critical content of the elevation at critical content of the elevation of t	ptions are made to the minimum ion addressing Part 65.10 (b) (1) (ii) bove, please explain where and why prical records that ice-jamming can elice-jam analysis profile and evidence al locations (tabulate values at each 100-Year Water Surface Elevation 2344.74 2336.23 2325.72 2303.59	freeboard requirement i) of the National Floory:  effect the 100-year wate that the minimum free that the minimum free levee crest grade channels become contact the contact that the minimum free contact the contact that the minimum free contact that the co	er surface reboard di	elevations scussed
req   reg	2.0 feet above 100-year still ase note, occasionally exceptuested, attach documentations.  The is answered to any of the answered to any of the answered an indication from history (a) No If yes, provide ove still exists.  The isolate the elevations at critical contains and the elevation at critical contains and critical contains and critical contains and critical con	botions are made to the minimum from addressing Part 65.10 (b) (1) (ii) bove, please explain where and why brical records that ice-jamming can elice-jam analysis profile and evidence al locations (tabulate values at each 100-Year Water Surface Elevation 2344.74 2336.23 2325.72	freeboard requirement i) of the National Floory:  effect the 100-year wate that the minimum free levee crest grade channels between the control of the contr	er surface reboard di	elevations scussed

## 5. SEDMENT TRANSPORT CONSIDERATIONS

	ā.		r indication from histori 100-year water surface (	elevations?	diment transport (including sco	ur and deposition) can
	b.			geomorphology, ve	Yes No getative cover and developmen	
			leposition) to affect the	e 100-year water su	ential for debris and sediment trurface elevations and/or the free	
	rj.	10 v w ga	Trans.	L	☐ Yes ☐ No	
	If ti	he answer to	either 1a or 1b is yes:			
		A. Wh	nat is the estimated sed	diment (bed materi	ai) load?	
	*		cfs (attach gradatio	on curve)		
		Explain me	thod used to estimate	the sediment trans	sport and the depth of scour and	Vor deposition
	d.	- W			at the sandth and sandth for the sands	
	_		ill sediment accumulat annel)?		the levee/floodwall (such as alo	ng any bends in the
	¥		· · · · · · · · · · · · · · · · · · ·	,	☐ Yes ☐ No	
	£:	Ify	yes, what is the minimu	um freeboard at th	ese locations?	feet.
_		<u></u>		6. CLOSURI	ES	
	O <sub>1</sub>	neninas throu	ugh the levee system:			
	_		do not exist			
	-	-	st, list all closures:			
		-		0	417 -h -is Plainting Sap	Town of
	_	hannel tation	<u>Left or Right</u> <u>Bank</u>	Opening Type	Highest Elevation for Opening Invert	Type of Closure Device
	(E.	xteng table o	on an added sheet as no	aded and reterence	•)	
_			to all adapas			
		11	14 5 4 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5			
<b>FO</b>		nical and geol	•	م موملہ عدد عام	S. A. A. A. S. S. St. and S. S. Sanda	* ****
<b>?</b> 0	in a	addition to th	he required detail anal	lysis reports, data o hmitted in a tabula	obtained during field and labora ated summary form for the follo	tory investigations and

### 4. EMBANKMENT PROTECTION

1.	Maximum lev	ree slope land	side:	<u></u>	:				
2.			dside:						1
<b>3</b> .		-year riverine	flood velocitie				· -		iin.)
4.	Embankmen	t material is po	rotected by (de		nd): so				
5.	Riprap Desig	n Parameters:	(Include refe	rences)	0	Velocity;	Tracti		
	Reach	<u>Sideslope</u>	Flow depth	<u>Velocity</u>	<u>Curve or</u> <u>Straight</u>	Stone Rij D100 D20 TI		Depth of <u>Toedown</u>	
Sta_	to	-							
Sta_ Sta_	to	_							
6.			I sheet as need sis and design			No			
7.	Describe the	e analysis for o	other kinds of p	protection us	ed (include c	opies of the de	sign analy:	sis):	

Attach engineering analysis to support construction plans.

### 7. EMBANKMENT AND FOUNDATION STABILITY

☐ Overall	height: Sta	height	10	_ft.	
-	g foundation soil strength:				
	Sta	, <b>dep</b> th0	to	50+	
	strength $\mathcal{D} = 38$			psf	
Slope: S	$iS = \frac{7}{}$ (h) to	1	_(v)		
(Repeat as no	eeded on an added sheet fo	or additional slopes	and locations)		
.45		·			ak infinisa alama
	embankment stability analy circular arc				
Summary of	stability analysis results:				
Cara	Loading Conditions		Critical <u>Safety Fac</u>		Criteria (Min.)
Case	LOSQING CONGITUOTS	2	301ELY FOL	<u>.tor</u>	<u> </u>
i	End of construction	n			1.3
H "	Sudden drawdown	1		<del> </del>	1.0
116 ,	Critical flood stage	•		<del></del>	1.4
IV	Steady seepage at	flood stage	N/A		1.4
VI	Earthquake (Case I	)	1.4		1.0
(Reference:	U.S. Army Corps of Engine	pers EM-1110-2-191	3 Table 6-1)		
(Mererance.	o.s. ramy corps or angine				
Was a seepa	age analysis for the emban	kment performed?			□ Yes 🖾 No
Desci	ribe methodology used:				
			·	<u> </u>	<del> </del>
Was a seepa	age analysis for the founda	ation performed?			□ Yes 🖸 No
Were	e uplift pressures at the em	bankment landside	toe checked?		□ Yes □ No
Were	e seepage exit gradients ch	necked for piping po	otential?		☐ Yes ☐ No

### B. FLOODWALL AND FOUNDATION STABILITY

	□ UBC (1988) •	r 🔲 Other (s	pecify)			<del></del>	<del></del>
•	Stability analysis submit Overturning;	tted provides fo	or: If not, explai	n			
•	Loading included in the	analyses were					
	☐ Lateral earth @ PA	\		_psf; P =		psf	
	Surcharge-Slope					•	psf
	□ Wind @Pw=				· · · · · · · · · · · · · · · · · · ·		
	Seepage (Uplift)			Earthquake @	Peq =		%g
	☐ 100-year significa	nt wave height		ft.			
	100-year significa	nt wave period					
	Summary of Stability A loading condition limit			ech.		-	
Lo	pading Condition	Criteria (Mi Overturn	n) Sliding	Sta Overturn	Sliding	Sta Overturn	
ead	& Wind	1.5	1.5				
ead	& Soil	1.5	1.5				
ead,	Soil, Flood & Impact	1.5	1.5				
ead,	, Soil & Seismic	1.3	1.3				
	(Ref: FEMA 114 !	•		-	ence)	•	•
•	Foundation bearing st	trength for eacl	h soil type:				
	Bearing Pressure		<u>Sust</u>	tained Load		Short Te	erm Load
	Computed design max	kimum			_psf	<del></del>	psf
	Maximum allowable		<del></del>		_psf	<del></del>	psi
<b>-</b>	Foundation scour pro	tection 🔲 is,	is not p	rovided, (descri	ibe)		
6.	•	<del></del>			ibe)		

