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**Stantec**  
**88100091**

**April 1, 2002**

**Michael Baker Jr., Inc.**  
**3601 Eisenhower Avenue, Suite 600**  
**Alexandria, VA 22304**

**Subject: Conditional Letter of Map Revision (C-LOMR) request for Green Park Channel/Culvert System for Flood Insurance Rate Maps (FIRM) 32003C2568 D, 32003C2569 D, and 32003C2910 D**

**Attention: Ms. Pernille Buch-Pedersen**

Dear Ms Buch-Pedersen:

Stantec Consultants is submitting the Green Park Channel/Culvert System for your review and approval of this Conditional Letter of Map Revision (C-LOMR) request. Enclosed within this request is the hydraulic analysis, HEC-RAS and WSPG, for the future Green Park Channel/Culvert System. Also provided are the C-LOMR application forms, ALTA (Plat Maps), and a revised flood map showing that effect of the Green Park Holdings development. The Green Park Channel/Culvert System is comprised of two distinct conveyance facilities, the North Box and the South Box. The basis of this C-LOMR request is the revised hydrology determined by G. C. Wallace for the Duck Creek Flood Insurance Study (FIS) that is also being submitted at this time.

Buildings

Environment

Industrial

Transportation

Urban Land

Should you have additional questions and/or require further information please do not hesitate to call Alexander Kingston at Stantec Consultants at (702) 258-0115

Sincerely,  
**STANTEC CONSULTANTS**

Alexander Kingston, E.I.T.  
Senior Hydrologist



**FEDERAL EMERGENCY MANAGEMENT AGENCY  
REVISION REQUESTER AND COMMUNITY OFFICIAL**

O.M.B No. 3067-0148  
Expires April 30, 2001

**PUBLIC BURDEN DISCLOSURE NOTICE**

Public reporting burden for this form is estimated to average 2.13 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing and reviewing the form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, Federal Emergency Management Agency, 500 C Street, S.W., Washington DC 20472; and to the Office of Management and Budget, Paperwork Reduction Project (3067-0148), Washington, DC 20503.

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**1. REQUESTED RESPONSE FROM FEMA**

This request is for a:

- ☒ **CLOMR** A letter from FEMA commenting on whether a proposed project, if built as proposed, would justify a map revision, or proposed hydrology changes (See 44 CFR Ch. 1, Parts 60,65 & 72).
- ☐ **LOMR** A letter from FEMA officially revising the current NFIP map to show the changes to floodplains, floodway or flood elevations. LOMRs typically decrease flood hazards. (See 44 CFR Ch. 1 Parts 60 & 65.)
- ☐ **Other** Describe: \_\_\_\_\_

**2. OVERVIEW**

1. The basis for this revision request is (are): (check all that apply)

- ☒ Physical Change                      ☐ Improved Methodology/Data                      ☒ Floodway Revision

☐ Other Describe: \_\_\_\_\_

Note: A photograph is not required, but is very helpful during review.

2. Flooding Source: Duck Creek

3. Project Name/Identifier: Green Park Channel/Culvert System

4. FEMA zone designations affected: "AE" "A"  
(example: A, AH, AO, A1-A30, A99, AE, V, V1-V30, VE, B, C, D, X)

5. The NFIP map panel(s) affected for all impacted communities is (are):

Community No.	Community Name	State	Map No.	Panel No.	Effective Date
Ex: 480301	Katy, City	TX	480301	0005D	02/08/83
480287	Harris County	TX	48201C	0220G	09/28/90
32003	Clark County	NV	32003C	2569D	08/16/95
				2568D	08/16/95
				2010D	08/16/95

6. The area of revision encompasses the following types of flooding and structures. Check all that apply.

**Types of Flooding**

- ☒ Riverine  
☐ Coastal  
☐ Alluvial fan  
☐ Shallow Flooding (e.g. Zones AO and AH)  
☐ Lakes  
☐ Other (describe) \_\_\_\_\_

**Structures**

- ☒ Channelization  
☐ Levee/Floodwall  
☒ Bridge/Culvert  
☐ Dam  
☐ Fill  
☐ Other (describe) \_\_\_\_\_

**PLEASE REFER TO THE INSTRUCTIONS FOR THE APPROPRIATE MAILING ADDRESS**

#### 4. ENCROACHMENT INFORMATION

1. Does the State have jurisdiction over the floodway or its adoption by communities participating in the NFIP? ☐ Yes ☒ No

If Yes, attach a copy of a letter notifying the appropriate State agency of the floodway revision and documentation of the approval of the revised floodway by the appropriate State agency.

2. Does the development in the floodway cause the 1% annual chance (base) elevation to increase at any location by more than 0.000 feet? ☐ Yes ☒ No ☐ N/A
3. Does the cumulative effect of all development that has occurred since the effective SFHA was originally identified cause the base flood elevation to increase at any location by more than one foot (or other increase limit if community or state has adopted more stringent criteria - even if a floodway has not been delineated by FEMA)? ☐ Yes ☒ No

If the answer to either items is Yes, please attach documentation that all requirements of Section 65.12 of the NFIP regulations have been met, regarding evaluation of alternatives, notice to individual legal property owners, concurrence of CEO, and certification that no insurable structures are impacted.

#### 5. MAINTENANCE RESPONSIBILITY

The community is willing to assume responsibility for ☐ performing ☒ overseeing compliance with the maintenance and operation plans of the Green Park Channel/Culvert System flood (Name) control structure. If not performed promptly by an owner other than the community, the community will provide the necessary services without cost to the Federal government.

Operation and maintenance plans are attached. ☐ Yes ☒ No ☐ N/A

#### 6. REVIEW FEE

The review fee for the appropriate request category has been included. ☒ Yes Fee amount: \$3,100.00  
OR

This request is based on a federally sponsored flood-control project where 50 percent or more of the project's cost is federally sponsored, or the request is based on detailed hydrologic and hydraulic studies conducted by Federal, State, or local agencies to replace approximate studies conducted by FEMA and shown on the effective FIRM; thus the project is fee exempt. ☐ Yes

Please see Instructions for Fee Amounts

#### 7. SIGNATURE

Note: I understand that my signature indicates that all information submitted in support of this request is correct

Alexander Kingston

Signature of Revision Requester

Alexander Kingston

Printed Name and Title of Revision Requester

Stantec Consulting

Company Name

(702) 258-0115

Telephone No.

Date

Note: Signature indicates that the community understands, from the revision requester, the impacts of the revision on flooding conditions in the community.

Signature of Community Official

Printed Name and Title of Community Official

Community Name

Telephone No.

Date

#### CERTIFICATION BY REGISTERED PROFESSIONAL ENGINEER

This certification is in accordance with 24 C.F.R. 1, Sect 65.2

DENNIS WEATZLER

Printed Name and Title of Revision Requester

Registr No. 11970

Expires (Date) 10/16/02

State NV

Type of License/Expertise: CIVIL

#### Check which forms have been included with this request

##### Form Name and (Number)

- ☐ Hydrologic (3)  
☒ Hydraulic (4)  
☐ Mapping (5)  
☒ Channelization (6)  
☒ Bridge/Culvert (7)  
☐ Levee/Floodwall (8)  
☐ Coastal (9)  
☐ Coastal Structures (10)  
☐ Dam (11)  
☐ Alluvial Fan (12)

##### Required If .....

- new or revised discharges  
new or revised water-surface elevations  
floodplain/floodway changes  
channel is modified  
addition/revision of bridge/culvert  
addition/revision of levee/floodwall  
new or revised coastal elevations  
addition/revision of coastal structure  
addition/revision of dam  
structures proposed on alluvial fan

FEDERAL EMERGENCY MANAGEMENT AGENCY  
RIVERINE/COASTAL MAPPING

O.M.B No. 3067-0148  
Expires April 30, 2001

PUBLIC BURDEN DISCLOSURE NOTICE

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Note: Fill out one form for each flooding source studied

Community Name: Clark County

Flooding Source: Duck Creek

Project Name/Identifier: Green Park Channel/Culvert System

This is a ☒ Manual ☐ Digital submission. Digital map submissions may be used to update digital FIRMs (DFIRMs). For updating DFIRMs, these submissions must be coordinated with FEMA Headquarters as far in advance as possible.

1. MAPPING CHANGES

1. A topographic workmap must be submitted showing the following information (check N/A when not applicable):

- |   |   |  |   |
|---|---|--|---|
| a. Revised approximate 100-year floodplain boundaries (Zone A)  | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| b. Revised detailed 100- and 500-year floodplain boundaries.  | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| c. Revised floodway boundaries  | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| d. Location and alignment of all cross sections with stationing control indicated.  | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| e. Stream alignments, road alignments and dam alignments.   | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| f. Current community boundaries.  | <input type="checkbox"/> Yes            | <input checked="" type="checkbox"/> No | <input checked="" type="checkbox"/> N/A |
| g. Effective 100-year floodplain and floodway boundaries from FIRM/FBFM reduced or enlarged to the scale of the topographic workmap | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| h. Tie-ins between the effective and revised 100-, 500-year and floodway boundaries   | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| i. The requester's property boundaries and community easements  | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| j. The signed certification of a registered professional engineer   | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| k. Location and description of reference marks  | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| l. Vertical datum (example: NGVD, NAVD)   | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input type="checkbox"/> N/A            |
| m. Coastal zone designations tie into adjacent areas not being revised  | <input checked="" type="checkbox"/> Yes | <input type="checkbox"/> No            | <input checked="" type="checkbox"/> N/A |
| n. Location and alignment of all coastal transects used to revise the coastal analyze   | <input type="checkbox"/> Yes            | <input type="checkbox"/> No            | <input checked="" type="checkbox"/> N/A |
| o. V-zone has been delineated to extend landward to the heel of the primary frontal dune  | <input type="checkbox"/> Yes            | <input type="checkbox"/> No            | <input checked="" type="checkbox"/> N/A |

If any items are marked No or N/A please attach an explanation.

2. What is the source and date of the updated topographic information (example: orthophoto maps, July 1985; filed survey, May 1979, beach profile, June 1987 etc.)?

Orthophoto/Grading Plan Map

October, 2000

3. What is the scale and contour interval of the following workmaps?

Effective FIS Scale 1"=400 Contour Interval 4-foot

Revision Request Scale 1"=400 Contour Interval 4-foot

NOTE: Revised topographic information must be of equal or greater detail than effective.

4. Attach an annotated FIRM/FBFM at the scale of the effective FIRM/FBFM showing the revised 100- and 500-year floodplain and the floodway boundaries and how they tie into those shown on the effective FIRM/FBFM downstream and upstream of the revisions or adjacent to the area of revision for coastal studies. FIRM/FBFM attached? ☒ Yes ☐ No

PLEASE REFER TO THE INSTRUCTIONS FOR THE APPROPRIATE MAILING ADDRESS

## 2. EARTH FILL PLACEMENT

1. The fill is: ☒ Existing ☐ Proposed

2. Has fill been/will be placed in the regulatory floodway?

☐ Yes

☒ No

If Yes, please attach completed Riverine Hydraulic Analysis Form (Form 4).

3. Has fill been/will be placed in floodway fringe (area between the floodway and 100-year floodplain boundaries)?

☐ Yes

☒ No

If Yes, then complete A, B, C, and D below.

a. Are fill slopes for granular materials steeper than one vertical on one-and-one-half horizontal?

☐ Yes

☐ No

If Yes, justify steeper slopes \_\_\_\_\_

b. Is adequate erosion protection provided for fill slopes exposed to moving flood waters? (Slopes exposed to flows with velocities of up to 5 feet per second (fps) during the 100-year flood must, at a minimum, be protected by a cover of grass, vines, weeds, or similar vegetation; slopes exposed to flows with velocities greater than 5 fps during the 100-year flood must, at a minimum, be protected by stone or rock riprap.)

☐ Yes

☐ No

If No, describe erosion protection provided \_\_\_\_\_

c. Has all fill placed in revised 100-year floodplain been compacted to 95 percent of the maximum density obtainable with the Standard Proctor Test Method or acceptable equivalent method? ☐ Yes ☐ No

d. Can structures conceivably be constructed on the fill at any time in the future? ☐ Yes ☐ No

If Yes, attach certification of fill compaction (item 3c. above) by the community's NFIP permit official, a registered professional Engineer, or an accredited soils engineer in accordance with Subparagraph 65.5(a)(6) of the NFIP regulations.

Fill certification attached

☐ Yes

☐ No

4. Has fill been/will be placed in a V zone?

☐ Yes

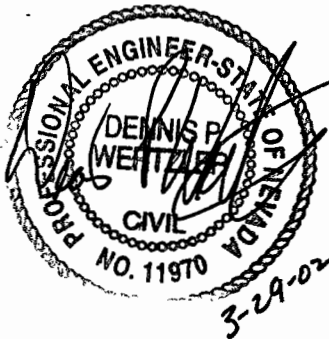
☒ No

If Yes, is the fill protected from erosion by a flood control structure such as a revetment or seawall?

☐ Yes

☐ No

If Yes, attach the Coastal Structures Form (Form 10).



FEDERAL EMERGENCY MANAGEMENT AGENCY  
CHANNELIZATION

O.M.B. Burden No. 3067-0148  
Expires April 30, 2001

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Community Name: Clark County  
Flooding Source: Duck Creek  
Project Name/Identifier: Green Park Channel/Culvert System

1. REACH TO BE REVISED

Describe the limits of the revision OR submit a copy of the FIRM with the revision area clearly highlighted.  
Copy of FIRM(s) attached depicting area of the revision (highlighted, or circled)? ☒ Yes

Downstream Limit: Las Vegas Blvd (North Box) Las Vegas Blvd (South Box)  
Upstream Limit: Parvin Street (North Box) I-15 (South Box)

2. CHANNEL DESCRIPTION

Attach the following information about the channel (check box if information has been provided):

- ☒ Description of the inlet and outlet  
☒ Description of the shape of the channel (both cross sectional and planimetric configuration) and its lining (channel bottom and sides):

3. ACCESSORY STRUCTURES

The channelization includes:

- ☐ Levees (Attach Levee/Floodwall System Analysis Form - Form 8)  
☒ Drop structures  
☐ Superelevated sections  
☒ Transitions in cross sectional geometry  
☐ Debris basin/detention basin  
☒ Energy dissipater  
☐ Other (Describe):

4. DRAWING CHECKLIST

Attach the plans of the channelization certified by a registered professional engineer. The plan detail and information should include (check box if information has been provided):

- ☒ Channel alignment and locations of inlet, outlet, and accessory structures  
☒ Channel lining  
☒ Typical cross sections and profiles of channel banks and invert

PLEASE REFER TO THE INSTRUCTIONS FOR THE APPROPRIATE MAILING ADDRESS

## 5. HYDRAULIC CONSIDERATIONS

1. The channel was designed to carry 5080 (cfs) and/or the 100-year flood. (North Box)
2. The design elevation in the channel based on: 1808 cfs 100-year  
2543 cfs 100-year (South Box)

- ☒ Subcritical flow  
☐ Critical flow  
☒ Supercritical flow  
☐ Energy grade line

3. If there is the potential for a hydraulic jump at the following locations, check the box(es) that apply and attach an explanation of how the hydraulic jump is controlled without affecting the stability of the channel.

N/A

- Inlet to channel? ☐ Yes
- Outlet of channel? ☐ Yes
- At Drop Structures? ☐ Yes
- At Transitions? ☐ Yes
- Other locations? ☐ Yes

Explanation Attached? ☒ Yes ☐ No ☐ N/A

## 6. SEDIMENT TRANSPORT CONSIDERATIONS

If there is any indication from historical records that sediment transport (including scour and deposition) can affect the 100-year (base flood) water-surface elevations; and/or based on the stream geomorphology, vegetative cover, development of the watershed and bank conditions, there is a potential for debris and sediment transport (including sewer and deposition) to affect the base flood water-surface elevations, then provide the following information (Check the box if provided):

- N/A
- ☐ Estimated sediment load
- ☐ Method used to estimate sediment transport
- ☐ Method used to estimate scour and/or deposition
- ☐ Method used to revise hydraulic or hydrologic analysis (model) to account for sediment transport

## **1.0 GENERAL LOCATION AND DEVELOPMENT DESCRIPTION**

### **1.1 Scope**

The purpose of this report is to provide detailed hydrologic and hydraulic analyses to support the design of several major storm drain facilities that will collect and convey the most significant offsite storm flows across the proposed site. This study will address the existing conditions that affect the proposed storm drain facilities onsite and offsite drainage. It is our intention that the construction of these facilities will allow the overall subject site to be removed from the FEMA designated flood zone. It is expected that this study will be submitted to FEMA with the F.I.S. restudy currently being prepared by the Clark County Regional Flood Control District (CCRFCDD) to obtain a Conditional Letter of Map Revision (CLOMR) for our subject site. Upon construction of the proposed facilities a Letter of Map Revision (LOMR) will be obtained. The criteria set forth in the Clark County Regional Flood Control District "Hydrologic Criteria and Drainage Design Manual," hereafter referred to as the "Drainage Criteria Manual," (Reference 6) has been used as the technical basis for this study. It is expected that the subject site will eventually be developed as a mixed-use commercial property. Additional technical drainage studies will be required to support the design of any onsite development and perimeter streets.

### **1.2 Location**

The subject is located within Clark County, Nevada. The project site lies south of Silverado Ranch Parkway (Gomer Road), west of Las Vegas Boulevard, north of Cactus Avenue, and east of Interstate I-15 (See Figure 1, Vicinity Map). The proposed project is within a portion of Section 29, Township 22 South, Range 61 East, M.D.M., Clark County Nevada.

### **1.3 Description of Project**

The overall project site consists of approximately 200 acres. The future land use of the subject site has not been determined beyond the general

**Job #: 0288.0040**



designation of "mixed commercial". At this time, the proposed facilities are limited to the construction of extensive storm drain facilities consisting of multiple arch culverts (See Figure 2, *Master Drainage Facility Exhibit*). The site itself is typical and undeveloped desert, with the exception of a few scattered single-family homes. There are several natural washes throughout the site and existing bridges and culverts along the eastern and western property boundaries (I-15 to the west and Las Vegas Blvd to the east). The north portion of the site generally slopes to the east at approximately 1.3 %. The south portion of the site generally slopes to the northeast at approximately 1%.

The proposed storm drains will collect flow from the culvert crossings under I-15, and the flows that impact Cactus Road from the south (flow comes from under Lake Mead Boulevard). All the flows will be collected into the proposed storm drains and will be conveyed to Las Vegas Boulevard. The flows will be released on the east side of Las Vegas Boulevard in the approximate location of the historic flows. Historically, the offsite flows that impacted the site from the south and southwest have been routed to Las Vegas Boulevard where the flows pond up and weir over the top of Las Vegas Boulevard. Some flows would be conveyed under Las Vegas Boulevard via several small, undersized, existing box culverts.

A large portion of the overall site lies within FEMA designated flood zones. The flood zones have been analyzed in the past and water surface elevations have been determined. The limits of the established flood zone within the proposed site was based on an assumption in the original Duck Creek Hydrologic Unit F.I.S. Study (Ref 9) that a significant amount of flows weir over the top of I-15 from the west. However, the recently submitted FIS study by G.C. Wallace (Ref 8) has shown that a reduced flow overtops I-15. The majority of the flows impacting I-15 are forced north by the elevated freeway.

We have met the CCRFCD and it is our understanding that they are in the process of preparing a Duck Creek F.I.S. restudy that would incorporate

**Started**

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### Consultants

Legend

Notes

<b>Revision</b>	By _____	Appd. _____	YY.MM.DD _____
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____
_____	_____	_____	_____
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<b>Issued</b>	By _____	Appd. _____	YY.MM.DD _____
_____	_____	_____	_____
<b>File Name:</b> _____	Dwn _____	Chkd _____	Dsan _____
_____	_____	_____	YY.MM.DD _____

Permit-Seal

Client/Project  
**GREEN PARK HOLDINGS, LLC**

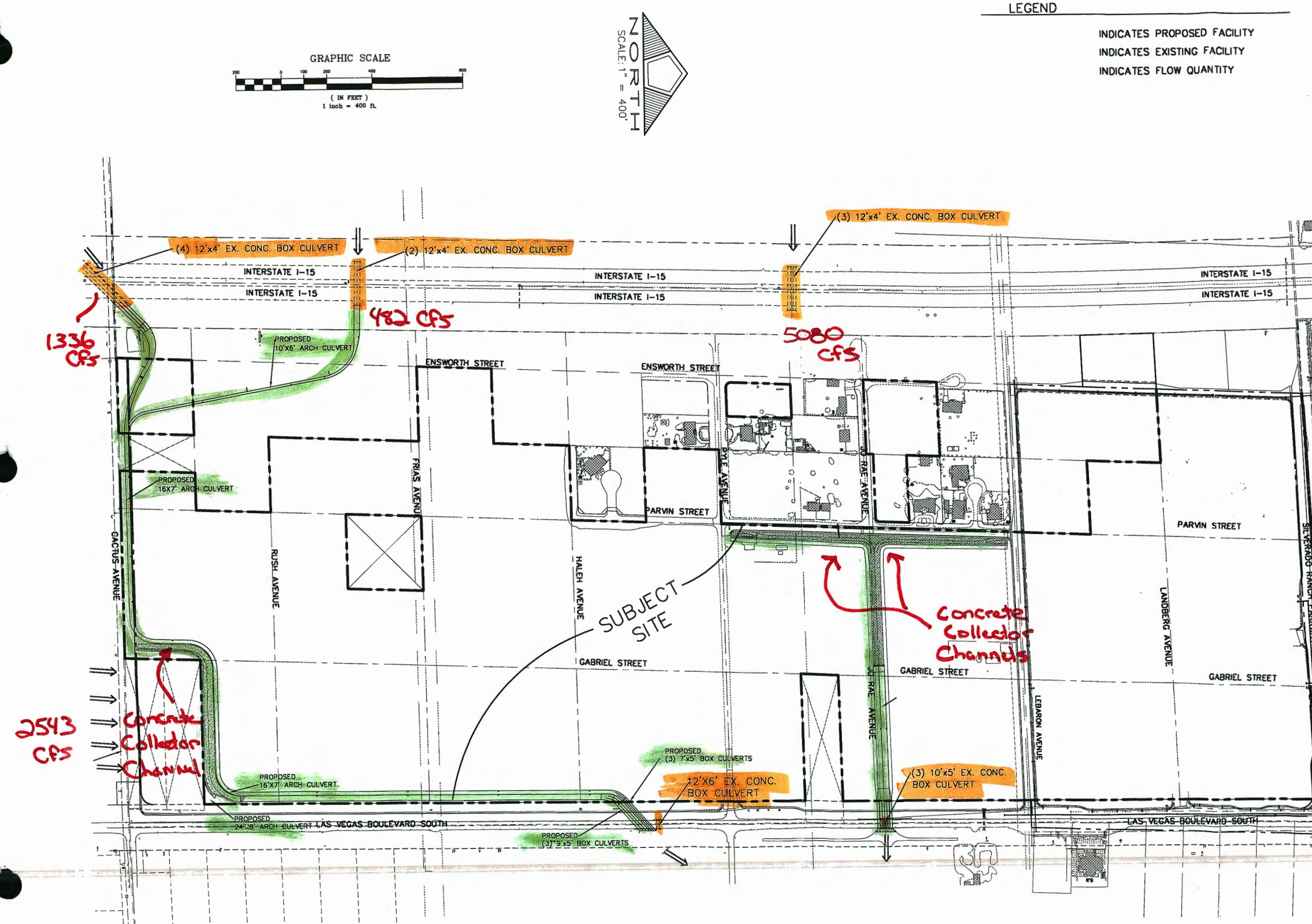
Las Vegas NV U.S.A.