### 9. SETTLEMENT

Settlement of the levee crest is determined to be primarily from:    Foundation consolidation	Computed range of settlement :	0.05 ft.to	0.10 t.
Foundation consolidation   Embankment compression   Other (describe)   Combined   and   both   azid   short   term			
Other (describe)		, <b>,,,</b>	
Other (describe)	Embankment compression		
has has not been accommodated in the structural design and construction.  Attach engineering analysis to support construction plans.  18. BITERIOR DRABMAGE  Specify size of each interior watershed Draining to pressure conduit Draining to ponding area  Relationships Established  Ponding elevation vs. storage	Other (describe) Comb	bined and both and s	short term
Specify size of each interior watershed  Draining to pressure conduit  Draining to ponding area  Relationships Established  Ponding elevation vs. storage   Yes   No   Ponding elevation vs. gravity flow   Yes   No   Differential head vs. gravity flow   Yes   No   The river flow duration curve is enclosed   Yes   No   Specify the discharge capacity of the head pressure conduit  Which Flooding Conditions Were Analyzed?  • Gravity flow (Interior Watershed)   Yes   No   • Historical ponding probability   Yes   No   If no, explain why:  Interior drainage has been analyzed based on joint probability of interior and exterior flooding and tapacities of pumping and outlet facilities to provide the established level of flood protection.	Many .		
Specify size of each interior watershed  Draining to pressure conduit  Draining to ponding area  Relationships Established  Ponding elevation vs. storage   Yes   No   Ponding elevation vs. gravity flow   Yes   No   Differential head vs. gravity flow   Yes   No   The river flow duration curve is enclosed   Yes   No   Specify the discharge capacity of the head pressure conduit  Which Flooding Conditions Were Analyzed?  • Gravity flow (Interior Watershed)   Yes   No   • Historical ponding probability   Yes   No   If no, explain why:  Interior drainage has been analyzed based on joint probability of interior and exterior flooding and tapacities of pumping and outlet facilities to provide the established level of flood protection.		nodated in the structural design	and construction.
Specify size of each interior watershed  Draining to pressure conduit  Draining to ponding area  Relationships Established  Ponding elevation vs. storage Ponding elevation vs. gravity flow Differential head vs. gravity flow Differential head vs. gravity flow Per low duration curve is enclosed  Specify the discharge capacity of the head pressure conduit  Which Flooding Conditions Were Analyzed?  Gravity flow (Interior Watershed) Formum storm (River Watershed) First low		port construction plans.	
Specify size of each interior watershed  Draining to pressure conduit  Draining to ponding area  Relationships Established  Ponding elevation vs. storage	and		
Draining to pressure conduit Draining to ponding area  Relationships Established  Ponding elevation vs. storage		10. MITERIOR DRAMAGE	
Relationships Established  Ponding elevation vs. storage	•		
Relationships Established  Ponding elevation vs. storage	•	t	
Ponding elevation vs. storage	Draining to ponding area		
Ponding elevation vs. gravity flow Differential head vs. gravity flow Differential hea	Relationships Established		
Differential head vs. gravity flow	_		
The river flow duration curve is enclosed	•	<del>-</del>	
Specify the discharge capacity of the head pressure conduit  Which Flooding Conditions Were Analyzed?  Gravity flow (Interior Watershed)	Differential head vs. gravity	TIOW	∐ Yes □ No
Which Flooding Conditions Were Analyzed?  Gravity flow (Interior Watershed)	The river flow duration curve is ea	nclosed	☐ Yes ☐ No
Gravity flow (Interior Watershed) Common storm (River Watershed) Historical ponding probability Coastal wave overtopping The coastal wave overtopping  Interior drainage has been analyzed based on joint probability of interior and exterior flooding and to capacities of pumping and outlet facilities to provide the established level of flood protection.  The common storm (River Watershed) Yes No  Interior drainage has been analyzed based on joint probability of interior and exterior flooding and to capacities of pumping and outlet facilities to provide the established level of flood protection.  The common storm (River Watershed) Yes No  Interior drainage has been analyzed based on joint probability of interior and exterior flooding and to capacities of pumping and outlet facilities to provide the established level of flood protection.	Specify the discharge capacity of	the head pressure conduit	***************************************
Common storm (River Watershed) Historical ponding probability Coastal wave overtopping Tyes No  If no, explain why:  Interior drainage has been analyzed based on joint probability of interior and exterior flooding and to capacities of pumping and outlet facilities to provide the established level of flood protection.  Yes No  If no, explain why:  If no, explain why:	Which Flooding Conditions Were	: Analyzed?	
Common storm (River Watershed) Historical ponding probability Coastal wave overtopping Tyes No  If no, explain why:  Interior drainage has been analyzed based on joint probability of interior and exterior flooding and to capacities of pumping and outlet facilities to provide the established level of flood protection.  Yes No  If no, explain why:  If no, explain why:	<del>,</del>		
Coastal wave overtopping  If no, explain why:  Interior drainage has been analyzed based on joint probability of interior and exterior flooding and to capacities of pumping and outlet facilities to provide the established level of flood protection.  Yes No  If no, explain why:	<ul> <li>Common storm (River</li> </ul>	er Watershed)	<del>-</del>
If no, explain why:  Interior drainage has been analyzed based on joint probability of interior and exterior flooding and to capacities of pumping and outlet facilities to provide the established level of flood protection.    Yes   No		-	
Interior drainage has been analyzed based on joint probability of interior and exterior flooding and to capacities of pumping and outlet facilities to provide the established level of flood protection.    Yes   No   If no, explain why:	•	•	
capacities of pumping and outlet facilities to provide the established level of flood protection.    Yes   No	it no, explain why:		
capacities of pumping and outlet facilities to provide the established level of flood protection.    Yes   No	Interior desirana has been seed	zed hazed on inine analything	of interior and autorior flooding and at
If no, explain why:			lished level of flood protection.
	If no. explain why:		

### 10. MTERIOR DRAMAGE (Cont'd)

•	The length of levee system used to drive the seepage rate in item 7:		ft.
	Will a pumping plant(s) be used for interior drainage?	🗆 Yes 🚨 No	
	If yes, include the number of pumping plants: For each pumping plant, list:	Plant #1	Plant #2
	The number of pumps		
	The ponding storage capacity		
	The maximum pumping rate	<del></del>	
	The maximum pumping head	<del></del>	<del>مجاد خاندانی</del>
	The pumping starting elevation	· ·	<del></del>
	The pumping stopping elevation		
	is the discharge facility protected?	<del></del>	<del></del>
	Is there a flood warning plan?		
	How much time is available between warning and flooding?		
	Will the operations be automatic? If the pumps are electric, are there backup power sources?	☐ Yes ☐ No ☐ Yes ☐ No	
	(Reference: U.S. Army Corps of Engineers EM-1110-2-3101, 3	102, 3103, 3104, and 31	05)
	11. OTHER DESIGN CRITERIA		
	The following items have been addressed as stated:	**************************************	
	Liquifaction 📋 is 📋 is not a problem.		
	Hydrocompaction  is is not a problem		
	Heave differential movement due to soils of high shrink/sw	vell 🔲 is 🔲 is no	t a problem.
_	For each of these problems, state the basic facts and corrective act	Namankan	
<b>5-</b>			
•			
3.	If the levee/floodwall is new or enlarged, will the structure adver- floodside of the structure?	sely impact flood levels	and/or now velocities
	naaavaa an mie sn <i>ach</i> nia:	🗆 Yes 🗅 N	0
Att	ach supporting documentation		
Att	ach supporting documentation		

### OPERATIONAL PLAN AND CRITERIA

the NFIP	regulations?	☐ Yes ☑ No
		ions for interior drainage as required in Section 65.10 (c) (2
or the N	FIP regulations?	☐ Yes 🖾 No
*		
*		

# FEDERAL EMERGENCY MANAGEMENT AGENCY DAM FORM

FEMA USE ONLY

O.M.B. No. 3067-0148 Expires July 31, 1997

### PUBLIC BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 0.5 hour per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing and reviewing the form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden, to: Information Collections Management, Federal Emergency Management Agency, 500 C Street, S.W., Washington, IXC 20472.

You are not required to respond to this collection of information unless a valid OMB Control Number is displayed in the upper right corner of this form. City of Henderson Community Name: Unanmed Wash Flooding Source: East C-1 Detention Basin Project Name/Identifier: 1. IDENTIFIER Name of Dam: East C-1 Detention Basin Location of dam along flood source (in terms of stream distance or cross section identifier): Check one of the following: □ Existing dam ☑ New dam ■ Modifications of existing dam (describe modifications) Was the dam designed by: Federal agency State agency X Private organization? Local government agency 2. BACKGROUND Ø Yes O No Does the dam have dedicated flood control storage? Does the project involve revised hydrology? X Yes □ No If yes, complete Hydrologic Analysis Form (Form 3) and include calculations of the 100-year inflow flood hydrograph routed through the dam with the beginning pool at the normal pool elevation (spillway crest elevation for ungated spillway). Include any inflow hydrograph bulking by watershed sediment yield and provide necessary debris and sediment yield analysis. Does the revised hydrology affect the 100-year water-surface elevation behind the dam or downstream of the dam? If yes, complete the Riverine Hydraulic Analysis Form (Form 4) and complete the table shown on the following page.

# 3. RESULTS

	Stillwater Elevation Behi	no the Deni
	FIS	Revised
-year		•
-year		0.405
10-year	_	2485
00-year 3 ormal Pool Elevation	, .	-0-
Was long term sediment accumulation taken elevation?	into consideration in del s  No	termining the normal po
Was the dam designed to withstand the hydrogreater than the 100-year flood?	ostatic and hydrodynamic f is 🗆 No	iorces associated with floo
If no, and the dam has a reasonable probadam break analysis.	bility of failure during the 10	00-year flood, please attact
rovide the following data on the dam:		
Dimensional Height: 64 ft  Crest Elevation of top of dam: 2510 ft		
100-year flood storage capacity: 347		L
Freeboard (measured from 100-year water surfa		
pillwäy(s):	Outlet(s):	
Type: 🗍 gated 🗎 ungated	Type:	gated ungated
Dimensional Width: 300' Ogee Crest	ted Weir Width:	
Dimensional Height: R1'	Height	
Crest Elevation of top of spillway: 2500	Diameter	•
• • • • • • • • • • • • • • • • • • • •	Invert Ele	vation:
Explain flow regulation plan:	:	<del></del> , .
	· · · · · · · · · · · · · · · · · · ·	
Are the project features, including the emergency spil	huse designed to accommo	data the 100-year flood
	es No	date de 100-year 11000
Was the dam designed in accordance with all current regulations?	ly applicable local, State, and Yes 🔲 No	d Federal
If no, please provide explanation.		
ii no, presse provide explanedors.	······································	
		<del></del>
FEMA may request a list of regulations that		and supporting documenta
FEMA may request a list of regulations that demonstrating compliance with these regulations and maintenance places.	ions.	and supporting documenta

# INTERLOCAL CONTRACT

# 1998/1999 ANNUAL MAINTENANCE PROGRAM

## WITNESSETH:

WHEREAS, pursuant to Chapter 543 of the Nevada Revised Statutes, the District may approve and fund projects to maintain flood control improvements; and

WHEREAS, the City desires to maintain flood control improvements within the City in accordance with the maintenance program set forth herein, and hereinafter referred to as "Project"; and

WHEREAS, the facility upon which maintenance will be done is a facility described in the District's Master Plan.

NOW, THEREFORE, in consideration of the covenants, conditions, agreements, and promises of the parties hereto, the District authorizes the project as it is mutually understood and agreed as follows:

### **SECTION I - SCOPE OF PROJECT**

This Interlocal Contract applies to the maintenance of flood control facilities, which are identified in the District's Master Plan facilities including updates and amendments subsequently approved. The basic maintenance to the facilities will be in accordance with performance standards of the Operations and Maintenance Manual. The Project is more specifically described in Exhibit "A", which is attached hereto, and by this reference incorporated herein.

## **SECTION II - PROJECT COSTS**

The District agrees to provide reimbursement for Project costs within the limits specified below:

1. The Project costs shall not exceed \$298,629.00.

The amounts allocated to each individual facility within the Project must be specified in Exhibit "A".

Any changes to said allocated amounts must be approved by the Chief Engineer of the Clark County Regional Flood Control District in accordance with Section 4.24 of the District Operations and Maintenance Manual.

A written request must be made to the District and a Supplemental Interlocal Contract approved to increase the amount noted above prior to payment of any additional funds.