Title  
MASTER DRAINAGE FACILITY EXHIBIT

Project No. 88100091	Scale 1"=400'	
Drawing No.	Sheet	Revision

10

1 of 1  
Figure 2



these latest findings. This F.I.S. restudy would redefine the flood zones across the subject site. It is anticipated that the new flood zones would show that the flows were contained within the proposed storm drain. Stantec has met with CCRFCD and G.C. Wallace, who is preparing the new Duck Creek F.I.S. study. The meeting was to discuss issues such as: what hydrologic data is available, which data presents the most accurate representation of the flows that impact the subject site, timing of the F.I.S. restudy, scope of the F.I.S. restudy, possible impacts of the F.I.S. restudy on the MPU, and discussion on how our proposed facilities would tie into the MPU facilities. It was mutually agreed that the offsite flows impacting the site have been well quantified by several previously approved studies.

The flow that impacts the site from the south was quantified in the Nimbus "*Pittman Wash F.I.S. Restudy*" (Ref 7). This information was incorporated in the hydrology used by G.C. Wallace for the Duck Creek FIS study. This hydrology has been accepted by Stantec for the purposes of determining all onsite hydraulics.

As a result of our meeting with CCRFCD, it has been determined that because of the timing of this study and the new Duck Creek F.I.S. study, our project could be included as an appendix to the F.I.S. submittal to FEMA later this year. This FEMA submittal will be the equivalent to a CLOMR (conditional letter of map revision). Once the Duck Creek F.I.S. study is approved and the storm drains built, we will submit for a LOMR (letter of map revision)

## 2.0 DRAINAGE BASIN DESCRIPTION

### 2.1 Offsite Drainage Description

The offsite drainage basins that contribute flows impacting the subject site are quite large and the flows are significant. The offsite basins that impact the subject site are from two distinct watersheds relative to the subject site, southern and southwestern (See Figure 3, *Watershed Map*).

#### Southern Watershed

The southern watershed is approximately 35 square miles. The majority of the flows are generated from basins located south of Lake Mead Boulevard (St. Rose Parkway). The delineation of the drainage basins, the hydrology, and the direction of the flows from the area south of Lake Mead Boulevard were studied extensively as part of the approved *Pittman Wash F.I.S* study (Ref 7). In our meetings with CCRFCD, during the research portion of preparing this study, the attendees (include Stantec Staff, CCRFCD staff and GC Wallace staff) agreed that the *Pittman Wash F.I.S.* study represented the best available information that describes the flows south of Lake Mead Boulevard. The hydrology used by G.C. Wallace incorporated this information. The most important aspect as it relates to our subject site, is the determination of the flow diversions south of Lake Mead Boulevard. A review of the hydrology shows that 2,534 cfs at Cactus Avenue during a 100 year storm event.

#### Southwestern Watershed

The southwestern watershed is approximately 85 square miles of land west of Interstate 15. This area includes the recently developed Southern Highlands master planned community.

This southwestern watershed area has been extensively studied as part of the recently submitted G.C. Wallace Duck Creek FIS study (Ref 8). This study not only quantified the flows from basins west of I-15 but also determined the diversion of the flows through existing drainage facilities under and over I-15 from Cactus Avenue to Blue Diamond interchange.



This study clearly shows that I-15 acts as a dike just west of the subject site. Attached is the exhibits (Figure 3A and 3B, *Duck Creek FIS Maps 6A and 6B*) that best shows the accepted flow diversions. There are three culverts that impact the site which are identified in the FIS study as I-151, I-152 and I-153. These culverts carry 1,336 cfs, 482 cfs, and 1,571 cfs (This flow when combined with the flow over I-15 is 5080 cfs) respectively.

In our meetings with CCRFCD during the research portion of preparing this study, the attendees (include Stantec staff, CCRFCD staff and GC Wallace staff) agreed that this study represented the best available information that describes the flows west of the subject site.

## 2.2 Onsite Drainage Description

The subject site is predominately undeveloped desert with several large washes that cut through the site generally from west and southwest to the east. The vegetation and terrain are typical of the local desert.

The subject site is the historical confluence point for large offsite flows from the south and west. The onsite drainage patterns for the northern portion of subject site is generally from the west to the east at a slope of approximately 1.3%. The onsite drainage patterns for the southern portion of the subject site are generally from the south to the northeast at a slope of approximately 1%. The site currently drains towards Las Vegas Boulevard, which appears to have been constructed well above the grade of the natural ground. The existing storm drain facilities under Las Vegas Boulevard are undersized causing the existing flows to pond up and weir over the roadway during major storm events.

### 2.3 FEMA Flood Hazard Zone

The Special Flood Hazard Areas for Clark County, Nevada are outlined in the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Maps dated August 16, 1995. Upon reviewing Community Panel No. 32003 C 2568 D and No. 32003 C 2910 D (Figure 4, *FEMA Map*) it was determined that the subject site lies within the following zones:

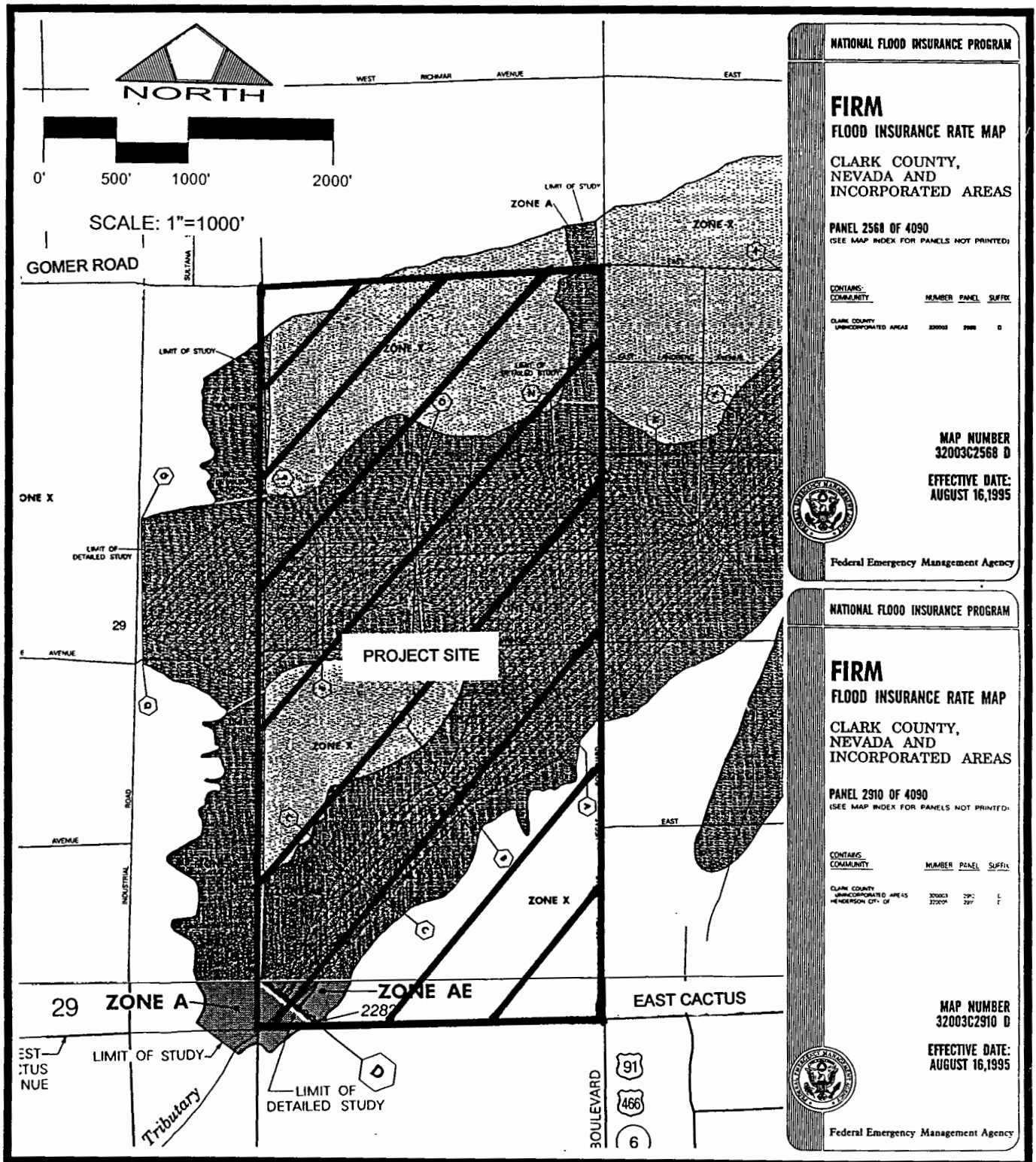
- Zone "A", and area determined to be inside the 100-year floodplain without available base flood elevation.
- Zone "AE" with Floodway, an area determined to be inside the 100-year floodplain where base flood elevations have been determined.
- Zone "X" with portions determined to be outside and inside of the 500-year floodplain.

It is expected that the limits of all the flood zones on the site will be substantially revised as a result of the pending F.I.S. restudy. The F.I.S. study will also include a CLOMR for the proposed storm drains contained within this study (See Figure 5, *Flood Zone Exhibit*).

### 2.4 Regional Flood Control Master Plan Update

#### Existing Facilities

There are existing local and proposed CCRFCD facilities that cross the subject site (See Figure 6A, Figure 6B, and Figure 7, *CCRFCD MPU Facilities*). The proposed site has three existing bridge spans on the western boundary that convey flows under Interstate I-15 identified as local facilities in the CCRFCD Master Plan. The most northerly is a 36-foot wide, 4-foot deep, three span bridge (CCRFCD local facility IS15 0233). The middle structure is a 24-foot wide, 4-foot deep two span bridge (CCRFCD local facility IS15 0199). The southern most structure is at Cactus Avenue and does not have a local identifier but consists of a 36-foot wide, 4-foot deep three span bridge. Several box culverts currently cross under Las Vegas Boulevard. The existing facilities under Las Vegas Boulevard are unaccounted for in the Master Plan Update.



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## FIRM MAP

Greenpark Development  
Greenpark

Figure 4

Job #: 0288.0040



TABLE 2: SUMMARY OF DIVERSION OUTPUT

DIVERSION ID	FACILITY ID	FACILITY DESCRIPTION	DIVERSION Q100 (CFS)	DIVERSION DIRECTION	BYPASS Q100 (CFS)	BYPASS DIRECTION
DVBD2	BD2	2-36" RCP	466	EAST	786	NORTH
DVBD3	BD3	7' x 4' RCB	290	EAST	1281	NORTH
DVBD4	BD4	2-36" RCP	227	EAST	1022	NORTH
DVBD7	NA	NA	1260	EAST	630	EAST
DV151	I151	3-12" x 4' RCB	1326	EAST	1301	NORTH
DV152	I152	2-12" x 4' RCB	482	EAST	816	NORTH
DV153	I153	3-12" x 4' RCB	5125	EAST	4385	NORTH
DV154	I154	2-12" x 4' RCB	1230	EAST	3516	NORTH
DV155	I155	3-12" x 5' RCB	2270	EAST	1421	NORTH
DVPITT	NA	NA	3160	EAST	3282	NORTH
DVROSE	NA	NA	644	NORTHEAST	2683	NORTH
TORAW	NA	NA	611	SOUTH	2605	EAST

TABLE 1: SUMMARY OF HEC-1 MODEL

Location	Area	SDN	DARF	Flow	Location	Area	SDN	DARF	Flow
B13A	1.29	3	0.930	933	D010	10.43	3	0.957	3520
B13B	0.54	3	0.975	223	D07	4.26	3	0.908	1804
PT1	1.83	3	0.950	1050	PT7	32.65	5	0.725	6413
PT2	0.82	3	0.975	419	DV15B	0.57	3	0.975	391
PT2	2.84	3	0.925	1252	PT8	33.21	5	0.725	6442
DVBD2	2.64	3	0.925	466	DVPITT	33.21	5	0.725	2160
DVBD2	2.64	3	0.925	786	DVPITT	33.21	5	0.725	3282
B13C	2.01	3	0.925	1432	DVROSE	33.21	5	0.725	644
PT3	4.85	3	0.908	1550	DVROSE	33.21	5	0.725	2634
DVBD3	4.85	3	0.908	290	D4A	1.87	3	0.950	1634
DVBD4	4.85	3	0.908	1261	PT25	117.75	5	0.580	6470
DVBD4	4.85	3	0.908	227	D4B	2.03	3	0.925	584
DVBD4	4.85	3	0.908	1022	PT26	119.78	5	0.580	6470*
B115	1.12	3	0.950	459	PT27	94.15	5	0.640	3499
HYDRO1	0.53	3	0.975	823	PT28	1.26	3	0.950	732
PT4	6.40	3	0.895	1961	PT28	132.53	5	0.580	9693
DVBD7	6.40	3	0.895	1290	LDCDB	132.53	5	0.580	7261
DVBD7	6.40	3	0.895	630	D4A	1.33	3	0.950	1261
B14B	0.99	3	0.975	440	PT29	133.86	5	0.580	7484
PT22W	7.39	3	0.885	754	B16B	0.76	3	0.975	846
DV15A	10.21	4	0.857	2630	PT30	134.56	5	0.580	7484*
DV151	10.21	4	0.857	1328	B16A	0.91	3	0.975	411
DV151	10.21	4	0.857	1291	D5B	0.60	3	0.975	398
DV152	10.21	4	0.857	482	PT31	136.07	5	0.580	7484
DV152	10.21	4	0.857	919	PT32	143.12	5	0.580	8477
DV152	10.21	4	0.857	3753	HYDRO2	0.36	3	0.950	36
D005	2.73	3	0.925	1464	B12	0.97	3	0.975	262
PT5	12.27	4	0.812	3550	PT33W	0.64	3	0.950	244
D02	5.91	3	0.903	2778	PT33	7.78	3	0.885	1286
D03	11.99	4	0.850	4758	B15A	0.96	3	0.975	509
D04	6.95	3	0.895	3153	PT34W	8.71	4	0.875	1590
PT10	18.94	5	0.804	5920	PT34	145.42	5	0.580	8551
D006	5.52	4	0.875	3559	MNWB	0.44	3	0.980	236
PT11	33.37	5	0.725	7588	MNWC	0.80	3	0.975	322
D2A	0.99	3	0.975	1028	PT35	1.04	3	0.950	529
PT12	46.63	5	0.685	9635	MNWD	0.08	3	0.950	72
D2B	7.50	3	0.885	2188	PT36	1.12	3	0.950	549
PT13	84.13	5	0.640	8471	MC2	0.92	3	0.975	367
D06	11.88	4	0.850	3919	PT37	2.03	3	0.925	846
D3	8.64	3	0.895	1518	B10A	1.60	3	0.950	1361
PT14	82.66	5	0.640	9510	PT38	2.64	3	0.915	1428
DV153	82.66	5	0.640	5125	PT39	149.06	5	0.580	8562
DV153	82.66	5	0.640	4385	D6	1.58	3	0.950	1098
D1A	2.29	3	0.925	1156	PT40	150.62	5	0.530	8562*
D1B	0.32	3	0.990	149	D7	0.85	3	0.975	399
PT15	2.81	3	0.925	1299	PT41	151.47	5	0.530	8562*
D1C	0.56	3	0.975	203	D8	1.26	3	0.950	1169
PT16	3.17	3	0.915	1420	PT42	152.73	5	0.530	8562*
D1D	1.23	3	0.950	764	B10B	0.85	3	0.975	476
D1E	0.22	3	0.990	81	D9A	0.86	3	0.975	227
D1F	0.81	3	0.975	162	PT43	1.71	3	0.950	646
PT17	5.43	3	0.903	1920	PT44	154.44	5	0.530	8562*
B18A	1.50	3	0.950	294	D9B	1.54	3	0.950	915
PT18	89.59	5	0.640	4746	PT45	155.99	5	0.530	8562*
DV154	89.59	5	0.640	1230	D10	2.51	3	0.925	1653
B18B	1.92	3	0.950	3516	PT46	156.49	5	0.530	8562*
PT19	94.15	5	0.640	3890	TORAW	2.80	3	0.925	3216
DV155	94.15	5	0.640	2270	TORAW	2.80	3	0.925	2605
DV155	94.15	5	0.640	1421	MCUB	2.80	3	0.925	52
B14A	1.35	3	0.950	1145	PT47	2.80	3	0.925	558
PT20	4.85	3	0.908	516	RH	1.36	3	0.950	551
PT21	97.51	5	0.640	1441	PT48W	4.16	3	0.905	1158
PT22	100.25	5	0.590	1441*	B20	0.79	3	0.975	395
B15B	0.95	3	0.975	336	PT48	163.44	5	0.530	8562*
PT23	101.20	5	0.590	1441*	D12	0.18	3	0.990	72
PT24	10.21	4	0.857	1605	D11A	0.16	3	0.990	131
D009	1.52	3	0.915	2594	PT49	0.33	3	0.990	133
D004	2.40	3	0.925	1565	PT50	163.78	5	0.530	8562*
PT5	5.02	3	0.895	3901	D11B	0.99	3	0.975	932
D08	11.91	4	0.850	3522	PT51	164.76	5	0.530	8562*
PT6	17.93	5	0.804	4564					

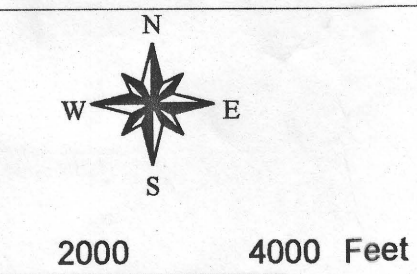
\* ADOPTED FLOW, UPSTREAM FLOW CONTROLS  
AREA IN SQUARE MILES / FLOW IN CFS

# DUCK CREEK / BLUE DIAMOND WASHES FIS RESTUDY

Existing Condition Drainage Map

FIGURE 6B

LEGEND	
D010	Basin Name
BD2	Bridge / Culvert ID
P13	HEC-1 Combination Point
DVBD2	Diversion / Inflow Hydrograph
MAIN	Street Name
P	HEC-1 Combination Point
●	Bridge / Culvert Location
→	Inflow Hydrograph
→	Flow Direction
---	Wash / Channel Centerline
---	Streets
---	Contour Line
---	Index Contour
○	Detention Basin
○	HEC-1 Drainage Basin Boundary



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1533 South Rainbow Blvd., Las Vegas, NV 89146

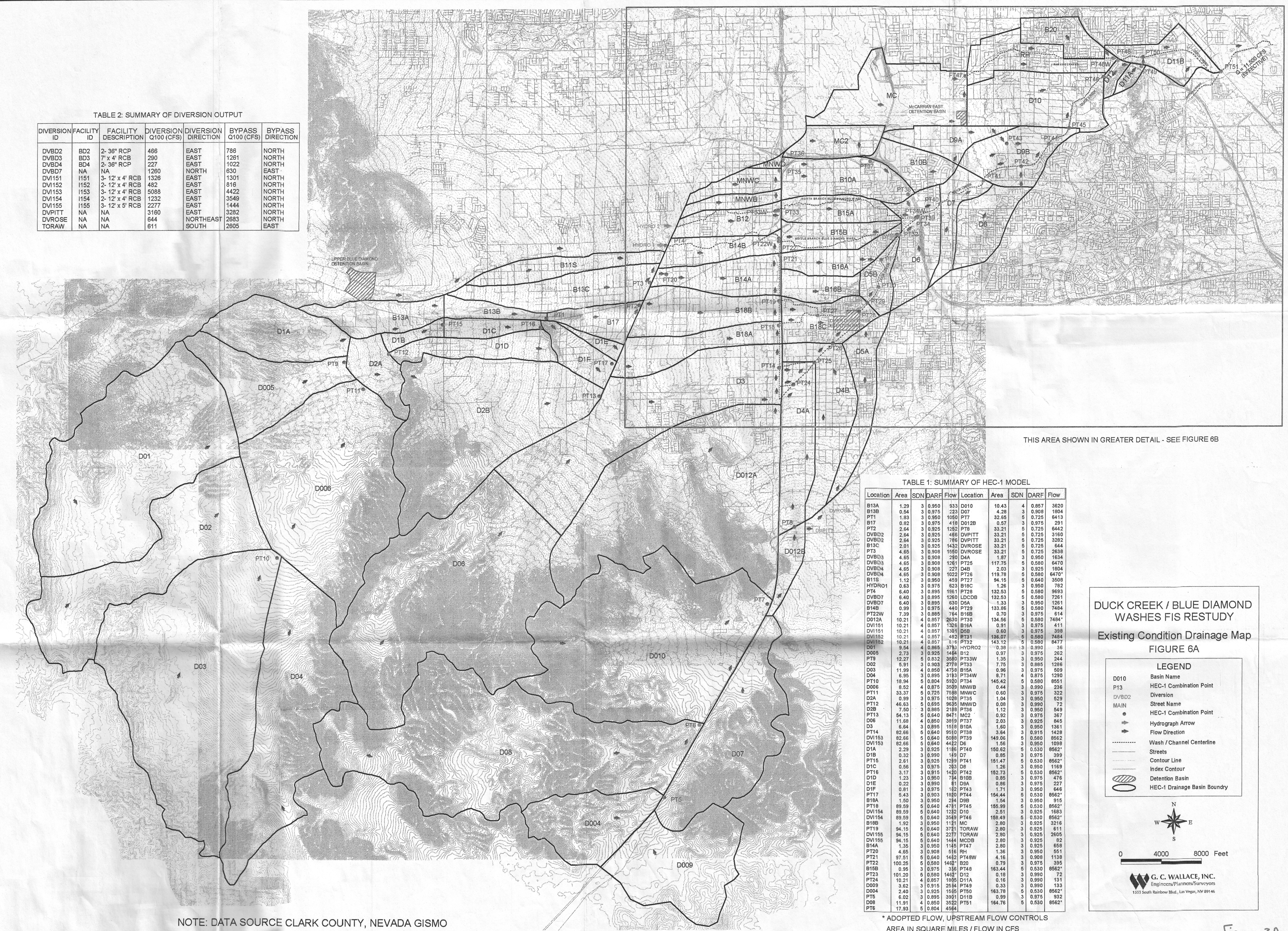
Figure 3B

NOTE: DATA SOURCE CLARK COUNTY, NEVADA GISMO



TABLE 2: SUMMARY OF DIVERSION OUTPUT

DIVERSION ID	FACILITY ID	FACILITY DESCRIPTION	DIVERSION Q100 (CFS)	DIVERSION DIRECTION	BYPASS Q100 (CFS)	BYPASS DIRECTION
DVB02	BD2	2-36" RCP	466	EAST	786	NORTH
DVB03	BD3	7' x 4' RCB	290	EAST	1281	NORTH
DVB04	BD4	2-36" RCP	227	EAST	1022	NORTH
DVB07	NA	NA	1260	NORTH	630	EAST
DV151	I151	3-12' x 4' RCB	1326	EAST	1301	NORTH
DV152	I152	2-12' x 4' RCB	482	EAST	816	NORTH
DV153	I153	3-12' x 4' RCB	5088	EAST	4422	NORTH
DV154	I154	2-12' x 4' RCB	1232	EAST	3549	NORTH
DV155	I155	3-12' x 5' RCB	2277	EAST	1444	NORTH
DVPITT	NA	NA	3160	EAST	3282	NORTH
DVROSE	NA	NA	644	NORTHEAST	2683	NORTH
TORAW	NA	NA	611	SOUTH	2605	EAST