- 2. "Authorization to Proceed" is herein granted for maintenance of facilities in Exhibit "A" in amount not to exceed \$74,657, effective July 1, 1998.
- 3. A separate request for an "Authorization to Proceed" will be required for additional facility maintenance funds.
- 4. A written request must be made to the District and a Supplemental Interlocal Contract approved to increase the amount noted above prior to payment of any additional funds.
- 5. The City and District will comply with Section 4.12 of the Operations and Maintenance Manual. In accordance with said manual the City shall submit invoices together with a detailed summary report of the maintenance service performed. The City shall submit an Invoice Voucher prepared in duplicate in the manner prescribed by the District. The vouchers shall include such information as is necessary for the District to determine the nature of all expenditures. Each voucher will clearly indicate that it is for services rendered in performance under this contract. Each voucher will also be accompanied by a written certification from the City stating that it is for performance of maintenance activities under this contract and is composed of completed elements set forth in the annual work program. All invoices must be submitted for payment to:

Gale W. Fraser, II, P.E., General Manager/Chief Engineer Clark County Regional Flood Control District 301 East Clark, Suite 301 Las Vegas, NV 89101

Payment shall be considered timely if made by the District within 30 days. Pursuant to Section IV, Paragraph 6, the District may, in its sole discretion, withhold payments to the City for services rendered if the City fails to satisfactorily comply with any term or condition of this contract and/or the District's

Operations and Maintenance Manual.

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The City agrees to perform the Project to the satisfaction of the District prior to June 30, 1999. The District may grant extensions or terminate this contract and require all sums advanced to the City to be repaid if the City fails to perform by said date.

SECTION III - PROJECT TIME

# **SECTION IV - GENERAL**

1. The City will complete the Project as set forth in Exhibit "A". The City staff personnel responsible for coordination work under this contract are as listed below:

> Mark T. Calhoun, Director of Public Works W. Curtis Chandler, Land Development Manager Carl Noyes, Support Services Manager

It is understood that staff named above will be responsible for work coordination throughout the period of this contract unless the District is informed in writing of changes in these personnel assignments.

- 2. In addition to the specific terms set forth in this Contract, the parties hereto shall be subject to and governed by the District's Operations and Maintenance Manual, and any applicable portions of the Policies and Procedures adopted by the District.
- 3. It is the intent of the District that scheduling of maintenance and repair of drainage and flood control facilities in general and Master Plan Facilities specifically be coordinated among entities. Therefore, in those cases where Master Plan approved and District-funded projects have regional flood control significance impacting more than one entity, the City will allow all impacted entities an opportunity to review the maintenance schedule in order to coordinate maintenance efforts.
- 4. The Chief Engineer of the District shall be responsible for monitoring the performance of the City, approval for payment of billings and expenses submitted by the City and the acceptance of any reports provided by the City. The City shall be responsible for monitoring performance of City staff or private contractors, and the City shall maintain detailed records of all payments made to contractors and make such

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records available to the District upon request.

- 5. The City shall provide right of access to its facilities to the District or Chief Engineer at all reasonable times, in order to monitor and evaluate performance, compliance, and/or quality assurance under this contract.
- 6. In the event the City fails to perform the maintenance according to the standards specified in this contract and in the District's Operations and Maintenance Manual, the District may perform or cause to be performed the maintenance necessary to assure proper operation of the facility. Cost incurred by the District shall be reimbursed by the City or be deducted from the amount authorized by this contract. The District may not exercise this right without giving the City specific written notice of the maintenance required and allowing the City 60 days within which to perform said maintenance. The notice required by this provision must be sent to:

Mark T. Calhoun, Director of Public Works 240 Water Street Henderson, NV 89015

- 7. The records of the City and/or private contractors pertaining to the subject matter of this contract shall at all reasonable times be subject to inspection and audit by the District, County Auditor, or an Agent of the District.
- 8. If any provision of this contract shall be deemed in conflict with any statute or rule of law, such provision shall be deemed modified to be in conformance with said statute or rule of law.
  - 9. All parties to this contract shall comply with applicable local, state, and federal laws.
- 10. Any costs found to be improperly allocated in the project will be refunded by the City to the
- 11. It is specifically understood and agreed to by and between the parties hereto that it is not intended by any of the provisions of any part of this contract to create in the public or any member thereof a third party beneficiary hereunder, or to authorize anyone not a party to this contract to maintain a suit for

Page 4 of 5

bersonal injuries or property damage pursuant to the terms or provisions of this contract. 1 2 The City hereby indemnifies and shall defend and hold harmless the District, its 12. 3 representatives and their employees (or their authorized representatives) from and against any and all suits, actions, legal or administrative proceedings, claims, demands, damages, liabilities, interest, attorney's fees, 5 costs and expenses whatsoever of any kind or nature whether arising before or after completion of the work 6 hereunder and in any manner directly or indirectly caused, occasioned or contributed to in whole or in part, 7 by reason of any act, omission, fault or negligence whether active or passive of the City, of anyone acting 8 under its direction or control, or on its behalf in connection with or incident to the performance of this g Contract. The City's aforesaid indemnity and hold harmless obligations, or portions or applications thereof, 10 shall apply to the fullest extent permitted by law, but in no event shall they apply to liability caused by the sole 11 negligence or willful misconduct of the party indemnified or held harmless. 12 IN WITNESS WHEREOF, the parties have caused this contract to be executed the day and year first 13 14 15 Date of Council Action: 16 ES B. GIBSON, Mayor 17 ATTEST: 18 20 ARK COUNTY REGIONAL FL 21 Date of District Action: JUN. 1 1 1998 22 BY: ánce Malone, Chairman 23 ATTEST APPROVED AS TO FORM **Board Secretary** 25 TOPHER FIGGINS Deputy District Attorney 26 F:\HOME\SHRDPW\WP\E\_LAND\CCRFCD\ICANNLMT.96

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# CONSULTING ENGINEERS • PLANNERS • SURVEYORS PROVIDING QUALITY PROFESSIONAL SERVICES SINCE 1960

November 13, 1998 W.O.#<del>\$941</del> 5244

Federal Emergency Management Agency P.O. Box 3173 Merrifield, VA 22116-3173

# ATTN: FEE-COLLECTION SYSTEM ADMINISTRATOR

Enclosed is a check (#54817) in the amount of \$5000.00 per your request in a letter dated October 29, 1998, reference Case No. 99-09-066R. This fee is required to begin processing of the East C-1 Detention Basin LOMR request. Thank-you for your assistance.

Sincerely, VTN Nevada

Lora Vennettilli, E.I.

cc:

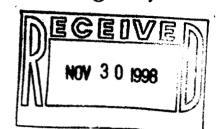
Mr. Kevin L. Eubanks, P.E. Assistant General Manager Clark County Regional Flood Control District

Mr. Curt Chandler, P.E. Land Development Manager Department of Public Works City of Henderson



# Federal Emergency Management Agency

Washington, D.C. 20472



November 24, 1998

Ms. Lora Vennettilli, E.I. VTN Nevada 2727 South Rainbow Boulevard Las Vegas, Nevada 89146-5148 IN REPLY REFER TO: Case No.: 99-09-066R

Community: City of Henderson, Nevada

Community No.: 320005

316-ACK.FRQ

Dear Ms. Vennettilli:

This responds to your letter dated November 13, 1998, concerning an October 16, 1998, request that the Federal Emergency Management Agency (FEMA) issue a conditional revision to the Flood Insurance Rate Map (FIRM) for Clark County, Nevada and Incorporated Areas. Pertinent information about the request is listed below.