THIS AREA SHOWN IN GREATER DETAIL - SEE FIGURE 6B

TABLE 1: SUMMARY OF HEC-1 MODEL

Location	Area	SDN	DARR	Flow	Location	Area	SDN	DARR	Flow
B13A	1.29	3	0.950	333	D010	10.43	4	0.957	3620
B13B	0.64	3	0.975	223	D07	4.28	3	0.908	1804
PT1	1.83	3	0.950	1050	PT7	32.65	5	0.725	6413
B17	0.82	3	0.975	410	D012B	0.57	3	0.975	331
PT2	2.64	3	0.925	1282	PT8	33.21	5	0.725	6442
DVB02	2.64	3	0.925	466	DVPITT	33.21	5	0.725	3160
DVB03	2.64	3	0.925	786	DVPITT	33.21	5	0.725	3392
B13C	2.01	3	0.925	1432	DVROSE	33.21	5	0.725	644
DVB03	4.65	3	0.908	1580	DVROSE	33.21	5	0.725	2638
DVB03	4.65	3	0.908	280	D4A	1.87	3	0.950	1634
DVB03	4.65	3	0.908	1261	PT25	117.75	5	0.580	6470
DVB04	4.65	3	0.908	227	D4B	2.03	3	0.925	1804
DVB04	4.65	3	0.908	1022	PT26	119.78	5	0.580	6470
B11S	1.12	3	0.950	458	PT27	94.15	5	0.640	3508
HYDR01	0.63	3	0.975	623	B18C	1.26	3	0.950	782
PT4	6.40	3	0.895	1561	PT28	132.53	5	0.580	6993
DVB07	6.40	3	0.895	1260	LDCCB	132.53	5	0.580	7261
DVB07	6.40	3	0.895	830	D4A	1.33	3	0.950	1281
B14B	0.99	3	0.975	440	PT29	133.86	5	0.580	7484
PT22W	7.39	3	0.895	764	B18B	0.70	3	0.975	514
D012A	10.21	4	0.857	2630	PT30	134.66	5	0.580	7484
DV151	10.21	4	0.857	1326	B16A	0.91	3	0.975	411
DV151	10.21	4	0.857	1301	D8B	0.60	3	0.975	388
DV152	10.21	4	0.857	482	PT31	136.07	5	0.580	7484
DV152	10.21	4	0.857	1114	PT32	143.12	5	0.580	8477
D01	9.54	4	0.885	3735	HYDR02	0.38	3	0.950	36
D005	2.73	3	0.925	1458	B12	0.97	3	0.975	262
PT5	12.27	5	0.332	3650	PT33W	1.35	3	0.950	244
D02	5.91	3	0.903	2778	PT33	7.75	3	0.885	1286
D03	11.99	4	0.850	4758	B15A	0.96	3	0.975	509
D03	6.95	3	0.895	3183	PT34W	8.71	4	0.875	1290
PT10	18.94	5	0.804	5920	PT34	145.42	5	0.580	8561
D006	8.52	4	0.875	3559	MNWB	0.44	3	0.950	236
PT11	33.37	5	0.725	7598	MNWC	0.60	3	0.975	222
D2A	0.99	3	0.975	1028	PT35	1.04	3	0.950	529
PT12	46.63	5	0.695	3635	MNWD	0.08	3	0.990	72
D2B	7.50	3	0.885	2198	PT36	1.12	3	0.950	549
PT13	84.13	5	0.640	3471	MC2	0.92	3	0.975	367
D06	11.88	4	0.850	3910	PT37	2.03	3	0.925	845
D3	6.64	3	0.895	1518	B10A	1.80	3	0.950	1361
PT14	82.86	5	0.640	3510	PT38	3.84	3	0.915	1428
DV153	82.86	5	0.640	5039	PT39	149.06	5	0.580	8562
DV153	82.86	5	0.640	4422	D6	1.56	3	0.950	1098
D1A	2.29	3	0.925	1180	PT40	156.62	5	0.530	8562
D1B	0.32	3	0.990	155	D7	0.95	3	0.975	399
PT15	2.61	3	0.925	1299	PT41	151.47	5	0.530	8562
D1C	0.66	3	0.975	230	D8	1.26	3	0.950	1138
PT16	3.17	3	0.915	1420	PT42	152.73	5	0.530	8562
D1D	1.23	3	0.950	784	B10B	0.85	3	0.975	476
D1E	0.22	3	0.990	81	D9A	0.99	3	0.975	527
D1F	0.81	3	0.975	182	PT43	1.71	3	0.950	646
PT17	5.43	3	0.903	1920	PT44	154.44	5	0.530	8562
B18A	1.50	3	0.950	234	D9B	1.54	3	0.950	915
PT18	89.59	5	0.640	4781	PT45	155.99	5	0.530	8562
DV154	89.59	5	0.640	1332	D10	2.91	3	0.925	1693
DV154	89.59	5	0.640	3549	PT46	158.49	5	0.530	8562
B18B	1.92	3	0.950	1121	MC	2.80	3	0.925	3216
PT19	94.15	5	0.640	3721	TORAW	2.90	3	0.925	611
DV155	94.15	5	0.640	2277	TORAW	2.90	3	0.925	2605
DV155	94.15	5	0.640	1444	MCDB	2.80	3	0.925	92
B14A	1.35	3	0.950	1145	PT47	2.80	3	0.925	658
PT20	4.65	3	0.908	516	MC	1.36	3	0.950	551
PT21	97.51	5	0.640	1452	PT48W	4.16	3	0.908	1138
PT22	100.25	5	0.580	1462	B20	0.79	3	0.975	395
B15B	0.95	3	0.975	336	PT48	163.44	5	0.530	8562
PT23	101.20	5	0.580	1462	D12	0.19	3	0.990	72
PT24	10.21	4	0.857	1895	D11A	0.16	3	0.990	131
D009	3.62	3	0.915	2534	PT49	0.33	3	0.990	133
D004	2.40	3	0.925	1555	PT50	163.78	5	0.530	8562
PT5	6.02	3	0.895	3921	D11B	0.99	3	0.975	532
D08	11.91	4	0.850	3520	PT51	164.78	5	0.530	8562
PT6	17.93	5	0.804	4564					

\* ADOPTED FLOW, UPSTREAM FLOW CONTROLS  
AREA IN SQUARE MILES / FLOW IN CFS

DUCK CREEK / BLUE DIAMOND WASHES FIS RESTUDY

Existing Condition Drainage Map

FIGURE 6A

LEGEND

- D010 Basin Name
- PT3 HEC-1 Combination Point
- DVB02 Diversion
- MAIN Street Name
- HEC-1 Combination Point
- Hydrograph Arrow
- Flow Direction
- Wash / Channel Centerline
- Streets
- Contour Line
- Index Contour
- Detention Basin
- HEC-1 Drainage Basin Boundary

0 4000 8000 Feet

G. C. WALLACE, INC.  
Engineers/Planners/Surveyors  
1331 South Rainbow Blvd., Las Vegas, NV 89146

Figure 3A



#### Proposed Facilities Across The Subject Site

A proposed 15-foot wide, 4.5-foot depth, 2:1 side sloped Gabion Channel (CCRFCF facility BD01 0012) will convey flows, from (CCRFCF local facility IS15 0233), easterly across the site and exit through two proposed 6' x 5' Reinforced Concrete Box Culverts at Las Vegas Boulevard (CCRFCF facility BD01 0010).

Five additional 9' x 6' Reinforced Concrete Box Culverts are proposed to be added to the existing unidentified 3-12' x 4' Reinforced Box Culverts at Cactus and I-15 (CCRFCF facility DCWA 1429) to deliver flows originating west of Interstate I-15. This flow is then proposed to be carried by a 40-foot width, 7-foot depth, 2:1 side sloped Gabion Channel (CCRFCF) facility DCWA 1378) that transitions downstream to a depth of 7.5 foot depth (CCRFCF) facility DCWA 1362). This flow will exit through one existing 12' x 6' Reinforced Box Culvert and three proposed 12' x 6' Reinforced Concrete Box Culverts (CCRFCF facility DCWA 1358).

It is our intention that the proposed arch culvert storm drains will replace or supplement the proposed MPU facilities. CCRFCF facility BD01 0012 will be replaced by our north box facility. The north arch culvert is designed to convey 5,080 cfs. Currently, the proposed CCRFCF facility at this location is planned to convey only 500 cfs.

The south box facility is designed to convey a combined flow of 4,343 cfs. Currently the proposed CCRFCF facility at this location is planned to convey only 3,260 cfs. Our facility is more than adequately sized for the future condition of 3,260 cfs. A portion of the arch system will have to be extended west to I-15 in the future (by CCRFCF or others). Cactus Avenue right-of-way will be available for this extension. The flow being conveyed by these Master Plan facilities will eventually reach the existing Lower Duck Detention Basin. Once this study is approved, a CCRFCF Master Plan Update revision will be required to show the revised conveyance facilities and alignments. It should be noted that by constructing the proposed facilities, Greenpark would have provided an

important part of the master planned infrastructure at no cost to CCRFCD or the Public. The proposed development of this site is in compliance with the Clark County Regional Flood Control District Master Plan Update (Ref 5).

## 2.5 Previous Drainage Studies

The offsite drainage areas have been previously analyzed by the approved "*Pittman Wash F.I.S. Restudy*" (Ref 7), and the recently submitted FIS study "*Duck Creek / Blue Diamond Washes FIS Restudy*" by G.C. Wallace (Ref 8). The onsite area was analyzed in the approved "Duck Creek Hydrologic Unit F.I.S. Restudy" (Ref 9). Both studies have been reviewed and conceptually agreed with for this study.

### 3.0 PROPOSED DRAINAGE FACILITIES

#### 3.1 General Description

The purpose of this study is to support the design of storm water conveyance facilities that are needed to mitigate the negative impact on the subject site caused by the significant flows from offsite basins. It is expected that these proposed facilities will provide the same function as the facilities identified in the MPU. This study is not intended to address any onsite development activity beyond the previously described storm water conveyance facilities, nor is it intended to support perimeter street improvements. Further technical studies will be needed to support those improvements.

##### North Box Drainage Improvements

The north box flows will be collected at Parvin Street. The natural topography is such that the existing flows will be centered at the intersection of Parvin Street and Jo Rae Avenue. This flow will be allowed to weir over the eastern curb into two parallel collector channels. Parvin street is the exact location of the G.C. Wallace FIS study cross section 26000. The weir places the flow at critical while the existing section is subcritical. This results in a reduction of the water surface elevation at this point.

FIS Section WSE = 2266.46

Greenpark Weir WSE = 2266.22

The flows that weir into each of the collection channels were calculated to be 2492 cfs in the south channel and 2588 cfs in the north channel. The collection channels merge together after both flows have been turned east. A wall separates the flows until the flows are going in the same direction. A review of the collection channels hydraulics shows that the channels HGL and EGL are contained within the channel and are lower than the

weir crest in Parvin Street. The hydraulics through the collector channels was modeled using HEC-RAS. This analysis was compared to two other methodologies to determine accuracy. The first was a weir calculation at the confluence. This calculation showed a weir height of 2263.70. Another check was performed using the spillway analysis as outlined in the Bureau of Reclamation's *Design of Small Dams* (Ref 10). The calculations contained within the book outline how to design a side channel spillway. The crux of the design procedure involves determining the depth at a control section (20-00) and calculating the losses as you move up the collection channel. We have included the calculations and a copy of the pertinent section from the *Design of Small Dams*. The result of the calculations is a water surface that is approximately 1 foot higher than calculated by the HEC-RAS. The channel as designed will contain the higher water surface elevation with applicable freeboard. The water surface at the low point in Parvin Street is higher than the TC that was used as the weir crest. This will not effect the operation of the weir, as the channel flow depth is less than  $\frac{2}{3}$  of the crest height (per *Design of Small Dams*). These calculations are intended to be a back up to the HEC-RAS analysis. The numbers from the HEC-RAS analysis are still being used to set the HGL's in the collection system

The north box continues east toward Las Vegas Boulevard. Approximately 500 east of the confluence the flow will be collected in two arch-culverts. These arch culverts are then transitioned into a set of NDOT standard box culverts under Las Vegas Boulevard. The transition will end outside of the NDOT right-of-way and the flow will be released in the historical flow path (Please table in the appendix for all cross section comparisons).

#### South Box Drainage Improvements

The South Box Drainage Improvements generally applies to the proposed facilities that run west to east along Cactus Avenue and then turn north along the western edge of the Las Vegas Boulevard right of way. The South Box drainage improvements consist of two parallel drainage

systems that we will identify as "South Box A" and "South Box B" for discussion purposes within this study.

"South Box A" consists of proposed facilities that connect to two existing storm drain facilities under I-15 adjacent to the southern portion of the subject site. The southern most existing facility under I-15 consists of three 12' x 4' RCB's. The proposed improvements consists of extending the three 12' x 4' RCB's already in place under Interstate I-15. The existing boxes will be extended (approx. 170 ft) to the limits of the (NDOT) right-of-way. Once outside of the NDOT right-of-way the boxes will be transitioned, over a length of 160 ft, to a 16' x 7' arch culvert. The alignment of the proposed facility will be curved to get the proposed facility within the future Cactus Avenue right of way.

This arch culvert will be joined by another arch culvert (10' x 6' arch culvert). This 10' x 6' arch culvert that connects into the 16' x 7' arch culvert will convey flows from the other existing drainage facility under I-15 that is located approximately 1000 linear feet north of Cactus Avenue. The existing facility consists of two 12' x 4' RCB already in place under Interstate I-15. The proposed improvements include extending the existing boxes (approx. 107 ft) to the limits of the NDOT right-of-way. Once outside of the NDOT right-of-way the boxes will be transitioned, over a length of 112 ft, to a 10' x 6' arch culvert. This 10' x 6' arch culvert will join the 16' x 7' arch culvert with an entrance angle of less than 12 degrees. South box A will follow along Cactus Avenue and will curve north along side of Las Vegas Boulevard crossing. At Las Vegas Boulevard the 16' x 7' arch culvert will be transitioned into three 7' x 5' RCB. The transition will end outside of the NDOT right-of-way. The three 7' x 5' RCB will cross under Las Vegas Boulevard and the flow will be released in the historical flow path. Grouted rip-rap and cut-off walls have been placed at the outlet (See appendix for rip-rap sizing).

South Box B consists of a 24' x 8' arch culvert. The flow that enters this culvert will be from the Cactus Avenue Collector. A large, natural swale to the south is the obvious path of the offsite flows. This existing swale can be seen on aerial photographs (See Figure 11, Aerial Photo) and on

field visits. The natural swale fans out and becomes less defined as it approaches Cactus Avenue. Besides viewing aeriels and performing field visits, we prepared a normal depth calculation that showed that flow will be contained in the swale and will impact the proposed collection facility. The Cactus Avenue Collector will collect the 2,543-cfs that impacts Cactus Avenue from the south. The flows from the south will flow in its existing condition across an out parcel at the intersection of Cactus Avenue and Las Vegas Boulevard and into the concrete lined collection channel. The channel will route the flow east into a 24' x 8' arch culvert. South box B continues north until it reaches an existing Las Vegas Boulevard crossing. At Las Vegas Boulevard the 24' x 8' arch culvert will be transitioned into three 10' x 5' RCB. The transition will end outside of the NDOT right-of-way. The three 10' x 5' RCB will cross under Las Vegas Boulevard and the flow will be released in the historical flow path (Please table in the appendix for all cross section comparisons).

### 3.2 Compliance with Regulations and Adopted Plans

The proposed storm facilities will alter two adopted plans. These plans include the CCRFCD Las Vegas Master Plan Update and the FEMA flood plain map as proposed in accompanying FIS study.

The changes to the Las Vegas Master Plan consist of building the majority of the proposed future facilities that cross the site. The facilities will be a different design than was previously anticipated and along a slightly different alignment. It should be noted that by constructing the proposed facilities, Greenpark would have provided an important part of the master planned infrastructure at no cost to CCRFCD or the public. The proposed development of this site is in compliance with the Clark County Regional Flood Control District Master Plan Update (Ref 5).

The FEMA floodplain will be remapped as a part of the CCRFCD Duck Creek FIS restudy. Once the CLOMR is approved our client will construct the facilities and apply for a LOMR. It is anticipated that the LOMR will change the flood zone designation across the entire site to a zone "x" with

a designation, which shows the flows contained within the storm drain facilities.

### 3.3 Facility Design Calculations

A WSPGW analysis has been performed for all underground storm drains within the site and all open channel analysis has been performed using HEC-RAS. Each of the models input and output data can be found in the appendix. The “North Box”, which carries 5080 cfs, delivers flow from I-Parvin Street to Las Vegas Blvd. The north box system consists of a HEC-RAS analysis for the collector channel and open channel, WSPGW for the portion of the system that is underground, and HEC-RAS for the outlet of the system and the tie in to the FIS study.

The run for “South Box A”, also delivers flow from I-15 to Las Vegas Blvd., but across the southern portion of the site. This system carries an initial flow of 1336 cfs, and collects 482 cfs from the “South Box A Lateral”. This combined flow of 1808 cfs is contained through the system to the outlet. The “South Box B” system (24’ x 8’ arch) is structurally connected (no flow exchange between systems) to “South Box A”, which leads to a common outlet for the two systems.

The run for “South Box A Lateral” conveys 482 cfs. The outlet water surface elevation for the South Box A Lateral was determined from the “South Box A” results.

The “South Box B” analysis conveys flows impacting the site from the south. The flow is completely contained inside the structure after all of the flow is introduced. The outlet water surface was determined by using the HEC-RAS and iterating to a WSE that was consistent with both analyses.



#### 4.0

### CONCLUSIONS AND RECOMMENDATIONS

1. The major offsite flows impacting the project site can be safely conveyed around and through the subject site via the proposed storm drain facilities and open channels. The proposed storm drains will collect flow from the culvert crossings under I-15, and the flows that impact Cactus Road from the south (flow comes from under Lake Mead Boulevard). All the flows will be collected into the proposed arch culverts and be conveyed to Las Vegas Boulevard. The flows will be released on the east side of Las Vegas Boulevard in the approximate location of the historic flows.
2. Hydrology was based on the HEC-1 computer model as determined in the G.C. Wallace FIS study (Volume 1) and on generally accepted engineering practices in accordance with the Drainage Criteria Manual.
3. A considerable portion of the site is located within a Special Flood Hazard Zone. It is expected that the limits of all the flood zones on the site will be substantially revised as a result of the pending F.I.S. restudy. The F.I.S. study will also include a CLOMR for the proposed arch-culverts contained within the study.
4. The development of this site is in compliance with the Clark County Regional Flood Control District Flood Control Master Plan Update.
5. Additional technical drainage studies will be required to support the design of any onsite development and perimeter streets. Also numerous easements and dedications will have to be obtained prior to approval of these improvement plans.
6. Historically, the flows through the site have been collected at Las Vegas Boulevard. Due to elevated nature of Las Vegas Boulevard the flow would have had to pond up to considerable depth before weiring over. The proposed storm drain alleviated this condition by conveying all of the considerable offsite flows under Las Vegas Boulevard and releasing them back to the historical flow path.

## 5.0

### REFERENCES

1. Department of Public Works, Clark County, Nevada, "Improvement Standards," 1993
2. U.S. Department of Agriculture, Soils Conservation Services, Technical Release 55, "Urban Hydrology for Small Watersheds," Washington, D.C. January 1975,
3. U.S. Department of Agriculture, Soils Conservation Services, "Soil Survey of Las Vegas Valley Area," July 1985
4. U.S. Army Corps of Engineers, "HEC-1 Flood Hydrograph Package," September 1981, Revised January 1985
5. PBS&J and VTN Nevada, Flood Control Master Plan Update of The Las Vegas Valley," February 1997.
6. Clark County Regional Flood Control District, "Hydrologic Criteria and Drainage Design Manual," October 1990
7. Nimbus, "Pittman Was Flood Insurance Study Restudy," June 1995
8. G.C. Wallace "Duck Creek / Blue Diamond Washes FIS Restudy" March 2002
9. Michael Baker Jr., Inc., "Duck Creek Hydrologic Unit," August 1992

10. Bureau of Reclamation, "Design of Small Dams," 1987
11. CivilDesign, Inc., "User's Manual – Program Package WSPGW," 1994
12. U.S. Department of Commerce, "Design of Riprap Revetment," March 1989

□ □  
 HEC1 S/N: 1333000362      HMVersion: 6.40      Data File: duck5.txt

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*****
*
* FLOOD HYDROGRAPH PACKAGE (HEC-1)
*   SEPTEMBER 1990
*   VERSION 4.0
*
* RUN DATE 03/21/2002 TIME 09:09:37
*
*****
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*****
*
* U.S. ARMY CORPS OF ENGI
* HYDROLOGIC ENGINEERING
*   609 SECOND STREET
*   DAVIS, CALIFORNIA 95
*   (916) 756-1104
*
*****
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X   X XXXXXXXX XXXXX      X
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X   X X      X      X    X
XXXXXXX XXXX  X      XXXXX X
X   X X      X      X    X
X   X X      X      X    X
X   X XXXXXXXX XXXXX      XXX

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::::::::::::::::::::::::::::::::::::
::::::::::::::::::::::::::::::::::::
:::
::: Full Microcomputer Implementation :::
::: by :::
::: Haestad Methods, Inc. :::
:::
::::::::::::::::::::::::::::::::::::
::::::::::::::::::::::::::::::::::::

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37 Brookside Road \* Waterbury, Connecticut 06708 \* (203) 755-1666

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUC  
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSIO  
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,  
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL    LOSS RATE:GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

□ □

## HEC-1 INPUT

PAGE 1

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

## \*DIAGRAM

\*\*\* FREE \*\*\*

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1 ID DUCK CREEK/BLUE DIAMOND WASHES FIS RESTUDY
2 ID "G.C. WALLACE, INC. BY MJL"
3 ID DUCK5.XLS
4 ID "MODEL BASED FROM HEC-1 FILE, BDE100UN, FROM 1991 LAS VEGAS VALLEY FIS"
5 ID "BY JAMES M. MONTGOMERY CONSULTING ENGINEERS, INC."
6 ID UPDATED PRECIPITATION VALUES
7 ID % IMPERVIOUS ADDED TO LS CARD FOR DEVELOPMENT SINCE 1990
8 ID ADDITIONAL DIVERSIONS ADDED
9 ID DESIGN STORM = 100-YEAR 6-HOUR STORM
10 ID STORM DISTRIBUTION = SDN5
    *
11 ID JR CARDS CONTAIN DARFS BASED ON THE FOLLOWING VALUES:
12 ID
13 ID AREA (SQUARE MILES) DARF
14 ID 12 - 16 0.832
15 ID 16 - 20 0.804
16 ID 20 - 30 0.765
17 ID 30 - 40 0.725
18 ID 40 - 50 0.695
19 ID 50 -100 0.640
20 ID 100 - 150 0.580
21 ID 150 - 200 0.530
22 ID
23 IT 5 0 0 120
24 IO 5 0 0
25 JR PREC 0.832 0.804 0.765 0.725 0.695 0.640 0.580 0.53
    *

26 KK B13A
27 BA 1.290
28 FB 3.14
29 PC 0.000 0.020 0.059 0.080 0.110 0.144 0.150 0.160 0.168 0.171
30 PC 0.180 0.182 0.187 0.190 0.197 0.202 0.210 0.220 0.230 0.241
31 PC 0.250 0.259 0.265 0.280 0.290 0.300 0.305 0.309 0.310 0.317
32 PC 0.321 0.327 0.333 0.346 0.361 0.381 0.408 0.430 0.477 0.514
33 PC 0.561 0.630 0.710 0.720 0.731 0.752 0.779 0.790 0.795 0.804
34 PC 0.810 0.820 0.826 0.840 0.859 0.889 0.910 0.938 0.966 0.970
35 PC 0.974 0.979 0.981 0.983 0.985 0.989 0.990 0.992 0.993 0.996
36 PC 0.997 0.999 1.000
37 LS 0 88.0
38 UD 0.680
    *

39 KK RB13A
40 KM ROUTE B13A TO PT1
41 RM 7 0.64 0.1
    *

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□ □

## HEC-1 INPUT

PAGE 2

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
42	KK B13B
43	BA 0.538
44	PB 2.96
45	LS 0 88.0
46	UD 1.440
	*
47	KK PT1
48	KM COMBINE BASINS B13A AND B13B
49	HC 2
	*
50	KK RPT1
51	KM ROUTE PT1 TO PT2
52	RM 3 0.28 0.15
	*
53	KK B17
54	BA 0.816
55	PB 2.80
56	LS 0 88.0
57	UD 0.940
	*
58	KK PT2
59	KM COMBINE PT1 AND BASIN B17
60	HC 2
	*
61	KK DVBD2
62	KM "DIVERSION EAST THRU 2-36"" RCP'S AT UPRR LOCATED 300 FT SOUTH OF BLUE DIAMON
63	DT DVBD2
64	DI 0 121 500 1000 1500 2000
65	DQ 0 121 240 391 540 690
	*
66	KK RPT2
67	KM ROUTE BYPASS TO PT3
68	RD 2000 0.008 0.03 0 TRAP 35 3
	*
69	KK B13C
70	BA 2.005
71	PB 2.82
72	LS 0 88.1
73	UD 0.510
	*
74	KK PT3
75	KM COMBINE BYPASS FROM DVBD2 AND BASIN B13C
76	HC 2
	*

## HEC-1 INPUT

PAGE 3

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LINE      ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

77      KK  DVBD3
78      KM  DIVERSION EAST THRU 8'x4' RCB AT UPRR LOCATED 1700 FT NORTH OF BLUE DIAMOND
79      DT  DVBD3
80      DI      0      238      500      1000      1500      2000
81      DQ      0      238      252      271      288      304
      *

82      KK  RPT3A
83      KM  ROUTE BYPASS FROM DVBD3 NORTH ALONG WEST SIDE OF UPRR TO UPRR CULVERT BD4
84      KM  MUSKINGUM ROUTING UNSTABLE
85      RD  1200  0.008  0.03      0      TRAP      35      3
      *

86      KK  DVBD4
87      KM  "DIVERSION EAST THRU 2-36"" RCP'S AT UPRR LOCATED 3000 FT NORTH OF BLUE DIAMO
88      DT  DVBD4
89      DI      0      97      500      1000      1500      2000
90      DQ      0      97      149      202      252      296
      *

91      KK  RPT3B
92      KM  ROUTE BYPASS FROM DVBD4 NORTH ALONG WEST SIDE OF UPRR TO UPRR BRIDGE
93      RM      1  0.119  0.15
      *

94      KK  B11S
95      KM  SOUTHERN PORTION OF BASIN B11 TIES INTO HYDRO1 BOUNDARY AREA
96      BA  1.121
97      PB  2.82
98      LS      0      78.0
99      UD  0.580
      *

100     KK  HYDRO1
101     KM  INFLOW HYDROGRAPH
102     KM  "REFERENCED FROM ""LOMR FOR AREA DOWNSTREAM OF UPPER BLUE DIAMOND D.B.""
103     KM  "SEE COMBINATION POINT C-5, DARF RATIO 0.98, OF HEC-1 FROM REFERENCED LOMR"
104     BA  0.630
105     QI      0      0      0      0      0      0      0      0      0
106     QI      0      0      0      0      0      0      0      0      0
107     QI      0      0      1      7      14      29      45      58      69      76
108     QI      82      86      90      94      99      100      101      100      100      100
109     QI      102      109      126      161      223      318      430      538      604      623
110     QI      607      569      525      480      437      397      363      335      316      306
111     QI      304      310      321      333      339      332      314      289      264      239
112     QI      217      198      182      169      159      150      143      137      131      127
113     QI      122      118      114      110      107      104      102      101      100      99
114     QI      98      98      97      97      97      97      97      96      96      96
115     QI      96      96      96      97      96      96      96      96      96      96
116     QI      96      96      96      96      96      96      96      96      96      96
117     QI      96      96      96      96      96      96      96      96      96      96
118     QI      96      96      96      96      96      96      96      96      96      96
119     QI      96      96      96      96      96      96      96      96      96      96
120     QI      96      96      96      96      96      95      95      95      95      95

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## HEC-1 INPUT

PAGE 4

LINE	ID	1	2	3	4	5	6	7	8	9	10
121	QI	95	95	95	95	95	95	95	95	95	95
122	QI	95	95	95	95	95	95	95	95	95	95
123	QI	95	95	95	95	95	95	95	95	95	95
124	QI	95	95	95	95	95	95	95	95	95	95
125	QI	95	95	95	95	95	95	95	95	95	95
126	QI	95	95	95	95	95	95	95	95	95	95
127	QI	95	95	95	95	95	95	95	95	95	95
128	QI	95	95	94	94	94	94	94	94	94	94
129	QI	94	94	94	94	94	94	94	94	94	94
130	QI	94	94	94	94	94	94	94	94	94	94
131	QI	94	94	94	94	94	94	94	94	94	94
132	QI	94	94	94	94	94	94	94	94	94	94
133	QI	94	94	94	94	94	94	94	94	94	94
134	QI	94	94	94	94	94	94	94	94	94	94
	*										
135	KK	PT4									
136	KM	"COMBINE BASIN B11S, HYDRO2, AND BYPASS FROM DVBD4"									
137	KM	SEE COMBINATION POINT C-5 OF HEC-1 FROM REFERENCED LOMR									
138	KM	"PT C-5 REFERENCED FROM ""LOMR FOR AREA DOWNSTREAM OF UPPER BLUE DIAMOND D.B.									
139	HC	3									
	*										
140	KK	RPT4A									
141	KM	ROUTE PT4 TO DIVERSION DVBD7									
142	RM	3 0.22 0.15									
	*										
143	KK	DVBD7									
144	KM	DIVERSION NORTH TO NORTH BRANCH BLUE DIAMOND WASH									
145	KM	DIVERSION LOCATED NEAR INTERSECTION OF HINSON AND WINDMILL									
146	KM	"REFERENCED FROM BLUE DIAMOND/I-15 INTERCHANGE REPORT, BY PBS&J, JAN 2001"									
147	DT	DVBD7									
148	DI	0 1200 1500 1800 2100 2400 2700 3000									
149	DQ	0 800 1000 1200 1400 1600 1800 2000									
	*										
150	KK	RPT4B									
151	KM	ROUTE BYPASS TO PT22W									
152	RM	1 0.11 0.15									
	*										
153	KK	B14B									
154	BA	0.992									
155	PB	2.77									
156	LS	0 73.4 9.0									
157	UD	0.42									
	*										
158	KK	PT22W									
159	KM	COMBINE PT4 AND BASIN B14B									
160	HC	2									
	*										



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## HEC-1 INPUT

PAGE 5

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
161	KK D012A
162	BA 10.211
163	PB 2.95
164	LS 0 83.0
165	UD 1.320
	*
166	KK RD012A
167	KM ROUTE BASIN D012A TO I15 CULVERT 3 - 12'x4' RCB'S (DCWA 1429)
168	KM MUSKINGUM ROUTING UNSTABLE
169	RD 750 0.007 0.030 0 TRAP 65 10
	*
170	KK DVI151
171	KM DIVERSION EAST THRU I15 CULVERT 3 - 12'x4' RCB'S (DCWA 1429)
172	DT DVI151
173	DI 0 993 1305 1778 2400 3146
174	DQ 0 992 1100 1200 1298 1389
	*
175	KK RI151
176	KM ROUTE BYPASS FROM 3 - 12'x4' RCB'S TO I15 BRIDGE 24'x4' (IS15 0199)
177	RM 1 0.071 0.150
	*
178	KK DVI152
179	KM DIVERSION EAST THRU I15 BRIDGE 24'x4' (IS15 0199)
180	DT DVI152
181	DI 0 128 466 929 1506 2268 3133 3815
182	DQ 0 47 190 362 550 733 896 1040
	*
183	KK RI152
184	KM ROUTE BYPASS FROM I15 BRIDGE 24'x4' TO I15 BRIDGE 36'x4' (IS15 0233)
185	RM 1 0.123 0.150
	*
186	KK D01
187	BA 9.536
188	PB 3.42
189	LS 0 83.0
190	UD 1.000
	*
191	KK RD01
192	KM ROUTE BASIN D01 TO PT9
193	RM 3 0.64 0.150
	*

## HEC-1 INPUT

LINE	ID.	1	2	3	4	5	6	7	8	9	10
194	KK	D005									
195	BA	2.732									
196	PB	3.27									
197	LS	0	82.0								
198	UD	0.750									
	*										
199	KK	PT9									
200	KM	COMBINE BASINS D01 AND D005									
201	HC	2									
	*										
202	KK	RD005									
203	KM	ROUTE BASIN D005 TO PT12									
204	RM	1 0.2 0.150									
	*										
205	KK	D02									
206	BA	5.915									
207	PB	3.44									
208	LS	0	83.0								
209	UD	1.030									
	*										
210	KK	RD02									
211	KM	ROUTE BASIN D02 TO PT11									
212	RM	3 0.62 0.150									
	*										
213	KK	D03									
214	BA	11.991									
215	PB	3.42									
216	LS	0	83.0								
217	UD	0.960									
	*										
218	KK	D04									
219	BA	6.950									
220	PB	3.37									
221	LS	0	84.0								
222	UD	1.060									
	*										
223	KK	PT10									
224	KM	COMBINE BASINS D03 AND D04									
225	HC	2									
	*										
226	KK	RPT10									
227	KM	ROUTE PT10 TO PT11									
228	RM	3 0.82 0.150									
	*										

□ □

## HEC-1 INPUT

PAGE 7

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
229	KK D006
230	BA 8.518
231	PB 3.24
232	LS 0 83.0
233	UD 0.850
	*
234	KK PT11
235	KM "COMBINE PT10, BASINS D002 AND D006"
236	HC 3
	*
237	KK RPT11
238	KM ROUTE PT11 TO PT12
239	RM 1 0.18 0.150
	*
240	KK D2A
241	BA 0.988
242	PB 3.16
243	LS 0 84.0
244	UD 0.280
	*
245	KK PT12
246	KM "COMBINE PT9, PT11 AND BASIN D2A"
247	HC 3
	*
248	KK RPT12
249	KM ROUTE PT12 TO PT13
250	RM 3 0.83 0.150
	*
251	KK D2B
252	BA 7.503
253	PB 3.06
254	LS 0 80.1
255	UD 1.180
	*
256	KK PT13
257	KM COMBINE PT12 AND BASIN D2B
258	HC 2
	*
259	KK RPT13
260	KM ROUTE PT13 TO PT14
261	RM 4 0.9 0.100
	*

## HEC-1 INPUT

PAGE 8

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
262	KK D06
263	BA 11.677
264	PB 3.21
265	LS 0 84.0
266	UD 1.190
	*
267	KK RD06
268	KM ROUTE BASIN D06 TO PT14
269	RM 3 0.78 0.150
	*
270	KK D3
271	BA 6.637
272	PB 2.90
273	LS 0 77.0
274	UD 1.130
	*
275	KK PT14
276	KM "COMBINE BASINS D3, D06, PT13, AND BYPASS FROM I-15 CULVERT, I152 (IS15 0199)
277	HC 4
	*
278	KK DVI153
279	KM DIVERSION EAST THRU I15 BRIDGE 36'x4' (IS15 0233)
280	DT DVI153
281	DI 1910 3220 6620 8040 8770 9390 10030 10740
282	DQ 1190 1450 3280 4130 4550 5000 5470 5990
	*
283	KK RPT14
284	KM ROUTE BYPASS FROM I15 BRIDGE 36'x4' TO I15 BRIDGE 24'x4' (IS15 0315)
285	RM 6 0.484 0.150
	*
286	KK D1A
287	BA 2.290
288	PB 3.25
289	LS 0 83.0
290	UD 0.830
	*
291	KK RD1A
292	KM ROUTE BASIN D1A TO PT15
293	RM 2 0.17 0.150
	*
294	KK D1B
295	BA 0.316
296	PB 3.10
297	LS 0 84.0
298	UD 1.060
	*

## HEC-1 INPUT

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
299	KK PT15
300	KM COMBINE BASINS D1A AND D1B
301	HC 2
	*
302	KK RPT15
303	KM ROUTE PT15 TO PT16
304	RM 4 0.33 0.150
	*
305	KK D1C
306	BA 0.565
307	PB 2.97
308	LS 0 83.0
309	UD 1.290
	*
310	KK PT16
311	KM COMBINE PT15 AND BASIN D1C
312	HC 2
	*
313	KK RPT16
314	KM ROUTE PT16 TO PT17
315	RM 3 0.26 0.150
	*
316	KK D1D
317	BA 1.231
318	PB 2.99
319	LS 0 82.0
320	UD 0.500
	*
321	KK RD1D
322	KM ROUTE BASIN D1D TO PT17
323	RM 2 0.16 0.150
	*
324	KK D1E
325	BA 0.224
326	PB 2.83
327	LS 0 81.5
328	UD 1.060
	*
329	KK D1F
330	BA 0.805
331	PB 2.86
332	LS 0 75.2
333	UD 1.500
	*

□ □

## HEC-1 INPUT

PAGE 10

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
334	KK PT17
335	KM "COMBINE PT16, BASINS D1D, D1E, AND D1F"
336	HC 4
	*
337	KK RPT17
338	KM ROUTE PT17 TO PT18
339	RM 6 0.5 0.100
	*
340	KK B18A
341	BA 1.503
342	PB 2.79
343	LS 0 69.4 1.0
344	UD 0.700
	*
345	KK PT18
346	KM "COMBINE PT17, BASIN B18A, AND BYPASS FROM PT14 (IS15 0233)"
347	HC 3
	*
348	KK DVI154
349	KM DIVERSION EAST THRU I15 BRIDGE 24'x4' (IS15 0315)
350	DT DVI154
351	DI 0 2318 4128 5274 6452 7636 8617 9479
352	DQ 0 1040 1205 1252 1297 1341 1373 1404
	*
353	KK RPT18
354	KM ROUTE BYPASS FROM I15 BRIDGE 24'x4' TO I15 BRIDGE 36'x5' (IS15 0359)
355	RM 1 0.113 0.150
	*
356	KK B18B
357	BA 1.916
358	PB 2.77
359	LS 0 86.4
360	UD 0.640
	*
361	KK DVBD2
362	KM RETURN FLOW PASSING THRU UPRR CULVERT AT PT2 (UPRR 0471)
363	DR DVBD2
	*
364	KK RDVBD2
365	KM ROUTE DVBD2 TO PT19
366	RM 6 0.52 0.100
	*

## HEC-1 INPUT

PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10  
 367 KK PT19  
 368 KM "COMBINE BASIN B18B, DVBD2, AND BYPASS FROM PT18"  
 369 HC 3  
 \*  
 370 KK DVI155  
 371 KM DIVERSION EAST THRU I15 BRIDGE 36'x5' (IS15 0359)  
 372 DT DVI155  
 373 DI 0 2716 3378 4083 4548 4960 5791 6843  
 374 DQ 0 1980 2192 2367 2531 2651 3141 3890  
 \*  
 375 KK RPT19  
 376 KM ROUTE BYPASS AT PT19 TO PT21  
 377 RM 8 0.641 0.150  
 \*  
 378 KK B14A  
 379 BA 1.351  
 380 PB 2.77  
 381 LS 0 88.0 5.0  
 382 UD 0.400  
 \*  
 383 KK RDVBD3  
 384 KM "RETURN FLOW PASSING THRU UPRR CULVERT AT PT3, BD3 (UPRR 0511)"  
 385 DR DVBD3  
 \*  
 386 KK RDVBD4  
 387 KM "RETURN FLOW PASSING THRU UPRR CULVERT, BD4 (UPRR 0534)"  
 388 DR DVBD4  
 \*  
 389 KK PT20  
 390 KM COMBINE DVBD3 AND DVBD4  
 391 HC 2  
 \*  
 392 KK RDVBD4  
 393 KM ROUTE PT20 TO PT21  
 394 RM 6 0.51 0.100  
 \*  
 395 KK PT21  
 396 KM "COMBINE PT20, BASIN B14A, AND BYPASS FROM PT19"  
 397 HC 3  
 \*

## HEC-1 INPUT

PAGE 12

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
398	KK PT22
399	KM COMBINE PT21 AND PT22W
400	HC 2
	*
401	KK RPT22
402	KM ROUTE PT22 TO PT23
403	RM 4 0.3 0.100
	*
404	KK B15B
405	BA 0.954
406	PB 2.77
407	LS 0 75.0 13.0
408	UD 0.71
	*
409	KK PT23
410	KM COMBINE PT22 AND BASIN B15B
411	HC 2
	*
412	KK RDVI151
413	KM RETURN FLOW PASSING THRU I151 (DCWA 1429)
414	DR DVI151
	*
415	KK RDVI151
416	KM ROUTE DVI151 TO PT24
417	RM 1 0.099 0.150
	*
418	KK RDVI152
419	KM RETURN FLOW PASSING THRU I152 (IS15 0199)
420	DR DVI152
	*
421	KK RDVI152
422	KM ROUTE DVI152 TO PT24
423	RM 1 0.089 0.150
	*
424	KK PT24
425	KM COMBINE DVI151 AND DVI152
426	HC 2
	*
427	KK RPT24
428	KM ROUTE PT24 TO PT25
429	RM 1 0.086 0.150
	*



## HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
430	KK	D009									
431	BA	3.621									
432	PB	3.31									
433	LS	0	83.0								
434	UD	0.510									
	*										
435	KK	RD009									
436	KM	ROUTE BASIN D009 TO PT5									
437	RM	1	0.1	0.1							
	*										
438	KK	D004									
439	BA	2.400									
440	PB	3.30									
441	LS	0	85.0								
442	UD	0.700									
	*										
443	KK	PT5									
444	KM	COMBINE BASINS D009 AND D004									
445	HC	2									
	*										
446	KK	RET5									
447	KM	ROUTE PT5 TO PT6									
448	RM	2	0.44	0.15							
	*										
449	KK	D08									
450	BA	11.912									
451	PB	3.28									
452	LS	0	85.0								
453	UD	1.650									
	*										
454	KK	PT6									
455	KM	COMBINE PT5 AND BASIN D08									
456	HC	2									
	*										
457	KK	RET6									
458	KM	ROUTE PT6 TO PT7									
459	RM	3	0.81	0.150							
	*										
460	KK	D010									
461	BA	10.434									
462	PB	3.15									
463	LS	0	84.0								
464	UD	1.050									
	*										

## HEC-1 INPUT

PAGE 14

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
465	KK D07
466	BA 4.280
467	PB 3.20
468	LS 0 84.0
469	UD 1.100
	*
470	KK PT7
471	KM "COMBINE PT6, BASINS D010 AND D07"
472	HC 3
	*
473	KK RPT7
474	KM ROUTE PT7 TO PT8
475	RM 3 0.224 0.150
	*
476	KK D012B
477	BA 0.566
478	PB 2.77
479	LS 0 83.0
480	UD 0.642
	*
481	KK PT8
482	KM COMBINE BASIN D012B AND PT7
483	HC 2
	*
484	KK DVPITT
485	KM DIVERSION EAST ACROSS LAS VEGAS BLVD. TO PITTMAN WATERSHED - DOES NOT RETURN
486	KM REFERENCED FROM 1995 PITTMAN WASH FIS RESTUDY BY NIMBUS
487	KM "MODIFIED TO RETAIN 10% OF ACTUAL DIVERTED FLOW TO DIVERT LATER AT ""DVROSE""
488	DT DVPITT
489	DI 0 3586 3710 3835 3961 5795 7737
490	DQ 0 1554 1618 1681 1747 2774 3933
	*
491	KK DVROSE
492	KM DIVERSION NORTH ACROSS ST. ROSE PARKWAY (FORMERLY LAKE MEAD DRIVE)
493	KM DIVERSION BYPASSES LDCDB VIA BASIN D5A AND COMBINES AT PT29
494	KM REFERENCED FROM 1995 PITTMAN WASH FIS RESTUDY BY NIMBUS
495	DT DVROSE
496	DI 0 1673 1721 1770 2214 3021 3803
497	DQ 0 359 371 384 396 579 774
	*
498	KK RPT8
499	KM ROUTE BYPASS FROM DVROSE AT PT8 TO PT25
500	RM 6 0.49 0.150
	*

□ □

## HEC-1 INPUT

PAGE 15

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
501	KK D4A
502	BA 1.873
503	PB 2.80
504	LS 0 83.8 0.0
505	UD 0.245
	*
	* THE FOLLOWING COMBINE POINT IS ADDED TO CLARIFY THE EXISTING FLOWS
	* BETWEEN LAS VEGAS BLVD AND PT25(B)
506	KK PT25A COMBINES FLOWS WEST OF LAS VEGAS BLVD
507	KM "COMBINE PT24, BASIN D4A, AND BYPASS FROM PT8"
508	HC 3
	*
	*
509	KK RDVI153
510	KM RETURN FLOW PASSING THRU I153 AT PT14 (IS15 0233)
511	DR DVI153
	*
512	KK RDVI153
513	KM ROUTE DVI153 TO PT25
514	RM 1 0.112 0.150
515	KK RPT25
516	KM ROUTE PT25 TO PT26
517	RM 1 0.099 0.150
	*
518	KK PT25B
519	KM COMBINE PT25A AND DVI153
520	HC 2
	*
521	KK D4B
522	BA 2.033
523	PB 2.80
524	LS 0 83.8 8.0
525	UD 0.243
	*
526	KK PT26
527	KM COMBINE PT25 AND BASIN D4
528	HC 2
	*
529	KK RPT26
530	KM ROUTE PT26 TO LOWER DUCK CREEK DETENTION BASIN (LDCDB)
531	RM 1 0.083 0.150
	*

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## HEC-1 INPUT

PAGE 16

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
532	KK RDVI154
533	KM "RETURN FLOW PASSING THRU I15 BRIDGE, I154 (IS15 0315)"
534	DR DVI154
	*
535	KK RDVI154
536	KM ROUTE DVI154 TO LAS VEGAS BLVD.
537	RD 2700 0.007 0.015 0 TRAP 25 2
	*
538	KK RDVI154
539	KM ROUTE DVI154 FROM LAS VEGAS BLVD. TO PT27
540	KM "2-78"" CMP (EQUIVALENT RECTANGULAR CHANNEL DETERMINED BY COMPARING VELOCITY)
541	RD 1300 0.017 0.015 0 TRAP 28 0
	*
542	KK RDVI155
543	KM "RETURN FLOW PASSING THRU I15 BRIDGE, I155 (IS15 0359)"
544	DR DVI155
	*
545	KK RDVI155
546	KM ROUTE DVI155 TO AGATE AVENUE
547	RD 1100 0.01 0.040 0 TRAP 15 3
	*
548	KK RDVI155
549	KM ROUTE DVI155 FROM AGATE AVENUE TO LAS VEGAS BLVD.
550	KM "3-10'x5' RCB, INCLUDES PORTION OF TRAP CHANNEL"
551	RD 1530 0.01 0.015 0 TRAP 30 0
	*
552	KK RDVI155
553	KM ROUTE DVI155 FROM LAS VEGAS BLVD. TO PT27
554	KM "2-96"" CMP (EQUIVALENT RECTANGULAR CHANNEL DETERMINED BY COMPARING VELOCITY)
555	RD 800 0.006 0.015 0 TRAP 35 0
	*
556	KK RDVI155
557	KM ROUTE DVI155 FROM LAS VEGAS BLVD. TO PT27
558	KM "2-84"" CMP (EQUIVALENT RECTANGULAR CHANNEL DETERMINED BY COMPARING VELOCITY)
559	RD 1100 0.012 0.015 0 TRAP 30 0
	*
560	KK PT27
561	KM COMBINE DVI154 AND DVI155
562	HC 2
	*

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## HEC-1 INPUT

PAGE 17

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
563	KK RPT27
564	KM ROUTE PT27 TO LOWER DUCK CREEK DETENTION BASIN (LDCDB)
565	KM "3-96" CIPCP (EQUIVALENT RECTANGULAR CHANNEL)"
566	RD 1100 0.009 0.015 0 TRAP 56 0
	*
567	KK RPT27
568	KM ROUTE PT27 TO LOWER DUCK CREEK DETENTION BASIN (LDCDB)
569	KM 3-8'x8' RCB (EQUIVALENT RECTANGULAR CHANNEL)
570	RD 570 0.004 0.015 0 TRAP 24 0
	*
571	KK B18C
572	BA 1.259
573	PB 2.77
574	LS 0 75.4 25.0
575	UD 0.380
	*
576	KK PT28
577	KM "COMBINE PT26, PT27, AND BASIN B18C AT LDCDB"
578	HC 3
	*
579	KK LDCDB
580	KM LOWER DUCK CREEK DETENTION BASIN
581	RS 1 STOR 0
582	SV 0 2.8 37.5 120.3 229 340 453.8 570 689 810
583	SV 934 1061 1190
584	SE 2183 2184 2186 2188 2190 2192 2194 2196 2198 2200
585	SE 2202 2204 2206
586	SQ 0 126 654 1825 3310 4310 5118 5815 6437 7004
587	SQ 7528 8018 8480
588	SE 2183 2184 2186 2188 2190 2192 2194 2196 2198 2200
589	SE 2202 2204 2206
590	KO 1
	*
591	KK RLDCDB
592	KM ROUTE THE OUTFALL FROM THE LOWER DUCK CREEK DETENTION BASIN TO PT29
593	RD 1100 0.004 0.015 0 TRAP 46 1.5
	*
594	KK D5A
595	BA 1.325
596	PB 2.79
597	LS 0 88.0 15.0
598	UD 0.350
	*