Identifier:

East C-1 Detention Basin

Flooding Source:

Unnamed Wash

FIRM Panel(s) Affected:

32003C2620 D

We have completed an inventory of the items that you submitted. We have received the data and the review and processing fee (\$5,000) required to begin a detailed technical review of your request. If additional data are required, we will inform you within 30 days of the date of this letter.

Please direct all questions concerning your request to our Technical Evaluation Contractor at the following address:

Michael Baker Jr., Inc. 3601 Eisenhower Avenue, Suite 600 Alexandria, Virginia 22304

Attention: Ms. Pernille Buch-Pedersen (703) 317-6224

When you write us about your request, you must include the case number referenced above in your letter.

If you have any questions concerning FEMA policy, or the National Flood Insurance Program in general, please contact Mr. Max Yuan of our staff in Washington, DC, either by telephone at (202) 646-3843 or by facsimile at (202) 646-4596.

Sincerely,

Matthew B. Miller, P.E., Chief

rellip. 8 - mertetop

Hazards Study Branch Mitigation Directorate

cc:

Mr. Curt Chandler, P.E. Land Development Manager Department of Public Works City of Henderson



# CONSULTING ENGINEERS • PLANNERS • SURVEYORS

# PROVIDING QUALITY PROFESSIONAL SERVICES SINCE 1960

December 15, 1998 W.O.#5244

Michael Baker Jr., Inc. 3601 Eisenhower Avenue, Suite 600 Alexandria, VA 22304

Attention:

Mr. James Stitzel

SUBJECT:

Conditional Letter of Map Revision, Case No. 99-09-066R

This letter is in response to our telephone conversation 12/11/98. The following items are submitted per your request:

- 1. East C-1 Detention Basin Flood Control Improvements (11" x 17"), dated April 1998.
- 2. Sediment Study for East C-1 Detention Basin, dated September 29, 1997.
- 3. Final Design Calculation Notebook for the East C-1 Detention Basin, dated April 1998.

Figure 1.1 in Appendix A is a subbasin map. The Hec-2 run for the levee is found in Section 8, and the Hec-6 run for the basin is found in Section 9. The sediment study addresses bed loads.

The enclosed information should satisfy the issues of concern. If you require further information or if you have any additional questions please contact our office at (702) 247-4020.

Sincerely,

VTN Nevada

Lora Vennettilli, E.I.



# LETTER OF TRANSMITTAL

TO: Michael Baker Jr., Inc.

**DATE: 3/30/99** 

3601 Eisenhower Avenue Suite 600

Alexandria, VA 22304

PROJECT: East C-1 Detention Basin

**Conditional Letter of Map Revision** 

Case No. 99-09-066R

ATTN: James Stitzel

W.O. NO.: 5244

BY MAIL:

BY MESSENGER:

PICK-UP:

**EXPRESS MAIL:** 

XX

**FAX:** 

**FEDERAL EXPRESS:** 

No. Copies:

**Description** 

1 Zip diskette containing Hec-6 model runs for the Sediment Study

1 Copy of correspondence letter from Chen Engineering Technology

**README.TXT** printout 1

# **COMMENTS:**

Here is the supplemental data you requested for the above mentioned project. Please call if you need anything else. Thank-you.

MATERIAL SENT FOR THE FOLLOWING REASONS:

**CHECKING:** 

FILING:

XX

**APPROVAL:** 

**YOUR FILES:** 

OTHER:

CC:

V. Gookin, E.I.

**ABOVE MATERIAL RECEIVED BY:** 



# CHEN ENGINEERING TECHNOLOGY

March 10, 1999

Ms. Lora Vennettilli VTN Nevada 2727 South Rainbow Boulevard Las Vegas, NV 89102

Re: East C-1 Detention Basin (Project No. NV-VTN-01)

Dear Lora:

Enclosed please find a Zip diskette containing all the HEC-6 model runs made for the above referenced project. A text file README.TXT briefly explains the contents of these files. A printout of README.TXT is attached. Tables 2-5 of the report are reproduced here along with three new columns added to the end of the tables. These added columns show where the values of stream sediment yields (bed-material loads) were found from the HEC-6 output files, including the specific HEC-6 output file name, the line number and the section ID.

Each HEC-6 input data file contains the inflow sediment loading relationships specified at the upstream boundaries (e.g., see the enclosed portion of C4C-PMP.DAT) of the model. For channels with relatively coarse bed materials and long reaches, the effects of these inflow sediment loading relationships on sediment loading to downstream basins are relatively minor, because sediment transport tends to reach an equilibrium within a short distance due to channel erosion and deposition (e.g., see the enclosed portion of C4C-PMP.OUT).

I believe that the enclosed materials should provide you with sufficient information to substantiate how we determined the stream sediment yields. However, if you have any questions or need more information, please call me.

Sincerely yours,

Yung Hai Chen, Ph.D., P.E.

Principal Engineer

cc: Mr. Ken Gilbreth

(1) HEC-6 MODEL INPUT DATA AND OUTPUT FILES FOR BASIN C4C, NATURAL CONDITIONS

```
C4C-PMP.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C4C, PMP STORM, NATURAL COND.
C4C-PMP.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, PMP STORM, NATURAL COND.
C4C-10K.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C4C, 10K-YR STORM, NATURAL COND.
C4C-10K.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 10K-YR STORM, NATURAL COND.
C4C-8K.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C4C, 8K-YR STORM, NATURAL COND.
C4C-8K.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 8K-YR STORM, NATURAL COND.
             HEC-6 MODEL INPUT DATA FOR BASIN C4C, 1000-YR STORM, NATURAL COND. HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 1000-YR STORM, NATURAL COND.
C4C-1K.DAT:
C4C-1K.OUT:
C4C-500.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C4C, 500-YR STORM, NATURAL COND.
C4C-500.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 500-YR STORM, NATURAL COND.
C4C-100.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C4C, 100-YR STORM, NATURAL COND.
C4C-100.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 100-YR STORM, NATURAL COND.
              HEC-6 MODEL INPUT DATA FOR BASIN C4C, 50-YR STORM, NATURAL COND.
C4C-50.DAT:
              HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 50-YR STORM, NATURAL COND.
C4C-50.OUT:
              HEC-6 MODEL INPUT DATA FOR BASIN C4C, 25-YR STORM, NATURAL COND. HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 25-YR STORM, NATURAL COND.
C4C-25.DAT:
C4C-25.OUT:
              HEC-6 MODEL INPUT DATA FOR BASIN C4C, 10-YR STORM, NATURAL COND.
C4C-10.DAT:
              HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 10-YR STORM, NATURAL COND.
C4C-10.OUT:
              HEC-6 MODEL INPUT DATA FOR BASIN C4C, 5-YR STORM, NATURAL COND. HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 5-YR STORM, NATURAL COND.
C4C-5.DAT:
C4C-5.OUT:
```