## HEC-1 INPUT

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
599	KK RD5A
600	KM ROUTE BASIN D5A TO PT29
601	RM 1 0.12 0.150
	*
602	KK RDVROSE
603	KM RETURN FLOW CROSSING ST. ROSE PARKWAY (FORMERLY LAKE MEAD DRIVE)
604	DR DVROSE
	*
605	KK RDVROSE
606	KM ROUTE DIVERSION DVROSE TO PT29
607	RM 6 0.49 0.150
	*
608	KK PT29
609	KM "COMBINE BASIN D5A, BYPASS FROM DVROSE, AND OUTFALL FROM LDCDB"
610	HC 3
	*
611	KK RPT29
612	KM ROUTE PT29 TO PT30
613	RD 1800 0.011 0.015 0 TRAP 46 1.5
	*
614	KK B16B
615	BA 0.701
616	PB 2.77
617	LS 0 83.1 32.0
618	UD 0.380
	*
619	KK RB16B
620	KM ROUTE BASIN B16B TO PT30
621	RD 1900 0.008 0.015 0 TRAP 0 50
	*
622	KK PT30
623	KM COMBINE PT29 AND BASIN B16B
624	HC 2
	*
625	KK RPT30
626	KM ROUTE PT30 TO PT31
627	RD 2900 0.005 0.015 0 TRAP 20 3
	*
628	KK B16A
629	BA 0.906
630	PB 2.77
631	LS 0 78.2 17.0
632	UD 0.710
	*

## HEC-1 INPUT

PAGE 19

LINE	ID	1	2	3	4	5	6	7	8	9	10
633	KK	D5B									
634	BA	0.602									
635	PB	2.77									
636	LS	0	82.8	46.0							
637	UD	0.710									
	*										
638	KK	PT31									
639	KM	"COMBINE PT30, BASINS B16A AND D5B"									
640	HC	3									
	*										
641	KK	RPT31									
642	KM	ROUTE PT31 TO PT32									
643	RD	3200	0.016	0.015	0	TRAP	15	1			
	*										
644	KK	PT32									
645	KM	COMBINE PT23 AND PT31 AT DUCK CREEK/MIDDLE BRANCH BLUE DIAMOND CONFLUENCE									
646	HC	2									
	*										
647	KK	RPT32									
648	KM	ROUTE PT32 TO PT34									
649	RD	2700	0.009	0.015	0	TRAP	70	2			
	*										
650	KK	HYDRO2									
651	KM	INFLOW HYDROGRAPH									
652	KM	"PT C-11 REFERENCED FROM ""LOMR FOR AREA DOWNSTREAM OF UPPER BLUE DIAMOND D.B									
653	KM	"SEE COMBINATION POINT C-11, DARF RATIO 0.98 OF HEC-1 FROM REFERENCED LOMR"									
654	BA	0.380									
655	QI	0	0	0	0	0	0	0	0	0	0
656	QI	0	0	0	0	0	0	0	0	0	0
657	QI	0	0	0	0	0	0	0	0	0	0
658	QI	0	0	0	0	0	0	0	0	0	0
659	QI	0	0	1	3	5	9	14	19	23	27
660	QI	30	32	33	33	32	31	29	28	27	27
661	QI	27	27	32	36	36	35	33	31	29	26
662	QI	24	22	20	19	17	16	15	14	13	12
663	QI	11	10	9	8	8	7	6	6	5	5
664	QI	4	4	4	3	3	3	3	3	2	2
665	QI	2	2	2	2	2	2	1	1	1	1
666	QI	1	1	1	1	1	1	1	1	1	1
667	QI	1	1	1	1	1	1	1	0	0	0
668	QI	0	0	0	0	0	0	0	0	0	0
669	QI	0	0	0	0	0	0	0	0	0	0
670	QI	0	0	0	0	0	0	0	0	0	0
671	QI	0	0	0	0	0	0	0	0	0	0
672	QI	0	0	0	0	0	0	0	0	0	0
673	QI	0	0	0	0	0	0	0	0	0	0
674	QI	0	0	0	0	0	0	0	0	0	0
675	QI	0	0	0	0	0	0	0	0	0	0
676	QI	0	0	0	0	0	0	0	0	0	0

## HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
677	QI	0	0	0	0	0	0	0	0	0	0
678	QI	0	0	0	0	0	0	0	0	0	0
679	QI	0	0	0	0	0	0	0	0	0	0
680	QI	0	0	0	0	0	0	0	0	0	0
681	QI	0	0	0	0	0	0	0	0	0	0
682	QI	0	0	0	0	0	0	0	0	0	0
683	QI	0	0	0	0	0	0	0	0	0	0
684	QI	0	0	0	0	0	0	0	0	0	0
*											
685	KK	RHYDRO2									
686	KM	ROUTE HYDRO2 TO PT33W									
687	RM	8	0.76	0.150							
*											
688	KK	B12									
689	BA	0.969									
690	PB	2.77									
691	LS	0	68.4	1.0							
692	UD	0.390									
*											
693	KK	PT33W									
694	KM	COMBINE HYDRO2 AND BASIN B12									
695	HC	2									
*											
696	KK	RDVBD7									
697	KM	"RETURN FLOW SPLIT, DVBD7, FROM MIDDLE BRANCH "									
698	DR	DVBD7									
*											
699	KK	RDVBD7									
700	KM	ROUTE DVBD7 TO PT33									
701	RM	2	0.2	0.150							
*											
702	KK	PT33									
703	KM	COMBINE PT33W AND DVBD7									
704	HC	2									
*											
705	KK	RPT33									
706	KM	ROUTE PT33 TO PT34W									
707	RM	4	0.3	0.100							
*											
708	KK	B15A									
709	BA	0.959									
710	PB	2.77									
711	LS	0	77.6	14.0							
712	UD	0.500									
*											



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## HEC-1 INPUT

PAGE 21

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
713	KK PT34W
714	KM COMBINE PT33 AND BASIN B15A
715	HC 2
	*
716	KK PT34
717	KM COMBINE PT34W AND PT32
718	HC 2
	*
719	KK MNWB
720	BA 0.442
721	PB 2.77
722	LS 0 85.0 2.0
723	UD 0.742
	*
724	KK RMNWB
725	KM ROUTE MNWB NORTH ALONG I15 TO PT35
726	RD 2200 0.007 0.025 0 TRAP 20 10
	*
727	KK MNWC
728	BA 0.596
729	PB 2.77
730	LS 0 85.0 7.0
731	UD 0.738
	*
732	KK PT35
733	KM COMBINE BASINS MNWB AND MNWC
734	HC 2
	*
735	KK RPT35
736	KM ROUTE PT35 TO PT36
737	RM 1 0.055 0.150
	*
738	KK MNWD
739	BA 0.078
740	PB 2.77
741	LS 0 85.0 13.0
742	UD 0.321
	*
743	KK PT36
744	KM COMBINE PT35 AND BASIN MNWD
745	HC 2
	*

□ □

## HEC-1 INPUT

PAGE 22

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
746	KK RPT36
747	KM ROUTE PT36 TO PT37
748	RM 2 0.192 0.150
	*
749	KK MC2
750	BA 0.919
751	PB 2.77
752	LS 0 74.9 28.0
753	UD 0.873
	*
754	KK PT37
755	KM COMBINE PT36 AND BASIN MC2
756	HC 2
	*
757	KK RPT37
758	KM ROUTE PT37 TO PT38
759	RD 5200 0.01 0.015 0 TRAP 20 2
	*
760	KK B10A
761	BA 1.601
762	PB 2.77
763	LS 0 74.9 32.0
764	UD 0.217
	*
765	KK PT38
766	KM COMBINE PT37 AND BASIN B10A
767	HC 2
	*
768	KK RPT38
769	KM ROUTE PT38 TO PT39
770	RD 2100 0.002 0.015 0 TRAP 20 2
	*
771	KK PT39
772	KM COMBINE PT34 AND PT38
773	HC 2
	*
774	KK RPT39
775	KM ROUTE PT39 TO PT40
776	RD 2500 0.006 0.025 0 TRAP 70 2
	*

## HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
777	KK	D6									
778	BA	1.564									
779	PB	2.77									
780	LS	0	78.3	43.0							
781	UD	0.500									
	*										
782	KK	PT40									
783	KM	COMBINE PT39 AND BASIN D6									
784	HC	2									
	*										
785	KK	RPT40									
786	KM	ROUTE PT40 TO PT41									
787	RM	1 0.08 0.200									
	*										
788	KK	D7									
789	BA	0.850									
790	PB	2.77									
791	LS	0	77.3	23.0							
792	UD	0.710									
	*										
793	KK	PT41									
794	KM	COMBINE PT40 AND BASIN D7									
795	HC	2									
	*										
796	KK	RPT41									
797	KM	ROUTE PT41 TO PT42									
798	RD	4150 0.006 0.015 0 TRAP 50 0									
	*										
799	KK	D8									
800	BA	1.258									
801	PB	2.77									
802	LS	0	77.8	34.0							
803	UD	0.230									
	*										
804	KK	RD8									
805	KM	ROUTE BASIN D8 TO PT42									
806	RM	1 0.088 0.150									
	*										
807	KK	PT42									
808	KM	COMBINE PT41 AND BASIN D8									
809	HC	2									
	*										

## HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10
810	KK	RPT42									
811	KM	ROUTE PT42 TO PT44									
812	RD	3650	0.011	0.015	0	TRAP	28	2			
	*										
813	KK	B10B									
814	BA	0.848									
815	PB	2.77									
816	LS	0	74.9	44.0							
817	UD	0.687									
	*										
818	KK	RB10B									
819	KM	ROUTE BASIN B10B TO PT43									
820	RM	6	0.46	0.100							
	*										
821	KK	D9A									
822	BA	0.861									
823	PB	2.77									
824	LS	0	72.8	16.0							
825	UD	1.100									
	*										
826	KK	PT43									
827	KM	COMBINE BASINS B10B AND D9A									
828	HC	2									
	*										
829	KK	RPT43									
830	KM	ROUTE PT43 TO PT44									
831	RD	4050	0.01	0.016	0	TRAP	0	16			
	*										
832	KK	PT44									
833	KM	COMBINE PT42 AND PT43									
834	HC	2									
	*										
835	KK	RPT44									
836	KM	ROUTE PT44 TO PT45									
837	RM	1	0.119	0.150							
	*										
838	KK	D9B									
839	BA	1.544									
840	PB	2.77									
841	LS	0	77.8	17.0							
842	UD	0.410									
	*										

□ □

## HEC-1 INPUT

PAGE 25

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
843	KK PT45
844	KM COMBINE PT44 AND BASIN D9B
845	HC 2
	*
846	KK RPT45
847	KM ROUTE PT45 TO PT46
848	RM 2 0.184 0.150
	*
849	KK D10
850	BA 2.508
851	PB 2.77
852	LS 0 79.0 33.0
853	UD 0.440
	*
854	KK PT46
855	KM COMBINE PT45 AND BASIN D10
856	HC 2
	*
857	KK RPT46
858	KM ROUTE PT46 TO PT48
859	RD 2500 0.018 0.035 0 TRAP 75 2
	*
860	KK MC
861	KM "TLAG AND AREA CALCULATED, CN REFERENCED FROM 1996 MPU"
862	KM THEREFORE NO INCREASE IN PERCENT IMPERVIOUS
863	BA 2.804
864	PB 2.77
865	LS 0 91.0
866	UD 0.244
	*
867	KK TORAW
868	KM DIVERT FLOW WHICH BYPASSES DETENTION BASIN
869	DT TORAW
870	DI 0 100 500 1000 2000 3000 4000
871	DQ 0 40 100 200 400 570 760
	*
872	KK MCDB
873	KM MCCARRAN AIRPORT DETENTION FACILITIES
874	KM SIMULATION OF TWO DETENTION BASINS - MCCARRAN EAST AND SPENCER
875	RS 1 STOR -1
876	SV 0 8 40 70 100 160 210 500
877	SQ 0 30 40 55 65 80 85 87
878	SE 0 2 4 6 8 12 15 16
879	KO 1
	*

## HEC-1 INPUT

LINE	ID	1	2	3	4	5	6	7	8	9	10	
880	KK	RMCDB										
881	KM	ROUTE OUTFALL FROM MCDB TO PT47										
882	RD	3300	0.006	0.013	0	CIRC	4	0				
	*											
883	KK	RTORAW										
884	KM	FLOW BYPASSING THE MCCARRAN DETENTION FACILITIES RETURNED IN RAWHIDE CHANNEL										
885	DR	TORAW										
	*											
886	KK	PT47										
887	KM	COMBINE OUTFALL FROM MCCARRAN DETENTION FACILITIES AND BYPASSED FLOW										
888	HC	2										
	*											
889	KK	RPT47										
890	KM	ROUTE PT47 THRU RAWHIDE CHANNEL TO PT48W										
891	RD	15700	0.015	0.015	0	TRAP	11	1.5				
	*											
892	KK	RH										
893	BA	1.355										
894	PB	2.77										
895	LS	0	72.0	6.0								
896	UD	0.340										
	*											
897	KK	PT48W										
898	KM	COMBINE PT47 AND BASIN RH										
899	HC	2										
	*											
900	KK	B20										
901	BA	0.790										
902	PB	2.77										
903	LS	0	82.4	8.0								
904	UD	0.710										
	*											
905	KK	RB20										
906	KM	ROUTE BASIN B20 TO PT48										
907	KM	MUSKINGUM ROUTING UNSTABLE										
908	RD	1800	0.017	0.030	0	TRAP	30	10				
	*											
909	KK	PT48										
910	KM	"COMBINE PT46, PT48W, AND BASIN B20"										
911	HC	3										
	*											

□ □

## HEC-1 INPUT

PAGE 27

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
912	KK RPT48
913	KM ROUTE PT48 TO PT50
914	RM 1 0.067 0.150
	*
915	KK D12
916	BA 0.176
917	PB 2.77
918	LS 0 77.8 7.0
919	UD 0.710
	*
920	KK RD12
921	KM ROUTE BASIN D12 TO PT49
922	KM MUSKINGUM ROUTING UNSTABLE
923	RD 1850 0.044 0.150 0 TRAP 50 10
	*
924	KK D11A
925	BA 0.159
926	PB 2.77
927	LS 0 77.3 5.0
928	UD 0.160
	*
929	KK PT49
930	KM COMBINE BASINS D12 AND D11A
931	HC 2
	*
932	KK RPT49
933	KM ROUTE PT49 TO PT50
934	KM MUSKINGUM ROUTING UNSTABLE
935	RD 1050 0.019 0.150 0 TRAP 45 10
	*
936	KK PT50
937	KM COMBINE PT48 AND PT49
938	HC 2
	*
939	KK RPT50
940	KM ROUTE PT50 TO PT51
941	RD 7000 0.003 0.015 0 TRAP 60 2
	*
942	KK D11B
943	BA 0.987
944	PB 2.77
945	LS 0 82.2 12.0
946	UD 0.220
	*

□ □

## HEC-1 INPUT

PAGE 28

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
947	KK PT51
948	KM COMBINE PT50 AND BASIN D11B
949	HC 2
950	ZZ



□ □

## SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT

(V) ROUTING

(---&gt;) DIVERSION OR PUMP FLOW

NO.

(.) CONNECTOR

(&lt;---) RETURN OF DIVERTED OR PUMPED FLOW

```

26      B13A
        V
        V
39      RB13A
        .
        .
42      .      B13B
        .      .
        .      .
47      PT1.....
        V
        V
50      RPT1
        .
        .
53      .      B17
        .      .
        .      .
58      PT2.....
        .
        .
63      .-----> DVBD2
61      DVBD2
        V
        V
66      RPT2
        .
        .
69      .      B13C
        .      .
        .      .
74      PT3.....
        .
        .
79      .-----> DVBD3
77      DVBD3
        V
        V
82      RPT3A
        .
        .
88      .-----> DVBD4
86      DVBD4
        V
        V
91      RPT3B
        .
        .
94      .      B11S
        .      .
        .      .
100     .      .      HYDRO1
        .      .      .
        .      .      .
135     PT4.....
        V
        V
140     RPT4A
        .
        .
147     .-----> DVBD7
143     DVBD7
        V
        V
150     RPT4B
        .
        .
153     .      B14B
        .      .
        .      .
158     PT22W.....
        .

```

```

161      D012A
      V
      V
166      RD012A
      .
      .
172      -----> DVI151
170      DVI151
      V
      V
175      RI151
      .
      .
180      -----> DVI152
178      DVI152
      V
      V
183      RI152
      .
      .
186      D01
      V
      V
191      RD01
      .
      .
194      D005
      .
199      PT9.....
      V
      V
202      RD005
      .
      .
205      D02
      V
      V
210      RD02
      .
      .
213      D03
      .
      .
218      D04
      .
      .
223      PT10.....
      V
      V
226      RPT10
      .
      .
229      D006
      .
      .
234      PT11.....
      V
      V
237      RPT11
      .
      .
240      D2A
      .
      .
245      PT12.....
      V
      V
248      RPT12
      .
      .
251      D2B
      .
      .
256      PT13.....
      V
      V
259      RPT13

```



```

363 . . . . .<----- DVBD2
361 . . . . .DVBD2
. . . . .V
. . . . .V
. . . . .RDVBD2
. . . . .
367 . . . . .PT19.....
. . . . .
372 . . . . .-----> DVI155
370 . . . . .DVI155
. . . . .V
. . . . .V
375 . . . . .RPT19
. . . . .
378 . . . . .B14A
. . . . .
385 . . . . .<----- DVBD3
383 . . . . .RDVBD3
. . . . .
388 . . . . .<----- DVBD4
386 . . . . .RDVBD4
. . . . .
389 . . . . .PT20.....
. . . . .V
. . . . .V
392 . . . . .RDVBD4
. . . . .
395 . . . . .PT21.....
. . . . .
398 . . . . .PT22.....
. . . . .V
. . . . .V
401 . . . . .RPT22
. . . . .
404 . . . . .B15B
. . . . .
409 . . . . .PT23.....
. . . . .
414 . . . . .<----- DVI151
412 . . . . .RDVI151
. . . . .V
. . . . .V
415 . . . . .RDVI151
. . . . .
420 . . . . .<----- DVI152
418 . . . . .RDVI152
. . . . .V
. . . . .V
421 . . . . .RDVI152
. . . . .
424 . . . . .PT24.....
. . . . .V
. . . . .V
427 . . . . .RPT24
. . . . .
430 . . . . .D009
. . . . .V
. . . . .V
435 . . . . .RD009
. . . . .
438 . . . . .D004
. . . . .
443 . . . . .PT5.....

```

```

.
.
.
446 . . . V
. . . V
. . . RPT5
. . .
. . . D08
. . .
. . .
454 . . . PT6.....
. . . V
. . . V
457 . . . RPT6
. . .
. . . D010
. . .
460 . . .
. . . D07
465 . . .
. . .
. . .
470 . . . PT7.....
. . . V
. . . V
473 . . . RPT7
. . .
. . . D012B
476 . . .
. . .
. . .
481 . . . PT8.....
. . .
. . .
488 . . . -----> DVPIIT
484 . . . DVPIIT
. . .
. . .
495 . . . -----> DVROSE
491 . . . DVROSE
. . . V
. . . V
498 . . . RPT8
. . .
. . . D4A
501 . . .
. . .
506 . . . PT25A.....
. . .
. . .
511 . . . .<----- DVI153
509 . . . RDVI153
. . . V
. . . V
512 . . . RDVI153
. . . V
. . . V
515 . . . RPT25
. . .
. . .
518 . . . PT25B.....
. . .
. . . D4B
521 . . .
. . .
526 . . . PT26.....
. . . V
. . . V
529 . . . RPT26
. . .
. . .
534 . . . .<----- DVI154
532 . . . RDVI154
. . . V
. . . V
535 . . . RDVI154
. . . V
. . . V
538 . . . RDVI154

```



```

641      .      RPT31
        .
        .
644      PT32.....
        V
        V
647      RPT32
        .
        .
650      .      HYDRO2
        .      V
        .      V
685      .      RHYDRO2
        .
        .
688      .      .      B12
        .      .
        .      .
693      .      PT33W.....
        .
        .
698      .      .      .      <----- DVBD7
696      .      .      RDVBD7
        .      .      V
        .      .      V
699      .      .      RDVBD7
        .      .
        .      .
702      .      PT33.....
        .      V
        .      V
705      .      RPT33
        .
        .
708      .      .      B15A
        .      .
        .      .
713      .      PT34W.....
        .
        .
16      PT34.....
        .
        .
719      .      MNWB
        .      V
        .      V
724      .      RMNWB
        .
        .
727      .      .      MNWC
        .      .
        .      .
732      .      PT35.....
        .      V
        .      V
735      .      RPT35
        .
        .
738      .      .      MNWD
        .      .
        .      .
743      .      PT36.....
        .      V
        .      V
746      .      RPT36
        .
        .
749      .      .      MC2
        .      .
        .      .
754      .      PT37.....
        .      V
        .      V
757      .      RPT37
        .
        .
760      .      .      B10A
        .      .
        .      .

```

```

765      .      PT38.....
       .      V
       .      V
768      .      RPT38
       .
       .
771      PT39.....
       V
       V
774      RPT39
       .
       .
777      .      D6
       .
       .
782      PT40.....
       V
       V
785      RPT40
       .
       .
788      .      D7
       .
       .
793      PT41.....
       V
       V
796      RPT41
       .
       .
799      .      D8
       .      V
       .      V
804      .      RD8
       .
       .
807      PT42.....
       V
       V
810      RPT42
       .
       .
813      .      B10B
       .      V
       .      V
818      .      RB10B
       .
       .
821      .      D9A
       .
       .
826      .      PT43.....
       .      V
       .      V
829      .      RPT43
       .
       .
832      PT44.....
       V
       V
835      RPT44
       .
       .
838      .      D9B
       .
       .
843      PT45.....
       V
       V
846      RPT45
       .
       .
849      .      D10
       .
       .
854      PT46.....
       V
       V
857      RPT46

```



```

860      .      MC
      .      .
      .      .
869      .      .-----> TORAW
867      .      TORAW
      .      V
      .      V
872      .      MCDB
      .      V
      .      V
880      .      RMCDB
      .      .
      .      .
885      .      .<----- TORAW
883      .      .      RTORAW
      .      .
      .      .
886      .      PT47.....
      .      V
      .      V
889      .      RPT47
      .      .
      .      .
892      .      .      RH
      .      .
      .      .
897      .      PT48W.....
      .      .
      .      .
900      .      .      B20
      .      .      V
      .      .      V
905      .      .      RB20
      .      .
      .      .
909      .      PT48.....
      .      V
      .      V
912      .      RPT48
      .      .
      .      .
915      .      D12
      .      V
      .      V
920      .      RD12
      .      .
      .      .
924      .      .      D11A
      .      .
      .      .
929      .      PT49.....
      .      V
      .      V
932      .      RPT49
      .      .
      .      .
936      .      PT50.....
      .      V
      .      V
939      .      RPT50
      .      .
      .      .
942      .      D11B
      .      .
      .      .
947      .      PT51.....

```

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

□ □

HEC1 S/N: 1333000362      HMVersion: 6.40      Data File: duck5.txt

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* SEPTEMBER 1990 *
* VERSION 4.0 *
* RUN DATE 03/21/2002 TIME 09:09:37 *
*****

```

```

*****
* U.S. ARMY CORPS OF ENGI *
* HYDROLOGIC ENGINEERING *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95 *
* (916) 756-1104 *
*****

```

DUCK CREEK/BLEU DIAMOND WASHES FIS RESTUDY  
 "G.C. WALLACE, INC. BY MJL"  
 DUCK5.XLS  
 "MODEL BASED FROM HEC-1 FILE, BDE100UN, FROM 1991 LAS VEGAS VALLEY FIS"  
 "BY JAMES M. MONTGOMERY CONSULTING ENGINEERS, INC."  
 UPDATED PRECIPITATION VALUES  
 % IMPERVIOUS ADDED TO LS CARD FOR DEVELOPMENT SINCE 1990  
 ADDITIONAL DIVERSIONS ADDED  
 DESIGN STORM = 100-YEAR 6-HOUR STORM  
 STORM DISTRIBUTION = SDN5  
 JR CARDS CONTAIN DARFS BASED ON THE FOLLOWING VALUES:

AREA (SQUARE MILES)	DARF
12 - 16	0.832
16 - 20	0.804
20 - 30	0.765
30 - 40	0.725
40 - 50	0.695
50 - 100	0.640
100 - 150	0.580
150 - 200	0.530

24 IO

## OUTPUT CONTROL VARIABLES

IPRNT	5	PRINT CONTROL
IPLOT	0	PLOT CONTROL
QSCAL	0.	HYDROGRAPH PLOT SCALE

IT

## HYDROGRAPH TIME DATA

NMIN	5	MINUTES IN COMPUTATION INTERVAL
IDATE	1	0 STARTING DATE
ITIME	0000	STARTING TIME
NQ	120	NUMBER OF HYDROGRAPH ORDINATES
NDDATE	1	0 ENDING DATE
NDTIME	0955	ENDING TIME
ICENT	19	CENTURY MARK

COMPUTATION INTERVAL	.08 HOURS
TOTAL TIME BASE	9.92 HOURS

## ENGLISH UNITS

DRAINAGE AREA	SQUARE MILES
PRECIPITATION DEPTH	INCHES
LENGTH, ELEVATION	FEET
FLOW	CUBIC FEET PER SECOND
STORAGE VOLUME	ACRE-FEET
SURFACE AREA	ACRES
TEMPERATURE	DEGREES FAHRENHEIT

JP

## MULTI-PLAN OPTION

NPLAN	1	NUMBER OF PLANS
-------	---	-----------------

JR

## MULTI-RATIO OPTION

## RATIOS OF PRECIPITATION

.83	.80	.76	.73	.69	.64	.58	.53
-----	-----	-----	-----	-----	-----	-----	-----

\*\*\*\*\*

```
*****
*
*   LDCDB
*
*****
```

579 KK

590 KO

## OUTPUT CONTROL VARIABLES

```
IPRNT      1  PRINT CONTROL
IPLOT      0  PLOT CONTROL
QSCAL      0.  HYDROGRAPH PLOT SCALE
```

## HYDROGRAPH ROUTING DATA

```
581 RS      STORAGE ROUTING
              NSTPS      1  NUMBER OF SUBREACHES
              ITYP      STOR  TYPE OF INITIAL CONDITION
              RSVRIC    .00  INITIAL CONDITION
              X          .00  WORKING R AND D COEFFICIENT

582 SV      STORAGE      .0      2.8      37.5      120.3      229.0      340.0      453.8      570.0      689.0
              934.0      1061.0      1190.0

584 SE      ELEVATION    2183.00  2184.00  2186.00  2188.00  2190.00  2192.00  2194.00  2196.00  2198.00
              2202.00  2204.00  2206.00

586 SQ      DISCHARGE    0.      126.      654.      1825.      3310.      4310.      5118.      5815.      6437.
              7528.      8018.      8480.

588 SE      ELEVATION    2183.00  2184.00  2186.00  2188.00  2190.00  2192.00  2194.00  2196.00  2198.00
              2202.00  2204.00  2206.00
```

\*\*\*

## COMPUTED STORAGE-OUTFLOW-ELEVATION DATA

```
STORAGE      .00      2.80      37.50      120.30      229.00      340.00      453.80      570.00      689.00      810.00
OUTFLOW      .00      126.00      654.00      1825.00      3310.00      4310.00      5118.00      5815.00      6437.00      7004.00
ELEVATION    2183.00  2184.00  2186.00  2188.00  2190.00  2192.00  2194.00  2196.00  2198.00  2200.00

STORAGE      934.00  1061.00  1190.00
OUTFLOW      7528.00  8018.00  8480.00
ELEVATION    2202.00  2204.00  2206.00
```

```
WARNING --- ROUTED OUTFLOW ( 8505.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 8704.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 8902.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 9100.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 9297.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 9491.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 9682.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 9868.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 10048.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 10222.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 10388.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 10545.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 10692.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 10829.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 10955.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 11069.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 11172.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
WARNING --- ROUTED OUTFLOW ( 11262.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE
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WARNING --- ROUTED OUTFLOW ( 11341.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11407.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11460.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11501.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11529.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11544.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11547.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11537.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11516.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11482.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11438.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11384.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11321.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11249.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11169.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11081.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10986.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10883.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10773.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10656.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10532.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10402.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10265.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10124.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9978.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9829.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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HYDROGRAPH AT STATION LDCDB  
PLAN 1, RATIO = .83

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	2183.0	*	1		0320	41	724.	42.4	2186.1	*	1		0640	81	9297.	1418
1		0005	2	0.	.0	2183.0	*	1		0325	42	852.	51.5	2186.3	*	1		0645	82	9491.	1472
1		0010	3	2.	.0	2183.0	*	1		0330	43	1010.	62.7	2186.6	*	1		0650	83	9682.	1525
1		0015	4	8.	.2	2183.1	*	1		0335	44	1207.	76.6	2186.9	*	1		0655	84	9868.	1577
1		0020	5	21.	.5	2183.2	*	1		0340	45	1441.	93.1	2187.3	*	1		0700	85	10048.	1628
1		0025	6	41.	.9	2183.3	*	1		0345	46	1695.	111.1	2187.8	*	1		0705	86	10222.	1676
1		0030	7	66.	1.5	2183.5	*	1		0350	47	1944.	129.0	2188.2	*	1		0710	87	10388.	1722
1		0035	8	91.	2.0	2183.7	*	1		0355	48	2176.	146.0	2188.5	*	1		0715	88	10545.	1766
1		0040	9	113.	2.5	2183.9	*	1		0400	49	2396.	162.1	2188.8	*	1		0720	89	10692.	1807
1		0045	10	127.	2.8	2184.0	*	1		0405	50	2608.	177.6	2189.1	*	1		0725	90	10829.	1845
1		0050	11	129.	3.0	2184.0	*	1		0410	51	2811.	192.5	2189.3	*	1		0730	91	10955.	1881
1		0055	12	129.	3.0	2184.0	*	1		0415	52	3007.	206.8	2189.6	*	1		0735	92	11069.	1912
1		0100	13	128.	2.9	2184.0	*	1		0420	53	3201.	221.0	2189.9	*	1		0740	93	11172.	1941
1		0105	14	122.	2.7	2184.0	*	1		0425	54	3372.	235.9	2190.1	*	1		0745	94	11262.	1966
1		0110	15	113.	2.5	2183.9	*	1		0430	55	3522.	252.5	2190.4	*	1		0750	95	11341.	1988
1		0115	16	104.	2.3	2183.8	*	1		0435	56	3696.	271.8	2190.8	*	1		0755	96	11407.	2007
1		0120	17	97.	2.2	2183.8	*	1		0440	57	3902.	294.8	2191.2	*	1		0800	97	11460.	2022
1		0125	18	93.	2.1	2183.7	*	1		0445	58	4147.	321.9	2191.7	*	1		0805	98	11501.	2033

1	0130	19	92.	2.0	2183.7	*	1	0450	59	4404.	353.3	2192.2	*	1	0810	99	11529.	2041
1	0135	20	95.	2.1	2183.8	*	1	0455	60	4656.	388.8	2192.9	*	1	0815	100	11544.	2045
1	0140	21	104.	2.3	2183.8	*	1	0500	61	4934.	427.9	2193.5	*	1	0820	101	11547.	2046
	0145	22	117.	2.6	2183.9	*	1	0505	62	5213.	469.6	2194.3	*	1	0825	102	11537.	2043
	0150	23	129.	3.0	2184.0	*	1	0510	63	5470.	512.5	2195.0	*	1	0830	103	11516.	2037
1	0155	24	136.	3.5	2184.0	*	1	0515	64	5730.	555.8	2195.8	*	1	0835	104	11482.	2028
1	0200	25	145.	4.0	2184.1	*	1	0520	65	5969.	599.4	2196.5	*	1	0840	105	11438.	2016
1	0205	26	156.	4.8	2184.1	*	1	0525	66	6198.	643.3	2197.2	*	1	0845	106	11384.	2001
1	0210	27	170.	5.7	2184.2	*	1	0530	67	6431.	687.9	2198.0	*	1	0850	107	11321.	1983
1	0215	28	186.	6.7	2184.2	*	1	0535	68	6646.	733.5	2198.7	*	1	0855	108	11249.	1963
1	0220	29	203.	7.9	2184.3	*	1	0540	69	6866.	780.5	2199.5	*	1	0900	109	11169.	1940
1	0225	30	221.	9.0	2184.4	*	1	0545	70	7083.	828.7	2200.3	*	1	0905	110	11081.	1916
1	0230	31	238.	10.1	2184.4	*	1	0550	71	7292.	878.2	2201.1	*	1	0910	111	10986.	1889
1	0235	32	255.	11.3	2184.5	*	1	0555	72	7506.	928.9	2201.9	*	1	0915	112	10883.	1861
1	0240	33	275.	12.6	2184.6	*	1	0600	73	7708.	980.7	2202.7	*	1	0920	113	10773.	1830
1	0245	34	297.	14.1	2184.6	*	1	0605	74	7912.	1033.6	2203.6	*	1	0925	114	10656.	1797
1	0250	35	325.	15.9	2184.8	*	1	0610	75	8113.	1087.4	2204.4	*	1	0930	115	10532.	1763
1	0255	36	359.	18.1	2184.9	*	1	0615	76	8308.	1142.0	2205.3	*	1	0935	116	10402.	1726
1	0300	37	403.	21.0	2185.0	*	1	0620	77	8505.	1197.1	2206.1	*	1	0940	117	10265.	1688
1	0305	38	459.	24.7	2185.3	*	1	0625	78	8704.	1252.5	2207.0	*	1	0945	118	10124.	1649
1	0310	39	530.	29.3	2185.5	*	1	0630	79	8902.	1307.9	2207.8	*	1	0950	119	9978.	1608
1	0315	40	618.	35.1	2185.9	*	1	0635	80	9100.	1363.2	2208.7	*	1	0955	120	9829.	1566

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PEAK FLOW	TIME		6-HR	24-HR	72-HR	9.92-HR□
(CFS)	(HR)	(CFS)□				
11547.	8.33		8513.	5292.	5292.	5292.
		(INCHES)	2.062	2.119	2.119	2.119
		(AC-FT)	4221.	4337.	4337.	4337.

PEAK STORAGE	TIME		6-HR	24-HR	72-HR	9.92-HR□
(AC-FT)	(HR)					
2046.	8.33		1297.	793.	793.	793.

PEAK STAGE	TIME		6-HR	24-HR	72-HR	9.92-HR□
(FEET)	(HR)					
2219.28	8.33		2207.43	2198.41	2198.41	2198.41

CUMULATIVE AREA = 38.38 SQ MI

WARNING --- ROUTED OUTFLOW ( 8552.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8736.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8918.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9098.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9275.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9447.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9615.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9776.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9930.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10076.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10213.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10340.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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WARNING --- ROUTED OUTFLOW ( 10743.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10816.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10878.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10928.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10966.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10992.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11006.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 11008.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10998.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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WARNING --- ROUTED OUTFLOW ( 10946.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10905.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10855.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10796.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10729.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10654.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10571.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10481.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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WARNING --- ROUTED OUTFLOW ( 10279.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10168.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10049.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9925.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9795.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9660.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9521.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9379.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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HYDROGRAPH AT STATION LDCDB  
PLAN 1, RATIO = .80

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	2183.0	*	1		0320	41	646.	37.0	2186.0	*	1		0640	81	8918.	1312
1		0005	2	0.	.0	2183.0	*	1		0325	42	763.	45.2	2186.2	*	1		0645	82	9098.	1362
1		0010	3	2.	.0	2183.0	*	1		0330	43	908.	55.5	2186.4	*	1		0650	83	9275.	1411
1		0015	4	8.	.2	2183.1	*	1		0335	44	1090.	68.3	2186.7	*	1		0655	84	9447.	1460
1		0020	5	21.	.5	2183.2	*	1		0340	45	1306.	83.6	2187.1	*	1		0700	85	9615.	1506
1		0025	6	40.	.9	2183.3	*	1		0345	46	1542.	100.3	2187.5	*	1		0705	86	9776.	1552
1		0030	7	64.	1.4	2183.5	*	1		0350	47	1777.	116.9	2187.9	*	1		0710	87	9930.	1595
1		0035	8	88.	2.0	2183.7	*	1		0355	48	1994.	132.7	2188.2	*	1		0715	88	10076.	1635
1		0040	9	109.	2.4	2183.9	*	1		0400	49	2200.	147.7	2188.5	*	1		0720	89	10213.	1673
1		0045	10	124.	2.7	2184.0	*	1		0405	50	2401.	162.4	2188.8	*	1		0725	90	10340.	1709
1		0050	11	127.	2.9	2184.0	*	1		0410	51	2596.	176.7	2189.0	*	1		0730	91	10456.	1741
1		0055	12	127.	2.9	2184.0	*	1		0415	52	2785.	190.6	2189.3	*	1		0735	92	10563.	1771
1		0100	13	124.	2.7	2184.0	*	1		0420	53	2971.	204.2	2189.5	*	1		0740	93	10658.	1798
1		0105	14	115.	2.6	2183.9	*	1		0425	54	3162.	218.2	2189.8	*	1		0745	94	10743.	1821
1		0110	15	106.	2.3	2183.8	*	1		0430	55	3349.	233.3	2190.1	*	1		0750	95	10816.	1842
1		0115	16	97.	2.2	2183.8	*	1		0435	56	3505.	250.6	2190.4	*	1		0755	96	10878.	1859
1		0120	17	90.	2.0	2183.7	*	1		0440	57	3690.	271.2	2190.8	*	1		0800	97	10928.	1873
1		0125	18	85.	1.9	2183.7	*	1		0445	58	3912.	295.8	2191.2	*	1		0805	98	10966.	1884
1		0130	19	83.	1.9	2183.7	*	1		0450	59	4171.	324.6	2191.7	*	1		0810	99	10992.	1891



1	0135	20	86.	1.9	2183.7	*	1	0455	60	4433.	357.3	2192.3	*	1	0815	100	11006.	1895
1	0140	21	93.	2.1	2183.7	*	1	0500	61	4689.	393.4	2192.9	*	1	0820	101	11008.	1895
1	0145	22	104.	2.3	2183.8	*	1	0505	62	4963.	432.0	2193.6	*	1	0825	102	10998.	1893
1	0150	23	118.	2.6	2183.9	*	1	0510	63	5227.	472.0	2194.3	*	1	0830	103	10977.	1887
1	0155	24	129.	3.0	2184.0	*	1	0515	64	5471.	512.6	2195.0	*	1	0835	104	10946.	1878
1	0200	25	136.	3.5	2184.0	*	1	0520	65	5716.	553.5	2195.7	*	1	0840	105	10905.	1867
1	0205	26	145.	4.0	2184.1	*	1	0525	66	5944.	594.7	2196.4	*	1	0845	106	10855.	1853
1	0210	27	156.	4.8	2184.1	*	1	0530	67	6162.	636.4	2197.1	*	1	0850	107	10796.	1836
1	0215	28	170.	5.7	2184.2	*	1	0535	68	6384.	678.9	2197.8	*	1	0855	108	10729.	1817
1	0220	29	184.	6.6	2184.2	*	1	0540	69	6594.	722.4	2198.6	*	1	0900	109	10654.	1796
1	0225	30	199.	7.6	2184.3	*	1	0545	70	6803.	767.1	2199.3	*	1	0905	110	10571.	1773
1	0230	31	213.	8.5	2184.3	*	1	0550	71	7016.	812.9	2200.0	*	1	0910	111	10481.	1748
1	0235	32	227.	9.5	2184.4	*	1	0555	72	7215.	859.9	2200.8	*	1	0915	112	10384.	1721
1	0240	33	243.	10.5	2184.4	*	1	0600	73	7418.	908.0	2201.6	*	1	0920	113	10279.	1692
1	0245	34	262.	11.8	2184.5	*	1	0605	74	7617.	956.9	2202.4	*	1	0925	114	10168.	1661
1	0250	35	286.	13.3	2184.6	*	1	0610	75	7809.	1006.7	2203.1	*	1	0930	115	10049.	1628
1	0255	36	315.	15.2	2184.7	*	1	0615	76	8003.	1057.1	2203.9	*	1	0935	116	9925.	1593
1	0300	37	353.	17.7	2184.9	*	1	0620	77	8186.	1107.9	2204.7	*	1	0940	117	9795.	1557
1	0305	38	403.	21.0	2185.0	*	1	0625	78	8369.	1159.0	2205.5	*	1	0945	118	9660.	1519
1	0310	39	466.	25.1	2185.3	*	1	0630	79	8552.	1210.2	2206.3	*	1	0950	119	9521.	1480
1	0315	40	546.	30.4	2185.6	*	1	0635	80	8736.	1261.4	2207.1	*	1	0955	120	9379.	1440

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PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	9.92-HR
11008.	8.33	(CFS)□	8127.	5045.	5045.	5045.
		(INCHES)	1.969	2.020	2.020	2.020
		(AC-FT)	4030.	4135.	4135.	4135.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	9.92-HR
1896.	8.33		1200.	733.	733.	733.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	9.92-HR
2216.94	8.33		2205.91	2197.45	2197.45	2197.45

CUMULATIVE AREA = 38.38 SQ MI

WARNING --- ROUTED OUTFLOW ( 8581.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8739.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8894.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9044.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9188.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9325.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9456.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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WARNING --- ROUTED OUTFLOW ( 10178.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10223.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10256.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10278.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10289.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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WARNING --- ROUTED OUTFLOW ( 10280.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10261.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10232.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10195.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10149.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10095.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 10034.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9965.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9889.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9805.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9714.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9616.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9511.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9400.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9284.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9162.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9036.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8906.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8773.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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HYDROGRAPH AT STATION LDCDB  
PLAN 1, RATIO = .76

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	2183.0	*	1		0320	41	540.	30.0	2185.6	*	1		0640	81	8420.	1173
1		0005	2	0.	.0	2183.0	*	1		0325	42	649.	37.2	2186.0	*	1		0645	82	8581.	1218
1		0010	3	2.	.0	2183.0	*	1		0330	43	776.	46.2	2186.2	*	1		0650	83	8739.	1262
1		0015	4	8.	.2	2183.1	*	1		0335	44	937.	57.5	2186.5	*	1		0655	84	8894.	1305
1		0020	5	20.	.4	2183.2	*	1		0340	45	1129.	71.1	2186.8	*	1		0700	85	9044.	1347
1		0025	6	38.	.8	2183.3	*	1		0345	46	1340.	86.0	2187.2	*	1		0705	86	9188.	1387
1		0030	7	61.	1.3	2183.5	*	1		0350	47	1550.	100.9	2187.5	*	1		0710	87	9325.	1426
1		0035	8	84.	1.9	2183.7	*	1		0355	48	1749.	115.0	2187.9	*	1		0715	88	9456.	1462
1		0040	9	104.	2.3	2183.8	*	1		0400	49	1937.	128.5	2188.2	*	1		0720	89	9578.	1496
1		0045	10	117.	2.6	2183.9	*	1		0405	50	2119.	141.8	2188.4	*	1		0725	90	9692.	1528
1		0050	11	123.	2.7	2184.0	*	1		0410	51	2301.	155.2	2188.6	*	1		0730	91	9798.	1557
1		0055	12	122.	2.7	2184.0	*	1		0415	52	2481.	168.3	2188.9	*	1		0735	92	9894.	1584
1		0100	13	116.	2.6	2183.9	*	1		0420	53	2658.	181.3	2189.1	*	1		0740	93	9980.	1609
1		0105	14	108.	2.4	2183.9	*	1		0425	54	2838.	194.4	2189.4	*	1		0745	94	10057.	1630
1		0110	15	99.	2.2	2183.8	*	1		0430	55	3027.	208.3	2189.6	*	1		0750	95	10123.	1648
1		0115	16	90.	2.0	2183.7	*	1		0435	56	3235.	223.5	2189.9	*	1		0755	96	10178.	1664
1		0120	17	82.	1.8	2183.6	*	1		0440	57	3419.	241.1	2190.2	*	1		0800	97	10223.	1676
1		0125	18	76.	1.7	2183.6	*	1		0445	58	3609.	262.2	2190.6	*	1		0805	98	10256.	1685
1		0130	19	74.	1.6	2183.6	*	1		0450	59	3834.	287.2	2191.0	*	1		0810	99	10278.	1692
1		0135	20	75.	1.7	2183.6	*	1		0455	60	4094.	316.0	2191.6	*	1		0815	100	10289.	1695
1		0140	21	79.	1.8	2183.6	*	1		0500	61	4367.	348.0	2192.1	*	1		0820	101	10290.	1695
1		0145	22	88.	2.0	2183.7	*	1		0505	62	4610.	382.3	2192.7	*	1		0825	102	10280.	1692
1		0150	23	99.	2.2	2183.8	*	1		0510	63	4864.	418.0	2193.4	*	1		0830	103	10261.	1687
1		0155	24	112.	2.5	2183.9	*	1		0515	64	5123.	454.6	2194.0	*	1		0835	104	10232.	1679
1		0200	25	126.	2.8	2184.0	*	1		0520	65	5345.	491.6	2194.7	*	1		0840	105	10195.	1668
1		0205	26	132.	3.2	2184.0	*	1		0525	66	5570.	529.2	2195.3	*	1		0845	106	10149.	1656

1	0210	27	141.	3.8	2184.1	*	1	0530	67	5798.	567.2	2196.0	*	1	0850	107	10095.	1641
1	0215	28	151.	4.4	2184.1	*	1	0535	68	6002.	605.8	2196.6	*	1	0855	108	10034.	1623
1	0220	29	161.	5.1	2184.1	*	1	0540	69	6208.	645.2	2197.3	*	1	0900	109	9965.	1604
1	0225	30	172.	5.8	2184.2	*	1	0545	70	6418.	685.4	2197.9	*	1	0905	110	9889.	1583
1	0230	31	183.	6.5	2184.2	*	1	0550	71	6612.	726.4	2198.6	*	1	0910	111	9805.	1560
1	0235	32	193.	7.2	2184.3	*	1	0555	72	6809.	768.3	2199.3	*	1	0915	112	9714.	1534
1	0240	33	205.	8.0	2184.3	*	1	0600	73	7009.	811.2	2200.0	*	1	0920	113	9616.	1507
1	0245	34	219.	8.9	2184.4	*	1	0605	74	7194.	855.0	2200.7	*	1	0925	114	9511.	1478
1	0250	35	237.	10.1	2184.4	*	1	0610	75	7382.	899.6	2201.4	*	1	0930	115	9400.	1447
1	0255	36	260.	11.6	2184.5	*	1	0615	76	7569.	944.7	2202.2	*	1	0935	116	9284.	1414
1	0300	37	291.	13.7	2184.6	*	1	0620	77	7745.	990.3	2202.9	*	1	0940	117	9162.	1380
1	0305	38	332.	16.4	2184.8	*	1	0625	78	7922.	1036.1	2203.6	*	1	0945	118	9036.	1345
1	0310	39	385.	19.8	2185.0	*	1	0630	79	8093.	1082.0	2204.3	*	1	0950	119	8906.	1308
1	0315	40	453.	24.3	2185.2	*	1	0635	80	8257.	1127.8	2205.0	*	1	0955	120	8773.	1271

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PEAK FLOW (CFS)	TIME (HR)	(CFS) □ (INCHES) (AC-FT)	MAXIMUM AVERAGE FLOW			
			6-HR	24-HR	72-HR	9.92-HR □
10290.	8.33		7605.	4712.	4712.	4712.
			1.842	1.887	1.887	1.887
			3771.	3862.	3862.	3862.