(2) HEC-6 MODEL INPUT DATA AND OUTPUT FILES FOR NATURAL CONDITIONS IN BASIN C5D1

```
C5D1-PMP.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C5D1, PMP STORM, NATURAL COND.
C5D1-PMP.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, PMP STORM, NATURAL COND.
C5D1-10K.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C5D1, 10K-YR STORM, NATURAL COND.
C5D1-10K.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, 10K-YR STORM, NATURAL COND.
C5D1-8K.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C5D1, 8K-YR STORM, NATURAL COND.
C5D1-8K.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, 8K-YR STORM, NATURAL COND.
C5D1-1K.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C5D1, 1000-YR STORM, NATURAL COND.
C5D1-1K.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, 1000-YR STORM, NATURAL CONI
C5D1-500.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C5D1, 500-YR STORM, NATURAL COND.
C5D1-500.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, 500-YR STORM, NATURAL COND.
C5D1-100.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C5D1, 100-YR STORM, NATURAL COND.
C5D1-100.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, 100-YR STORM, NATURAL COND.
C5D1-50.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C5D1, 50-YR STORM, NATURAL COND.
C5D1-50.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, 50-YR STORM, NATURAL COND.
C5D1-25.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C5D1, 25-YR STORM, NATURAL COND.
C5D1-25.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, 25-YR STORM, NATURAL COND.
C5D1-10.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C5D1, 10-YR STORM, NATURAL COND.
C5D1-10.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, 10-YR STORM, NATURAL COND.
C5D1-5.DAT:
            HEC-6 MODEL INPUT DATA FOR BASIN C5D1, 5-YR STORM, NATURAL COND.
             HEC-6 MODEL OUTPUT FILE FOR BASIN C5D1, 5-YR STORM, NATURAL COND.
C5D1-5.OUT:
```

(3) HEC-6 MODEL INPUT DATA AND OUTPUT FILES FOR NATURAL CONDITIONS IN BASIN C5D2

```
C5D2-PMP.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C5D2, PMP-YR STORM, NATURAL COND. C5D2-PMP.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C5D2, PMP-YR STORM, NATURAL COND. C5D2-10K.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 10K-YR STORM, NATURAL COND. C5D2-10K.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C5D2, 10K-YR STORM, NATURAL COND. C5D2-8K.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 8K-YR STORM, NATURAL COND. C5D2-8K.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D2, 8K-YR STORM, NATURAL COND. C5D2-1K.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 1000-YR STORM, NATURAL COND. C5D2-1K.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D2, 1000-YR STORM, NATURAL COND. C5D2-500.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 500-YR STORM, NATURAL COND. C5D2-500.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C5D2, 500-YR STORM, NATURAL COND.
```

```
C5D2-100.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 100-YR STORM, NATURAL COND. C5D2-100.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C5D2, 100-YR STORM, NATURAL COND. C5D2-50.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 50-YR STORM, NATURAL COND. C5D2-25.DAT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D2, 50-YR STORM, NATURAL COND. C5D2-25.OUT: HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 25-YR STORM, NATURAL COND. C5D2-10.DAT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D2, 25-YR STORM, NATURAL COND. C5D2-10.OUT: HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 10-YR STORM, NATURAL COND. C5D2-5.DAT: HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 10-YR STORM, NATURAL COND. C5D2-5.OUT: HEC-6 MODEL INPUT DATA FOR BASIN C5D2, 5-YR STORM, NATURAL COND. C5D2-5.OUT: HEC-6 MODEL OUTPUT FILE FOR BASIN C5D2, 5-YR STORM, NATURAL COND.
```

# (4) HEC-6 MODEL INPUT DATA AND OUTPUT FILES FOR BASIN C4C, ALTERNATIVE A

```
C4C1-PMP.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C4C, PMP STORM, ALT. A. C4C1-PMP.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, PMP STORM, ALT. A. C4C1-100.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C4C, 100-YR STORM, ALT. A. C4C1-100.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 100-YR STORM, ALT. A.
```

(5) HEC-6 MODEL INPUT DATA AND OUTPUT FILES FOR BASIN C4C, ALT. B, C, D

```
C4C2-PMP.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C4C, PMP STORM, ALT. B. C4C2-PMP.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, PMP STORM, ALT. B. C4C3-PMP.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C4C, PMP STORM, ALT. C. C4C3-PMP.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, PMP STORM, ALT. C. C4C3-100.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C4C, 100-YR STORM, ALT. C. C4C3-100.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, 100-YR STORM, ALT. C. C4C4-PMP.DAT:HEC-6 MODEL INPUT DATA FOR BASIN C4C, PMP STORM, ALT. D. C4C4-PMP.OUT:HEC-6 MODEL OUTPUT FILE FOR BASIN C4C, PMP STORM, ALT. D.
```

Table 2. Sediment Yields for Subbasin C4CA

Return Period (years)	Peak Discharge (cfs)	Flood Volume (acre-feet)	Upland Sediment Yield, Y. (tons)	Stream Sediment Yield, Y <sub>bm</sub> (tons)	Total Sediment Yield (tons)	HEC-6 Output Filename	Line Number	Section ID
PMP	17,030	2,453	283,400	70,700	354,100	C4C-PMP.OUT	16609	1.000
10,000	7,450	1,193	119,100	36,500	155,600	C4C-10K.OUT	19488	1.000
8,000	6,404	1,109	105,100	28,900	134,000	C4C-8K.OUT	18437	1.000
1,000	4,378	687	64,900	21,300	86,200	C4C-1K.OUT	19745	1.000
500	2,809	438	39,400	14,400	53,800	C4C-500.OUT	17890	1.000
100	2,048	318	27,600	10,200	37,800	C4C-100.OUT	17023	1.000
50	1,596	248	20,900	9,670	30,570	C4C-50.OUT	16236	1.000
25	1,150	180	14,500	7,590	22,090	C4C-25.OUT	14667	1.000
10	646	104	7,730	4,560	12,290	C4C-10.OUT	13108	1.000
5	316	54	3,590	3,030	6,620	C4C-5.OUT	12067	1.000

Table 3. Sediment Yields for Subbasin C4CB

Return Period (years)	Peak Discharge (cfs)	Flood Volume (acre-feet)	Upland Sediment Yield, Y. (tons)	Stream Sediment Yield, Y <sub>bm</sub> (tons)	Total Sediment Yield (tons)	HEC-6 Output Filename	Line Number	Section ID
PMP	2,677	310	26,140	10,300	36,440	C4C-PMP.OUT	16488	0.070
10,000	1,145	151	10,860	6,410	17,270	C4C-10K.OUT	19367	0.070
8,000	985	129	9,140	5,500	14,640	C4C-8K.OUT	18318	0.070
1,000	674	87	5,930	3,940	9,870	C4C-1K.OUT	19624	0.070
500	433	55	3,580	2,710	6,290	C4C-500.OUT	17769	0.070
100	316	40	2,510	2,040	4,550	C4C-100.OUT	16900	0.070
50	247	31	1,900	1,710	3,610	C4C-50.OUT	16115	0.070
25	178	23	1,340	1,260	2,600	C4C-25.OUT	14546	0.070
10	100	13	700	800	1,500	C4C-10.OUT	12987	0.070
5	49	7	330	450	780	C4C-5.OUT	11946	0.070

Table 4. Sediment Yields for Subbasin C4CC

Return Period (years)	Peak Discharge (cfs)	Flood Volume (acre-feet)	Upland Sediment Yield, Y. (tons)	Stream Sediment Yield, Y <sub>bm</sub> (tons)	Total Sediment Yield (tons)	HEC-6 Output Filename	Line Number	Section ID
PMP	2,474	234	21,720	9,030	30,750	C4C-PMP.OUT	16553	0.080
10,000	1,035	114	8,910	4,500	13,410	C4C-10K.OUT	19432	0.080
8,000	891	97	7,490	4,210	11,700	C4C-8K,OUT	18381	0.080
1,000	611	66	4,890	2,730	7,620	C4C-1K.OUT	19689	0.080
500	393	42	2,960	1,930	4,890	C4C-500.OUT	17834	0.080
100	286	30	2,050	1,410	3,460	C4C-100.OUT	16967	0.080
50	223	24	1,580	1,160	2,740	C4C-50.OUT	16180	0.080
25	162	17	1,090	840	1,930	C4C-25.OUT	14611	0.080
10	91	10	580	550	1,130	C4C-10.OUT	13052	0.080
5	44	5	260	290	550	C4C-5.OUT	12011	0.080

Table 5. Sediment Yields for Subbasin C5D1

Return Period (years)	Peak Discharge (cfs)	Flood Volume (acre-feet)	Upland Sediment Yield, Y, (tons)	Stream Sediment Yield, Y <sub>bm</sub> (tons)	Total Sediment Yield (tons)	HEC-6 Output Filename	Line Number	Section ID
PMP	795	47	13,670	5,630	19,300	C5D1-PMP.OUT	3184	0.160
10,000	298	23	5,290	3,110	8,400	C5D1-10K.OUT	2605	0.160
8,000	257	19	4,380	2,720	7,100	C5D1-8K.OUT	2565	0.160
1,000	180	13	2,900	1,900	4,800	C5D1-1K.OUT	2025	0.160
500	119	9	1,870	1,270	3,140	C5D1-500.OUT	1546	0.160
100	89	6	1,270	980	2,250	C5D1-100.OUT	1447	0.160
50	70	5	1,000	790	1,790	C5D1-50.OUT	1400	0.160
25	52	4	750	650	1,400	C5D1-25.OUT	1363	0.160
10	31	2	380	500	880	C5D1-10.OUT	1295	0.160
5	16	1	180	250	430	C5D1-5.OUT	1017	0.160

Table 6. Sediment Yields for Subbasin C5D2

Return Period (years)	Peak Discharge (cfs)	Flood Volume (acre-feet)	Upland Sediment Yield, Y, (tons)	Stream Sediment Yield, Y <sub>bm</sub> (tons)	Total Sediment Yield (tons)	HEC-6 Output Filename	Line Number	Section ID
PMP	1,109	80	8,690	7,370	16,060	C5D2-PMP.OUT	3913	0.210
10,000	433	39	3,430	4,110	7,540	C5D2-10K.OUT	3457	0.210
8,000	375	33	2,890	3,570	6,460	C5D2-8K.OUT	3349	0.210
1,000	261	23	1,920	2,560	4,480	C5D2-1K.OUT	3121	0.210
500	171	15	1,200	1,670	2,870	C5D2-500.OUT	2260	0.210
100	127	11	850	1,240	2,090	C5D2-100.OUT	1918	0.210
50	100	8	620	1,040	1,660	C5D2-50.OUT	1867	0.210
25	73	6	440	770	1,210	C5D2-25.OUT	1747	0.210
10	43	4	260	490	750	C5D2-10.OUT	1576	0.210
5	22	2	120	280	400	C5D2-5.OUT	1462	0.210