PEAK STORAGE (AC-FT)	TIME (HR)	MAXIMUM AVERAGE STORAGE			
		6-HR	24-HR	72-HR	9.92-HR □
1695.	8.33	1071.	654.	654.	654.

PEAK STAGE (FEET)	TIME (HR)	MAXIMUM AVERAGE STAGE			
		6-HR	24-HR	72-HR	9.92-HR □
2213.83	8.33	2203.88	2196.16	2196.16	2196.16

CUMULATIVE AREA = 38.38 SQ MI

WARNING --- ROUTED OUTFLOW ( 8489.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8617.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8739.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8855.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8964.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9065.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9159.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9244.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9320.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9387.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9445.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9492.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9530.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9558.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9576.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9585.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9584.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9575.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9557.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9531.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9497.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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WARNING --- ROUTED OUTFLOW ( 9406.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9350.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9286.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9214.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9136.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9050.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8958.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8859.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8755.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8646.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8532.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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HYDROGRAPH AT STATION LDCDB  
PLAN 1, RATIO = .73

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	2183.0	*	1		0320	41	445.	23.7	2185.2	*	1		0640	81	7930.	1038
1		0005	2	0.	.0	2183.0	*	1		0325	42	538.	29.9	2185.6	*	1		0645	82	8079.	1078
1		0010	3	2.	.0	2183.0	*	1		0330	43	656.	37.7	2186.0	*	1		0650	83	8219.	1117
1		0015	4	7.	.2	2183.1	*	1		0335	44	796.	47.5	2186.2	*	1		0655	84	8356.	1155
1		0020	5	19.	.4	2183.1	*	1		0340	45	965.	59.5	2186.5	*	1		0700	85	8489.	1192
1		0025	6	36.	.8	2183.3	*	1		0345	46	1151.	72.6	2186.8	*	1		0705	86	8617.	1228
1		0030	7	57.	1.3	2183.5	*	1		0350	47	1336.	85.7	2187.2	*	1		0710	87	8739.	1262
1		0035	8	80.	1.8	2183.6	*	1		0355	48	1512.	98.1	2187.5	*	1		0715	88	8855.	1294
1		0040	9	99.	2.2	2183.8	*	1		0400	49	1680.	110.0	2187.8	*	1		0720	89	8964.	1325
1		0045	10	111.	2.5	2183.9	*	1		0405	50	1846.	121.9	2188.0	*	1		0725	90	9065.	1353
1		0050	11	117.	2.6	2183.9	*	1		0410	51	2010.	133.9	2188.2	*	1		0730	91	9159.	1379
1		0055	12	116.	2.6	2183.9	*	1		0415	52	2177.	146.0	2188.5	*	1		0735	92	9244.	1403
1		0100	13	110.	2.5	2183.9	*	1		0420	53	2345.	158.3	2188.7	*	1		0740	93	9320.	1424
1		0105	14	102.	2.3	2183.8	*	1		0425	54	2515.	170.8	2188.9	*	1		0745	94	9387.	1443
1		0110	15	93.	2.1	2183.7	*	1		0430	55	2692.	183.7	2189.2	*	1		0750	95	9445.	1459
1		0115	16	84.	1.9	2183.7	*	1		0435	56	2882.	197.6	2189.4	*	1		0755	96	9492.	1472
1		0120	17	76.	1.7	2183.6	*	1		0440	57	3094.	213.2	2189.7	*	1		0800	97	9530.	1483
1		0125	18	70.	1.6	2183.6	*	1		0445	58	3329.	231.1	2190.0	*	1		0805	98	9558.	1491
1		0130	19	66.	1.5	2183.5	*	1		0450	59	3519.	252.2	2190.4	*	1		0810	99	9576.	1496
1		0135	20	65.	1.5	2183.5	*	1		0455	60	3741.	276.8	2190.9	*	1		0815	100	9585.	1498
1		0140	21	68.	1.5	2183.5	*	1		0500	61	3991.	304.6	2191.4	*	1		0820	101	9584.	1498
1		0145	22	74.	1.7	2183.6	*	1		0505	62	4262.	334.6	2191.9	*	1		0825	102	9575.	1495
1		0150	23	83.	1.8	2183.7	*	1		0510	63	4494.	365.9	2192.5	*	1		0830	103	9557.	1490
1		0155	24	93.	2.1	2183.7	*	1		0515	64	4723.	398.1	2193.0	*	1		0835	104	9531.	1483
1		0200	25	105.	2.3	2183.8	*	1		0520	65	4956.	431.0	2193.6	*	1		0840	105	9497.	1473
1		0205	26	118.	2.6	2183.9	*	1		0525	66	5183.	464.6	2194.2	*	1		0845	106	9455.	1462
1		0210	27	129.	3.0	2184.0	*	1		0530	67	5388.	498.8	2194.8	*	1		0850	107	9406.	1448
1		0215	28	136.	3.4	2184.0	*	1		0535	68	5597.	533.7	2195.4	*	1		0855	108	9350.	1432
1		0220	29	143.	3.9	2184.1	*	1		0540	69	5810.	569.2	2196.0	*	1		0900	109	9286.	1415
1		0225	30	151.	4.4	2184.1	*	1		0545	70	6000.	605.5	2196.6	*	1		0905	110	9214.	1395
1		0230	31	158.	4.9	2184.1	*	1		0550	71	6193.	642.4	2197.2	*	1		0910	111	9136.	1373
1		0235	32	165.	5.4	2184.1	*	1		0555	72	6389.	679.9	2197.8	*	1		0915	112	9050.	1349
1		0240	33	173.	5.9	2184.2	*	1		0600	73	6573.	718.0	2198.5	*	1		0920	113	8958.	1323
1		0245	34	183.	6.5	2184.2	*	1		0605	74	6755.	756.8	2199.1	*	1		0925	114	8859.	1295
1		0250	35	196.	7.4	2184.3	*	1		0610	75	6939.	796.1	2199.8	*	1		0930	115	8755.	1266
1		0255	36	214.	8.6	2184.3	*	1		0615	76	7114.	836.0	2200.4	*	1		0935	116	8646.	1236
1		0300	37	238.	10.2	2184.4	*	1		0620	77	7284.	876.2	2201.1	*	1		0940	117	8532.	1204
1		0305	38	271.	12.3	2184.5	*	1		0625	78	7455.	916.7	2201.7	*	1		0945	118	8415.	1171
1		0310	39	314.	15.2	2184.7	*	1		0630	79	7618.	957.3	2202.4	*	1		0950	119	8294.	1138
1		0315	40	371.	18.9	2184.9	*	1		0635	80	7774.	997.8	2203.0	*	1		0955	120	8172.	1103

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PEAK FLOW TIME

MAXIMUM AVERAGE FLOW  
6-HR 24-HR 72-HR 9.92-HR

(CFS)	(HR)	(CFS)□				
9585.	8.25		7082.	4380.	4380.	4380.
		(INCHES)	1.716	1.754	1.754	1.754
		(AC-FT)	3512.	3590.	3590.	3590.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE		
(AC-FT)	(HR)	6-HR	24-HR	72-HR	9.92-HR□
1498.	8.25	945.	576.	576.	576.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE		
(FEET)	(HR)	6-HR	24-HR	72-HR	9.92-HR□
2210.78	8.25	2201.88	2194.91	2194.91	2194.91

CUMULATIVE AREA = 38.38 SQ MI

WARNING --- ROUTED OUTFLOW ( 8518.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8609.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8693.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8770.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8838.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8898.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8949.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8991.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9024.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9048.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9064.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9071.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9070.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9061.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9044.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 9020.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8988.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8949.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8902.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8849.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8787.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8719.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8644.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

WARNING --- ROUTED OUTFLOW ( 8562.) IS GREATER THAN MAXIMUM OUTFLOW ( 8480.) IN STORAGE-OUTFLOW TABLE

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HYDROGRAPH AT STATION LDCDB  
PLAN 1, RATIO = .69

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1	0000	1	0.	.0	2183.0	*	1	0320	41	381.	19.6	2185.0	*	1	0640	81	7558.	941			
1	0005	2	0.	.0	2183.0	*	1	0325	42	464.	25.0	2185.3	*	1	0645	82	7697.	977			
1	0010	3	2.	.0	2183.0	*	1	0330	43	569.	31.9	2185.7	*	1	0650	83	7833.	1013			

1	0015	4	7.	.2	2183.1	*	1	0335	44	700.	40.8	2186.1	*	1	0655	84	7967.	1047
1	0020	5	18.	.4	2183.1	*	1	0340	45	853.	51.5	2186.3	*	1	0700	85	8090.	1081
1	0025	6	35.	.8	2183.3	*	1	0345	46	1021.	63.4	2186.6	*	1	0705	86	8205.	1113
1	0030	7	55.	1.2	2183.4	*	1	0350	47	1188.	75.2	2186.9	*	1	0710	87	8315.	1144
1	0035	8	76.	1.7	2183.6	*	1	0355	48	1346.	86.4	2187.2	*	1	0715	88	8420.	1173
1	0040	9	95.	2.1	2183.8	*	1	0400	49	1497.	97.1	2187.4	*	1	0720	89	8518.	1200
1	0045	10	107.	2.4	2183.8	*	1	0405	50	1648.	107.8	2187.7	*	1	0725	90	8609.	1226
1	0050	11	112.	2.5	2183.9	*	1	0410	51	1801.	118.6	2188.0	*	1	0730	91	8693.	1249
1	0055	12	111.	2.5	2183.9	*	1	0415	52	1955.	129.8	2188.2	*	1	0735	92	8770.	1270
1	0100	13	106.	2.3	2183.8	*	1	0420	53	2113.	141.4	2188.4	*	1	0740	93	8838.	1290
1	0105	14	98.	2.2	2183.8	*	1	0425	54	2276.	153.3	2188.6	*	1	0745	94	8898.	1306
1	0110	15	89.	2.0	2183.7	*	1	0430	55	2446.	165.8	2188.8	*	1	0750	95	8949.	1320
1	0115	16	80.	1.8	2183.6	*	1	0435	56	2626.	178.9	2189.1	*	1	0755	96	8991.	1332
1	0120	17	73.	1.6	2183.6	*	1	0440	57	2824.	193.4	2189.3	*	1	0800	97	9024.	1341
1	0125	18	66.	1.5	2183.5	*	1	0445	58	3048.	209.8	2189.6	*	1	0805	98	9048.	1348
1	0130	19	62.	1.4	2183.5	*	1	0450	59	3304.	228.5	2190.0	*	1	0810	99	9064.	1353
1	0135	20	60.	1.3	2183.5	*	1	0455	60	3500.	250.1	2190.4	*	1	0815	100	9071.	1354
1	0140	21	62.	1.4	2183.5	*	1	0500	61	3720.	274.5	2190.8	*	1	0820	101	9070.	1354
1	0145	22	66.	1.5	2183.5	*	1	0505	62	3961.	301.3	2191.3	*	1	0825	102	9061.	1352
1	0150	23	73.	1.6	2183.6	*	1	0510	63	4214.	329.4	2191.8	*	1	0830	103	9044.	1347
1	0155	24	81.	1.8	2183.6	*	1	0515	64	4439.	358.2	2192.3	*	1	0835	104	9020.	1340
1	0200	25	91.	2.0	2183.7	*	1	0520	65	4649.	387.8	2192.8	*	1	0840	105	8988.	1331
1	0205	26	102.	2.3	2183.8	*	1	0525	66	4864.	418.1	2193.4	*	1	0845	106	8949.	1320
1	0210	27	115.	2.6	2183.9	*	1	0530	67	5085.	449.2	2193.9	*	1	0850	107	8902.	1307
1	0215	28	127.	2.9	2184.0	*	1	0535	68	5282.	481.1	2194.5	*	1	0855	108	8849.	1292
1	0220	29	133.	3.2	2184.0	*	1	0540	69	5478.	513.9	2195.0	*	1	0900	109	8787.	1275
1	0225	30	138.	3.6	2184.0	*	1	0545	70	5679.	547.3	2195.6	*	1	0905	110	8719.	1256
1	0230	31	143.	3.9	2184.1	*	1	0550	71	5874.	581.2	2196.2	*	1	0910	111	8644.	1235
1	0235	32	147.	4.2	2184.1	*	1	0555	72	6054.	615.8	2196.8	*	1	0915	112	8562.	1212
1	0240	33	153.	4.6	2184.1	*	1	0600	73	6238.	650.9	2197.4	*	1	0920	113	8474.	1188
1	0245	34	160.	5.0	2184.1	*	1	0605	74	6424.	686.5	2198.0	*	1	0925	114	8380.	1162
1	0250	35	170.	5.7	2184.2	*	1	0610	75	6593.	722.4	2198.6	*	1	0930	115	8281.	1134
1	0255	36	184.	6.6	2184.2	*	1	0615	76	6763.	758.7	2199.2	*	1	0935	116	8177.	1105
1	0300	37	204.	7.9	2184.3	*	1	0620	77	6935.	795.2	2199.8	*	1	0940	117	8069.	1075
1	0305	38	231.	9.7	2184.4	*	1	0625	78	7096.	831.9	2200.4	*	1	0945	118	7954.	1044
1	0310	39	268.	12.1	2184.5	*	1	0630	79	7252.	868.6	2200.9	*	1	0950	119	7831.	1012
1	0315	40	317.	15.4	2184.7	*	1	0635	80	7407.	905.3	2201.5	*	1	0955	120	7707.	980

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PEAK FLOW	TIME		6-HR	24-HR	72-HR	9.92-HR
(CFS)	(HR)					
9071.	8.25	(CFS) □	6693.	4134.	4134.	4134.
		(INCHES)	1.621	1.655	1.655	1.655
		(AC-FT)	3319.	3388.	3388.	3388.

PEAK STORAGE	TIME		6-HR	24-HR	72-HR	9.92-HR
(AC-FT)	(HR)					
1355.	8.25		853.	520.	520.	520.

PEAK STAGE	TIME		6-HR	24-HR	72-HR	9.92-HR
(FEET)	(HR)					
2208.56	8.25		2200.42	2193.99	2193.99	2193.99

CUMULATIVE AREA = 38.38 SQ MI

HYDROGRAPH AT STATION LDCDB  
PLAN 1, RATIO = .64

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	2183.0	*	1		0320	41	284.	13.2	2184.6	*	1		0640	81	6841.	775
1		0005	2	0.	.0	2183.0	*	1		0325	42	348.	17.4	2184.8	*	1		0645	82	6979.	804
1		0010	3	2.	.0	2183.0	*	1		0330	43	432.	22.9	2185.2	*	1		0650	83	7103.	833
1		0015	4	6.	.1	2183.1	*	1		0335	44	540.	30.0	2185.6	*	1		0655	84	7222.	861
1		0020	5	16.	.4	2183.1	*	1		0340	45	671.	38.7	2186.0	*	1		0700	85	7337.	888
1		0025	6	32.	.7	2183.3	*	1		0345	46	808.	48.4	2186.3	*	1		0705	86	7447.	914
1		0030	7	51.	1.1	2183.4	*	1		0350	47	943.	58.0	2186.5	*	1		0710	87	7551.	939
1		0035	8	70.	1.6	2183.6	*	1		0355	48	1071.	67.0	2186.7	*	1		0715	88	7642.	963
1		0040	9	87.	1.9	2183.7	*	1		0400	49	1193.	75.6	2186.9	*	1		0720	89	7727.	985



1	0045	10	98.	2.2	2183.8	*	1	0405	50	1314.	84.2	2187.1	*	1	0725	90	7807.	1006
1	0050	11	103.	2.3	2183.8	*	1	0410	51	1438.	92.9	2187.3	*	1	0730	91	7879.	1025
1	0055	12	102.	2.3	2183.8	*	1	0415	52	1567.	102.1	2187.6	*	1	0735	92	7944.	1041
1	0100	13	97.	2.2	2183.8	*	1	0420	53	1704.	111.8	2187.8	*	1	0740	93	8002.	1056
1	0105	14	90.	2.0	2183.7	*	1	0425	54	1851.	122.2	2188.0	*	1	0745	94	8049.	1069
1	0110	15	82.	1.8	2183.7	*	1	0430	55	2006.	133.5	2188.2	*	1	0750	95	8089.	1080
1	0115	16	74.	1.6	2183.6	*	1	0435	56	2172.	145.7	2188.5	*	1	0755	96	8121.	1089
1	0120	17	67.	1.5	2183.5	*	1	0440	57	2351.	158.8	2188.7	*	1	0800	97	8147.	1097
1	0125	18	61.	1.3	2183.5	*	1	0445	58	2547.	173.1	2189.0	*	1	0805	98	8165.	1102
1	0130	19	56.	1.2	2183.4	*	1	0450	59	2764.	189.0	2189.3	*	1	0810	99	8177.	1105
1	0135	20	54.	1.2	2183.4	*	1	0455	60	3004.	206.6	2189.6	*	1	0815	100	8182.	1106
1	0140	21	53.	1.2	2183.4	*	1	0500	61	3265.	225.7	2189.9	*	1	0820	101	8179.	1106
1	0145	22	55.	1.2	2183.4	*	1	0505	62	3465.	246.2	2190.3	*	1	0825	102	8170.	1103
1	0150	23	59.	1.3	2183.5	*	1	0510	63	3662.	268.0	2190.7	*	1	0830	103	8153.	1098
1	0155	24	64.	1.4	2183.5	*	1	0515	64	3868.	290.9	2191.1	*	1	0835	104	8130.	1092
1	0200	25	70.	1.5	2183.6	*	1	0520	65	4081.	314.6	2191.5	*	1	0840	105	8100.	1083
1	0205	26	77.	1.7	2183.6	*	1	0525	66	4300.	338.9	2192.0	*	1	0845	106	8062.	1073
1	0210	27	87.	1.9	2183.7	*	1	0530	67	4480.	363.9	2192.4	*	1	0850	107	8018.	1060
1	0215	28	97.	2.2	2183.8	*	1	0535	68	4664.	389.9	2192.9	*	1	0855	108	7962.	1046
1	0220	29	107.	2.4	2183.8	*	1	0540	69	4855.	416.8	2193.3	*	1	0900	109	7900.	1030
1	0225	30	114.	2.5	2183.9	*	1	0545	70	5052.	444.5	2193.8	*	1	0905	110	7831.	1012
1	0230	31	119.	2.7	2183.9	*	1	0550	71	5232.	472.9	2194.3	*	1	0910	111	7756.	993
1	0235	32	123.	2.7	2184.0	*	1	0555	72	5407.	502.0	2194.8	*	1	0915	112	7675.	972
1	0240	33	127.	2.8	2184.0	*	1	0600	73	5585.	531.7	2195.3	*	1	0920	113	7588.	949
1	0245	34	129.	3.0	2184.0	*	1	0605	74	5766.	561.8	2195.9	*	1	0925	114	7494.	925
1	0250	35	134.	3.3	2184.0	*	1	0610	75	5931.	592.3	2196.4	*	1	0930	115	7388.	900
1	0255	36	142.	3.9	2184.1	*	1	0615	76	6092.	622.9	2196.9	*	1	0935	116	7279.	875
1	0300	37	154.	4.7	2184.1	*	1	0620	77	6252.	653.7	2197.4	*	1	0940	117	7167.	848
1	0305	38	173.	5.9	2184.2	*	1	0625	78	6413.	684.4	2197.9	*	1	0945	118	7052.	821
1	0310	39	199.	7.6	2184.3	*	1	0630	79	6558.	714.9	2198.4	*	1	0950	119	6927.	793
1	0315	40	236.	10.0	2184.4	*	1	0635	80	6700.	745.2	2198.9	*	1	0955	120	6796.	765

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PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	9.92-HR□
8182.	8.25	(CFS)□	5982.	3687.	3687.	3687.
		(INCHES)	1.449	1.476	1.476	1.476
		(AC-FT)	2966.	3022.	3022.	3022.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	9.92-HR□
1107.	8.25		693.	423.	423.	423.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	9.92-HR□
2204.71	8.25		2197.86	2192.38	2192.38	2192.38

CUMULATIVE AREA = 38.38 SQ MI

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HYDROGRAPH AT STATION LDCDB  
PLAN 1, RATIO = .58

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	2183.0	*	1		0320	41	208.	8.2	2184.3	*	1		0640	81	5994.	604
1		0005	2	0.	.0	2183.0	*	1		0325	42	254.	11.2	2184.5	*	1		0645	82	6119.	628
1		0010	3	2.	.0	2183.0	*	1		0330	43	316.	15.3	2184.7	*	1		0650	83	6241.	651
1		0015	4	6.	.1	2183.0	*	1		0335	44	397.	20.6	2185.0	*	1		0655	84	6360.	674
1		0020	5	15.	.3	2183.1	*	1		0340	45	500.	27.4	2185.4	*	1		0700	85	6470.	696
1		0025	6	29.	.6	2183.2	*	1		0345	46	613.	34.8	2185.8	*	1		0705	86	6568.	716
1		0030	7	46.	1.0	2183.4	*	1		0350	47	720.	42.2	2186.1	*	1		0710	87	6660.	736
1		0035	8	64.	1.4	2183.5	*	1		0355	48	817.	49.1	2186.3	*	1		0715	88	6748.	755
1		0040	9	79.	1.8	2183.6	*	1		0400	49	910.	55.6	2186.4	*	1		0720	89	6829.	772
1		0045	10	89.	2.0	2183.7	*	1		0405	50	1001.	62.0	2186.6	*	1		0725	90	6903.	788
1		0050	11	93.	2.1	2183.7	*	1		0410	51	1094.	68.6	2186.8	*	1		0730	91	6971.	802
1		0055	12	93.	2.1	2183.7	*	1		0415	52	1192.	75.5	2186.9	*	1		0735	92	7029.	815
1		0100	13	88.	2.0	2183.7	*	1		0420	53	1296.	82.9	2187.1	*	1		0740	93	7077.	827
1		0105	14	82.	1.8	2183.6	*	1		0425	54	1411.	91.1	2187.3	*	1		0745	94	7119.	837
1		0110	15	74.	1.7	2183.6	*	1		0430	55	1541.	100.2	2187.5	*	1		0750	95	7154.	845

1	0115	16	67.	1.5	2183.5	*	1	0435	56	1688.	110.6	2187.8	*	1	0755	96	7182.	852
1	0120	17	60.	1.3	2183.5	*	1	0440	57	1851.	122.2	2188.0	*	1	0800	97	7204.	857
1	0125	18	55.	1.2	2183.4	*	1	0445	58	2027.	135.1	2188.3	*	1	0805	98	7218.	860
1	0130	19	51.	1.1	2183.4	*	1	0450	59	2217.	149.0	2188.5	*	1	0810	99	7226.	862
1	0135	20	48.	1.1	2183.4	*	1	0455	60	2421.	163.9	2188.8	*	1	0815	100	7225.	862
1	0140	21	47.	1.1	2183.4	*	1	0500	61	2635.	179.6	2189.1	*	1	0820	101	7218.	860
1	0145	22	48.	1.1	2183.4	*	1	0505	62	2856.	195.8	2189.4	*	1	0825	102	7203.	857
1	0150	23	49.	1.1	2183.4	*	1	0510	63	3079.	212.1	2189.7	*	1	0830	103	7181.	851
1	0155	24	52.	1.1	2183.4	*	1	0515	64	3302.	228.4	2190.0	*	1	0835	104	7151.	844
1	0200	25	54.	1.2	2183.4	*	1	0520	65	3455.	245.1	2190.3	*	1	0840	105	7115.	836
1	0205	26	58.	1.3	2183.5	*	1	0525	66	3613.	262.6	2190.6	*	1	0845	106	7071.	825
1	0210	27	64.	1.4	2183.5	*	1	0530	67	3780.	281.2	2190.9	*	1	0850	107	7021.	814
1	0215	28	70.	1.6	2183.6	*	1	0535	68	3956.	300.8	2191.3	*	1	0855	108	6960.	800
1	0220	29	76.	1.7	2183.6	*	1	0540	69	4141.	321.2	2191.7	*	1	0900	109	6891.	785
1	0225	30	80.	1.8	2183.6	*	1	0545	70	4327.	342.4	2192.0	*	1	0905	110	6815.	769
1	0230	31	83.	1.8	2183.7	*	1	0550	71	4482.	364.3	2192.4	*	1	0910	111	6733.	752
1	0235	32	84.	1.9	2183.7	*	1	0555	72	4643.	386.9	2192.8	*	1	0915	112	6646.	733
1	0240	33	86.	1.9	2183.7	*	1	0600	73	4807.	410.1	2193.2	*	1	0920	113	6554.	714
1	0245	34	89.	2.0	2183.7	*	1	0605	74	4975.	433.7	2193.6	*	1	0925	114	6458.	693
1	0250	35	95.	2.1	2183.8	*	1	0610	75	5141.	457.7	2194.1	*	1	0930	115	6350.	672
1	0255	36	106.	2.4	2183.8	*	1	0615	76	5287.	482.0	2194.5	*	1	0935	116	6237.	650
1	0300	37	124.	2.7	2184.0	*	1	0620	77	5434.	506.5	2194.9	*	1	0940	117	6121.	628
1	0305	38	135.	3.4	2184.0	*	1	0625	78	5581.	531.1	2195.3	*	1	0945	118	6003.	605
1	0310	39	151.	4.5	2184.1	*	1	0630	79	5729.	555.6	2195.8	*	1	0950	119	5883.	582
1	0315	40	175.	6.0	2184.2	*	1	0635	80	5868.	580.1	2196.2	*	1	0955	120	5754.	559