						<i>C4</i>	C-PMP.	DAT		
· • • • • • • • • • • • • • • • • • • •	B/	AST C-1 I	DETENSION	BASIN			, ,			
TO T	ST	IIR-BASTN	C4C							ĺ
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X1	0.1	7	90. 2330. 2360.	260.	0.	0.	0.			- !
GR	2360.	0.	2330.	90.	2320.	120.	2320.	170.	2330.	260.
GR	2330.	340.	2360.	440.						!
HD	0.1	10.					_			ŗ
X1	0.35	7	90.	260.	1330.	1330.	1330.			
GR	2400.	0.	90. 2370. 2400.	90.	2360.	120.	2360.	170.	2370.	260.
GR	2370.	340.	2400.	440.						!
HD	0.35	10.	90. 2410.				2000			!
X1	0.60	7	90.	260.	1330.	1330.	1330.		~ ^	262
GR	2440.	0.	2410.	90.	2400.	120.	<b>24</b> 00.	170.	2410.	260.
CD	2410	340	2440	440 -						
HD	0.60	10.			22					!
X1	0.80	7	90. 2450.	260.	1100.	1100.	1100.	170	2450	260
GR	2480.	0.	2450.	90.	<b>244</b> U.	120.	2440.	I/U.	2 <b>4</b> 5U.	∠ou.
GR	2450.	340.	2480.	44U.						1
~	_	10.								,
Q.T.	2	5	90	270	590	590	580			!
ΔD	0.9u	٥	90. 2480.	270. an	2462	170.	2462	270	2480.	450
AD GD	2520. 2520.	0. 570.	240U.	<b>7</b> 0.	<b>4406</b> .	110.	4 <b>7</b> 02.	4,0.	<b>4300.</b>	300.
		10.								1
										!
¥± X1	1.0	6	250. 2480.	420.	350.	350.	350.			!
GR	2520.	0.	2480.	250.	2474.	370.	2474.	420.	2480.	470.
GR	2520.	630.		=	_					ŗ
										ŗ
	1.1	10. 6	200. 2490.	300.	410.	410.	410.			!
	2520.	U.	2490.	200:	2485.	250.	2485.	300.	2490.	360.
GR	2503.	600.								!
HD	1.1	10.		_		_				!
	1.67	6	200. 2602.	300.	3000.	3000.	3000.			
	2630.	0.	2602.	200.	2597.	250.	2597.	300.	2602.	360.
HD		10.								
X1		6	200.	300.		3000.		200		2.50
	2740.	0.	2714.	200.	2709.	250.	2709.	300.	2714.	360.
	2727.	600.								
HD		10.	200	200	2000	2000	2000			
X1		6	200.	300. 200	3000. 2821.			300.	2826.	360.
	2850.	0. 600	2826.	200.	2021.	250.	2041.	300.	4040.	30v.
	2839.	600.								ļ
HD V1		10. 6	200.	300.	3000	3000.	3000.			ļ
X1			200. 2938.	300. 200.		3000. 250.		300.	2938.	360.
	2962.	0. 600	<b>4730.</b>	۷٠٠.	4333.	2JU.	<i>4333</i> .	Juu.	47JU.	J00.
	2951.	600. 10.								
HD R.T		10.								
EJ ¢T	RIB									
ÇP										
T1		SUB-BASIN	י כערכ							
T2		)UD~DAG	Cacc							
T3										
NC		0.05	0.03	0.1	0.3					
X1		0.05 5	50.	170.	0.5	0.	0.			
	2468.	0.	2444.	50.	2441.	100.		170.	2468.	350.
HD		10.	<b>~</b>		~				~~~.	JJ 0 .
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X1	0.08	5	50.		430.		430.			2-2
	2480.	0.	2455.	50.	2452.	100.	2455.	170.	2480.	350.
	0.08	10.	240	220	1550.	1550.	1550.			
X1	0.37 2520.	5 0.	240. 2500.	320. 240.	2 <b>497</b> .	280.	2500.	320.	2520.	350.
	0.37	10.	2500.	240.	2431.	200.	2300.	520.	2320.	330.
X1	0.66	5	240.	320.	1550.	1550.	1550.			
	2582.	0.	2562.	240.	2559.	280.	2562.	320.	2582.	350.
	0.66	10.								
	0.95	5	240.	320.	1550.	1550.	1550.	220	2644.	250
	2644.	0.	2624.	240.	2621.	280.	2624.	320.	2644.	350.
	0.95 1.24	10. 5	240.	320.	1550.	1550.	1550.			
	2706.	0.	2686.	240.		280.		320.	2706.	350.
	1.24	10.								
EJ										
\$TR:										
CP	3	DACTN	CACD							
T1 T2	SUB	-BASIN	C4CB							
T3										
NC	0.05	0.05	0.03	0.1	0.3					
X1	0.00		110.		0.	0.	0.			
	2470.	0.	2466.	110.	2462.	160.	2466.	220.	2506.	350.
	0.00	10. 5	110.	220.	350.	350.	350.			
X1	0.07 2484.	0.	2480.	110.	2476.	160.	2480.	220.	2520.	350.
	0.07	10.	2400.	110.	2470.	200.	2100.	220.	2320.	330.
X1	0.16	5	180.	300.	480.	480.	480.			
	2503.	0.	2496.	180.	2493.	240.	2496.	300.	2520.	480.
	0.16	10.								
X1	0.44	_5	180.	300. 180.	1500. 2553.	1500. 240.	1500. 2556.	300.	2580.	480.
	2563. 0.44	0. 10.	2556.	100.	2555.	240.	2556.	300.	2560.	400.
X1	0.72	5	180.	300.	1500.	1500.	1500.	1.*		
	2623.	0.	2616.	180.	2613.	240.	2616.	300.	2640.	480.
HD	0.72	10.								
X1	1.00	_5	180.	300.	1500.	1500.	1500.	200	2722	400
	2683.	0.	2676.	180.	2673.	240.	2676.	300.	2700.	480.
HD X1	1.00 1.28	10. 5	180.	300.	1500.	1500.	1500.			
	2743.	0.		180.		240.	2736.	300.	2760.	480.
HD	1.28	10.								
X1	1.56	5			1500.		1500.			
	2803.	0.	2796.	180.	2793.	240.	2796.	300.	2820.	480.
HD	1.56	10.	OOO CECT	<b></b>						
EJ			OSS-SECTI TIONS FRO			מ_מווף	ASIN C4C			
							-PETER AN	D MULLEI	R COMBINA	TTON
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<b>T7</b>	÷									
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I1		5	_					~ ~	- 1. L	
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LF	MS	0.06	0.06	0.06	0.06	0.06	0.06	- <del>-</del>	sediment relationsh epstream ny C4C1	
LF	CS	0.10	0.10	0.10	0.10	0.10	0.10		·	

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           BED GRADATIONS FROM FIELD SAMPLES SUB-BASIN C4CC
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LFL VCS
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LFL VFG
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LFL FG

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T4 BED GRADATIONS FROM FIELD SAMPLES SUB-BASIN C4CB
            SEDIMENT TRANSPORT BY TOFFALETI AND MEYER-PETER AND MULLER COMBINATION
T5
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 RC
             2323.21 2323.45 2323.67 2323.89 2324.09 2324.28 2324.47 2324.65 2324.82
 RC
 RC
              2324.98 2325.14 2325.29 2325.44 2325.59 2325.73 2325.87 2326.00 2326.13
              2326.26 2326.39 2326.51 2326.63 2326.75 2326.86 2326.97 2327.09 2327.19
 RC
RC 2327.30 2327.41 2327.51 2327.61 2327.71 2327.81 2327.91 2328.01
 *
       Α
      1000. 234. 154.
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C4C-PMP: OUT

# STREAM SEGMENT # 1: EAST C-1 DETENSION BASIN (C4CA)

# SUMMARY TABLE: MASS AND VOLUME OF SEDIMENT

	SECTION	SEDIMENT	THROUGH	SECTION (ton	s)	SEDIM	ENT DEPOSIT	ED IN
	0101101			SILT	-		CUMULATIVE	
		TOTAL	SAND	этпт	CLAY	TOTAL	COMOLATIVE	SAN
	INFLOW	4601.	4601.	0.	0.	3664.		
/	3.380	71614.	71614.	0.	0.	-53376.		-5337
/	2.810	76514.	76514.	0.	0.	-3903.	-57279.	-390
	2.240	75939.	75939.	0.	0.	458.	-56820.	45
	1.670	75937.	75937.	0.	0.	2.	-56819.	
- 1	1.100	63131.	63131.	0.	0.	10200.	-46619.	1020
1	1.000	70700.	70700.	0.	0.	-6029.	-52648.	-602
	TRIB	6932.	6932.	0.	0.	5522.		
	0.900	88673.	88673.	0.	0.	-8793.	-61442.	-879
	TRIB	3969.	3969.	0.	0.	3162.		
	0.800	105255. 3	105255. 97643.	0.	0. 0.	-10046. 6063.		-1004
- 1	0.600		-					606
- 1	0.350	89055.	89055.	0.	0.	6840.		684
\	0.100	38548.	38548.	0.	0.	40229.	-18356.	4022

Based on inflow sediment rating relationship specified at the upstream boundary of C4CA.