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PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	9.92-HR
7226.	8.17	(CFS)□	5180.	3186.	3186.	3186.
		(INCHES)	1.255	1.276	1.276	1.276
		(AC-FT)	2568.	2611.	2611.	2611.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	9.92-HR
862.	8.17		535.	326.	326.	326.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	9.92-HR
2200.85	8.17		2195.25	2190.73	2190.73	2190.73

CUMULATIVE AREA = 38.38 SQ MI

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HYDROGRAPH AT STATION LDCDB  
PLAN 1, RATIO = .53

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	2183.0	*	1		0320	41	165.	5.3	2184.1	*	1		0640	81	5215.	470
1		0005	2	0.	.0	2183.0	*	1		0325	42	197.	7.5	2184.3	*	1		0645	82	5326.	488
1		0010	3	1.	.0	2183.0	*	1		0330	43	243.	10.5	2184.4	*	1		0650	83	5435.	506
1		0015	4	5.	.1	2183.0	*	1		0335	44	305.	14.6	2184.7	*	1		0655	84	5541.	524
1		0020	5	14.	.3	2183.1	*	1		0340	45	384.	19.8	2185.0	*	1		0700	85	5643.	541
1		0025	6	26.	.6	2183.2	*	1		0345	46	473.	25.6	2185.3	*	1		0705	86	5741.	557
1		0030	7	42.	.9	2183.3	*	1		0350	47	560.	31.3	2185.6	*	1		0710	87	5831.	573
1		0035	8	58.	1.3	2183.5	*	1		0355	48	641.	36.6	2185.9	*	1		0715	88	5907.	587
1		0040	9	72.	1.6	2183.6	*	1		0400	49	712.	41.6	2186.1	*	1		0720	89	5977.	601
1		0045	10	81.	1.8	2183.6	*	1		0405	50	781.	46.4	2186.2	*	1		0725	90	6042.	613
1		0050	11	85.	1.9	2183.7	*	1		0410	51	851.	51.4	2186.3	*	1		0730	91	6102.	624
1		0055	12	85.	1.9	2183.7	*	1		0415	52	924.	56.6	2186.5	*	1		0735	92	6155.	635
1		0100	13	81.	1.8	2183.6	*	1		0420	53	1003.	62.2	2186.6	*	1		0740	93	6201.	643
1		0105	14	75.	1.7	2183.6	*	1		0425	54	1091.	68.4	2186.7	*	1		0745	94	6240.	651
1		0110	15	68.	1.5	2183.5	*	1		0430	55	1191.	75.5	2186.9	*	1		0750	95	6273.	657
1		0115	16	61.	1.4	2183.5	*	1		0435	56	1308.	83.7	2187.1	*	1		0755	96	6297.	662
1		0120	17	55.	1.2	2183.4	*	1		0440	57	1443.	93.3	2187.3	*	1		0800	97	6315.	665
1		0125	18	50.	1.1	2183.4	*	1		0445	58	1599.	104.3	2187.6	*	1		0805	98	6324.	667
1		0130	19	46.	1.0	2183.4	*	1		0450	59	1772.	116.6	2187.9	*	1		0810	99	6326.	667
1		0135	20	44.	1.0	2183.3	*	1		0455	60	1955.	129.8	2188.2	*	1		0815	100	6321.	666
1		0140	21	43.	1.0	2183.3	*	1		0500	61	2143.	143.6	2188.4	*	1		0820	101	6308.	664

1	0145	22	44.	1.0	2183.3	*	1	0505	62	2331.	157.4	2188.7	*	1	0825	102	6287.	660
1	0150	23	45.	1.0	2183.4	*	1	0510	63	2517.	170.9	2188.9	*	1	0830	103	6259.	655
1	0155	24	46.	1.0	2183.4	*	1	0515	64	2698.	184.2	2189.2	*	1	0835	104	6224.	648
1	0200	25	47.	1.1	2183.4	*	1	0520	65	2877.	197.3	2189.4	*	1	0840	105	6182.	640
1	0205	26	49.	1.1	2183.4	*	1	0525	66	3056.	210.4	2189.7	*	1	0845	106	6133.	630
1	0210	27	52.	1.1	2183.4	*	1	0530	67	3239.	223.8	2189.9	*	1	0850	107	6078.	620
1	0215	28	55.	1.2	2183.4	*	1	0535	68	3388.	237.6	2190.2	*	1	0855	108	6016.	608
1	0220	29	58.	1.3	2183.5	*	1	0540	69	3520.	252.3	2190.4	*	1	0900	109	5948.	595
1	0225	30	60.	1.3	2183.5	*	1	0545	70	3661.	267.9	2190.7	*	1	0905	110	5875.	581
1	0230	31	61.	1.3	2183.5	*	1	0550	71	3810.	284.5	2191.0	*	1	0910	111	5794.	566
1	0235	32	61.	1.3	2183.5	*	1	0555	72	3965.	301.7	2191.3	*	1	0915	112	5700.	550
1	0240	33	61.	1.3	2183.5	*	1	0600	73	4126.	319.6	2191.6	*	1	0920	113	5602.	534
1	0245	34	62.	1.4	2183.5	*	1	0605	74	4290.	337.7	2192.0	*	1	0925	114	5502.	517
1	0250	35	65.	1.4	2183.5	*	1	0610	75	4425.	356.3	2192.3	*	1	0930	115	5399.	500
1	0255	36	71.	1.6	2183.6	*	1	0615	76	4559.	375.1	2192.6	*	1	0935	116	5292.	482
1	0300	37	83.	1.8	2183.7	*	1	0620	77	4694.	394.1	2193.0	*	1	0940	117	5184.	464
1	0305	38	101.	2.3	2183.8	*	1	0625	78	4830.	413.2	2193.3	*	1	0945	118	5065.	446
1	0310	39	127.	2.9	2184.0	*	1	0630	79	4965.	432.3	2193.6	*	1	0950	119	4934.	427
1	0315	40	142.	3.8	2184.1	*	1	0635	80	5100.	451.2	2194.0	*	1	0955	120	4803.	409

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PEAK FLOW	TIME		6-HR	24-HR	72-HR	9.92-HR
(CFS)	(HR)					
6326.	8.17	(CFS)	4452.	2735.	2735.	2735.
		(INCHES)	1.079	1.095	1.095	1.095
		(AC-FT)	2208.	2241.	2241.	2241.

PEAK STORAGE	TIME		6-HR	24-HR	72-HR	9.92-HR
(AC-FT)	(HR)					
668.	8.17		412.	251.	251.	251.

PEAK STAGE	TIME		6-HR	24-HR	72-HR	9.92-HR
(FEET)	(HR)					
2197.64	8.17		2193.15	2189.41	2189.41	2189.41

CUMULATIVE AREA = 38.38 SQ MI

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 872 KK \* MCDB \*  
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879 KO OUTPUT CONTROL VARIABLES  
 IPRNT 1 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE

## HYDROGRAPH ROUTING DATA

875 RS STORAGE ROUTING  
 NSTPS 1 NUMBER OF SUBREACHES  
 ITYP STOR TYPE OF INITIAL CONDITION  
 RSVRIC -1.00 INITIAL CONDITION  
 X .00 WORKING R AND D COEFFICIENT

876 SV	STORAGE	.0	8.0	40.0	70.0	100.0	160.0	210.0	500.0
877 SQ	DISCHARGE	0.	30.	40.	55.	65.	80.	85.	87.
878 SE	ELEVATION	.00	2.00	4.00	6.00	8.00	12.00	15.00	16.00

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HYDROGRAPH AT STATION MCDB  
 PLAN 1, RATIO = .83

*****																						
*****							*	*****							*	*****						
DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA	
1		0000	1	0.	.0	.0	*	1		0320	41	38.	32.1	3.5	*	1		0640	81	76.	142	
1		0005	2	0.	.0	.0	*	1		0325	42	39.	38.2	3.9	*	1		0645	82	75.	141	
1		0010	3	0.	.0	.0	*	1		0330	43	43.	45.8	4.4	*	1		0650	83	75.	141	
1		0015	4	0.	.0	.0	*	1		0335	44	48.	55.1	5.0	*	1		0655	84	75.	140	
1		0020	5	0.	.0	.0	*	1		0340	45	53.	65.2	5.7	*	1		0700	85	75.	140	
1		0025	6	0.	.1	.0	*	1		0345	46	57.	74.6	6.3	*	1		0705	86	75.	139	
1		0030	7	1.	.2	.1	*	1		0350	47	59.	82.4	6.8	*	1		0710	87	75.	139	
1		0035	8	2.	.5	.1	*	1		0355	48	61.	88.8	7.3	*	1		0715	88	75.	138	
1		0040	9	3.	.9	.2	*	1		0400	49	63.	94.2	7.6	*	1		0720	89	75.	138	
1		0045	10	5.	1.3	.3	*	1		0405	50	65.	98.7	7.9	*	1		0725	90	74.	137	
1		0050	11	6.	1.7	.4	*	1		0410	51	66.	102.4	8.2	*	1		0730	91	74.	137	
1		0055	12	8.	2.1	.5	*	1		0415	52	66.	105.2	8.3	*	1		0735	92	74.	136	
1		0100	13	9.	2.4	.6	*	1		0420	53	67.	107.4	8.5	*	1		0740	93	74.	136	
1		0105	14	10.	2.7	.7	*	1		0425	54	67.	109.3	8.6	*	1		0745	94	74.	135	
1		0110	15	11.	3.0	.8	*	1		0430	55	68.	111.2	8.7	*	1		0750	95	74.	135	
1		0115	16	12.	3.3	.8	*	1		0435	56	68.	113.3	8.9	*	1		0755	96	74.	134	
1		0120	17	13.	3.5	.9	*	1		0440	57	69.	116.1	9.1	*	1		0800	97	74.	134	
1		0125	18	14.	3.8	1.0	*	1		0445	58	70.	119.7	9.3	*	1		0805	98	73.	133	
1		0130	19	16.	4.2	1.1	*	1		0450	59	71.	124.0	9.6	*	1		0810	99	73.	133	
1		0135	20	18.	4.8	1.2	*	1		0455	60	72.	128.8	9.9	*	1		0815	100	73.	132	
1		0140	21	20.	5.4	1.4	*	1		0500	61	73.	133.5	10.2	*	1		0820	101	73.	132	
1		0145	22	23.	6.2	1.6	*	1		0505	62	74.	137.6	10.5	*	1		0825	102	73.	131	
1		0150	23	26.	7.0	1.8	*	1		0510	63	75.	140.6	10.7	*	1		0830	103	73.	131	
1		0155	24	30.	7.9	2.0	*	1		0515	64	76.	142.7	10.8	*	1		0835	104	73.	130	
1		0200	25	30.	8.8	2.0	*	1		0520	65	76.	144.1	10.9	*	1		0840	105	73.	130	
1		0205	26	31.	9.7	2.1	*	1		0525	66	76.	144.9	11.0	*	1		0845	106	72.	129	
1		0210	27	31.	10.8	2.2	*	1		0530	67	76.	145.4	11.0	*	1		0850	107	72.	129	
1		0215	28	31.	11.9	2.2	*	1		0535	68	76.	145.7	11.0	*	1		0855	108	72.	128	
1		0220	29	32.	12.8	2.3	*	1		0540	69	76.	145.8	11.1	*	1		0900	109	72.	128	
1		0225	30	32.	13.6	2.3	*	1		0545	70	76.	145.8	11.1	*	1		0905	110	72.	127	
1		0230	31	32.	14.1	2.4	*	1		0550	71	76.	145.7	11.0	*	1		0910	111	72.	127	
1		0235	32	32.	14.6	2.4	*	1		0555	72	76.	145.6	11.0	*	1		0915	112	72.	126	
1		0240	33	32.	15.0	2.4	*	1		0600	73	76.	145.4	11.0	*	1		0920	113	72.	126	
1		0245	34	32.	15.6	2.5	*	1		0605	74	76.	145.2	11.0	*	1		0925	114	71.	125	
1		0250	35	33.	16.2	2.5	*	1		0610	75	76.	145.0	11.0	*	1		0930	115	71.	125	
1		0255	36	33.	17.2	2.6	*	1		0615	76	76.	144.6	11.0	*	1		0935	116	71.	124	
1		0300	37	33.	18.7	2.7	*	1		0620	77	76.	144.3	11.0	*	1		0940	117	71.	124	
1		0305	38	34.	20.8	2.8	*	1		0625	78	76.	143.8	10.9	*	1		0945	118	71.	123	
1		0310	39	35.	23.6	3.0	*	1		0630	79	76.	143.4	10.9	*	1		0950	119	71.	123	
1		0315	40	36.	27.4	3.2	*	1		0635	80	76.	142.9	10.9	*	1		0955	120	71.	122	
*****							*	*****							*	*****						

PEAK FLOW	TIME	MAXIMUM AVERAGE FLOW			
(CFS)	(HR)	6-HR	24-HR	72-HR	9.92-HR
76.	5.67	73.	53.	53.	53.
(INCHES)		.242	.293	.293	.293
(AC-FT)		36.	44.	44.	44.

PEAK STORAGE	TIME	MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)	6-HR	24-HR	72-HR	9.92-HR
146.	5.67	132.	86.	86.	86.

PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			
(FEET)	(HR)	6-HR	24-HR	72-HR	9.92-HR
11.05	5.67	10.12	6.91	6.91	6.91

CUMULATIVE AREA = 2.80 SQ MI

HYDROGRAPH AT STATION MCDB  
PLAN 1, RATIO = .80

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1	0000	1	0.	.0	.0	*	1	0320	41	37.	29.7	3.4	*	1	0640	81	74.	134			

1	0005	2	0.	.0	.0 *	1	0325	42	39.	35.5	3.7 *	1	0645	82	74.	134
1	0010	3	0.	.0	.0 *	1	0330	43	41.	42.7	4.2 *	1	0650	83	73.	133
1	0015	4	0.	.0	.0 *	1	0335	44	46.	51.6	4.8 *	1	0655	84	73.	133
1	0020	5	0.	.0	.0 *	1	0340	45	51.	61.2	5.4 *	1	0700	85	73.	132
1	0025	6	0.	.0	.0 *	1	0345	46	55.	70.2	6.0 *	1	0705	86	73.	132
1	0030	7	1.	.2	.0 *	1	0350	47	58.	77.7	6.5 *	1	0710	87	73.	131
1	0035	8	2.	.4	.1 *	1	0355	48	60.	83.8	6.9 *	1	0715	88	73.	131
1	0040	9	3.	.7	.2 *	1	0400	49	61.	88.9	7.3 *	1	0720	89	73.	130
1	0045	10	4.	1.1	.3 *	1	0405	50	63.	93.2	7.5 *	1	0725	90	73.	130
1	0050	11	6.	1.5	.4 *	1	0410	51	64.	96.7	7.8 *	1	0730	91	72.	129
1	0055	12	7.	1.8	.5 *	1	0415	52	65.	99.4	8.0 *	1	0735	92	72.	129
1	0100	13	8.	2.1	.5 *	1	0420	53	65.	101.5	8.1 *	1	0740	93	72.	128
1	0105	14	9.	2.4	.6 *	1	0425	54	66.	103.3	8.2 *	1	0745	94	72.	128
1	0110	15	10.	2.6	.7 *	1	0430	55	66.	105.1	8.3 *	1	0750	95	72.	127
1	0115	16	11.	2.9	.7 *	1	0435	56	67.	107.1	8.5 *	1	0755	96	72.	127
1	0120	17	12.	3.1	.8 *	1	0440	57	67.	109.8	8.7 *	1	0800	97	72.	126
1	0125	18	13.	3.4	.9 *	1	0445	58	68.	113.2	8.9 *	1	0805	98	72.	126
1	0130	19	14.	3.8	.9 *	1	0450	59	69.	117.4	9.2 *	1	0810	99	71.	125
1	0135	20	16.	4.3	1.1 *	1	0455	60	70.	122.0	9.5 *	1	0815	100	71.	125
1	0140	21	18.	4.9	1.2 *	1	0500	61	72.	126.5	9.8 *	1	0820	101	71.	124
1	0145	22	21.	5.6	1.4 *	1	0505	62	73.	130.4	10.0 *	1	0825	102	71.	124
1	0150	23	24.	6.3	1.6 *	1	0510	63	73.	133.3	10.2 *	1	0830	103	71.	123
1	0155	24	27.	7.1	1.8 *	1	0515	64	74.	135.3	10.4 *	1	0835	104	71.	123
1	0200	25	30.	7.9	2.0 *	1	0520	65	74.	136.6	10.4 *	1	0840	105	71.	122
1	0205	26	30.	8.8	2.1 *	1	0525	66	74.	137.4	10.5 *	1	0845	106	71.	122
1	0210	27	31.	9.8	2.1 *	1	0530	67	74.	137.8	10.5 *	1	0850	107	70.	121
1	0215	28	31.	10.8	2.2 *	1	0535	68	75.	138.1	10.5 *	1	0855	108	70.	121
1	0220	29	31.	11.7	2.2 *	1	0540	69	75.	138.2	10.5 *	1	0900	109	70.	121
1	0225	30	31.	12.4	2.3 *	1	0545	70	75.	138.2	10.5 *	1	0905	110	70.	120
1	0230	31	32.	12.9	2.3 *	1	0550	71	75.	138.1	10.5 *	1	0910	111	70.	120
1	0235	32	32.	13.3	2.3 *	1	0555	72	74.	137.9	10.5 *	1	0915	112	70.	119
1	0240	33	32.	13.7	2.4 *	1	0600	73	74.	137.8	10.5 *	1	0920	113	70.	119
1	0245	34	32.	14.1	2.4 *	1	0605	74	74.	137.6	10.5 *	1	0925	114	70.	118
1	0250	35	32.	14.8	2.4 *	1	0610	75	74.	137.3	10.5 *	1	0930	115	70.	118
1	0255	36	32.	15.7	2.5 *	1	0615	76	74.	137.0	10.5 *	1	0935	116	69.	117
1	0300	37	33.	17.1	2.6 *	1	0620	77	74.	136.6	10.4 *	1	0940	117	69.	117
1	0305	38	33.	19.0	2.7 *	1	0625	78	74.	136.2	10.4 *	1	0945	118	69.	116
1	0310	39	34.	21.7	2.9 *	1	0630	79	74.	135.8	10.4 *	1	0950	119	69.	116
1	0315	40	35.	25.2	3.1 *	1	0635	80	74.	135.3	10.4 *	1	0955	120	69.	115

PEAK FLOW	TIME		6-HR	24-HR	72-HR	9.92-HRD
(CFS)	(HR)	(CFS)□				
75.	5.67		71.	52.	52.	52.
		(INCHES)	.236	.285	.285	.285
		(AC-FT)	35.	43.	43.	43.

PEAK STORAGE	TIME		6-HR	24-HR	72-HR	9.92-HRD
(AC-FT)	(HR)					
138.	5.67		125.	81.	81.	81.

PEAK STAGE	TIME		6-HR	24-HR	72-HR	9.92-HRD
(FEET)	(HR)					
10.55	5.67		9.64	6.58	6.58	6.58

CUMULATIVE AREA = 2.80 SQ MI

HYDROGRAPH AT STATION MCDB  
PLAN 1, RATIO = .76

						*							*						
DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	DA	MON	HRMN	ORD	OUTFLOW	STORA
						*							*						
1		0000	1	0.	.0	.0	*	1	0320	41	36.	26.5	3.2	*	1	0640	81	71.	124
1		0005	2	0.	.0	.0	*	1	0325	42	37.	31.8	3.5	*	1	0645	82	71.	123
1		0010	3	0.	.0	.0	*	1	0330	43	40.	38.5	3.9	*	1	0650	83	71.	123
1		0015	4	0.	.0	.0	*	1	0335	44	43.	46.8	4.5	*	1	0655	84	71.	122
1		0020	5	0.	.0	.0	*	1	0340	45	48.	55.8	5.1	*	1	0700	85	71.	122
1		0025	6	0.	.0	.0	*	1	0345	46	52.	64.2	5.6	*	1	0705	86	70.	122
1		0030	7	0.	.1	.0	*	1	0350	47	55.	71.2	6.1	*	1	0710	87	70.	121

1	0035	8	1.	.3	.1 *	1	0355	48	57.	76.9	6.5 *	1	0715	88	70.	121
1	0040	9	2.	.6	.1 *	1	0400	49	59.	81.7	6.8 *	1	0720	89	70.	120
1	0045	10	3.	.9	.2 *	1	0405	50	60.	85.8	7.1 *	1	0725	90	70.	120
1	0050	11	4.	1.2	.3 *	1	0410	51	61.	89.0	7.3 *	1	0730	91	70.	119
1	0055	12	6.	1.5	.4 *	1	0415	52	62.	91.5	7.4 *	1	0735	92	70.	119
1	0100	13	7.	1.7	.4 *	1	0420	53	63.	93.5	7.6 *	1	0740	93	70.	118
1	0105	14	7.	2.0	.5 *	1	0425	54	63.	95.2	7.7 *	1	0745	94	70.	118
1	0110	15	8.	2.2	.6 *	1	0430	55	64.	96.8	7.8 *	1	0750	95	69.	117
1	0115	16	9.	2.4	.6 *	1	0435	56	65.	98.7	7.9 *	1	0755	96	69.	117
1	0120	17	10.	2.6	.7 *	1	0440	57	65.	101.2	8.1 *	1	0800	97	69.	116
1	0125	18	11.	2.9	.7 *	1	0445	58	66.	104.4	8.3 *	1	0805	98	69.	116
1	0130	19	12.	3.2	.8 *	1	0450	59	67.	108.3	8.6 *	1	0810	99	69.	115
1	0135	20	14.	3.6	.9 *	1	0455	60	68.	112.6	8.8 *	1	0815	100	69.	115
1	0140	21	16.	4.2	1.0 *	1	0500	61	69.	116.8	9.1 *	1	0820	101	69.	114
1	0145	22	18.	4.8	1.2 *	1	0505	62	70.	120.5	9.4 *	1	0825	102	69.	114
1	0150	23	20.	5.5	1.4 *	1	0510	63	71.	123.2	9.5 *	1	0830	103	68.	113
1	0155	24	23.	6.2	1.5 *	1	0515	64	71.	125.1	9.7 *	1	0835	104	68.	113
1	0200	25	26.	6.9	1.7 *	1	0520	65	72.	126.3	9.8 *	1	0840	105	68.	112
1	0205	26	29.	7.7	1.9 *	1	0525	66	72.	127.0	9.8 *	1	0845	106	68.	112
1	0210	27	30.	8.6	2.0 *	1	0530	67	72.	127.4	9.8 *	1	0850	107	68.	111
1	0215	28	30.	9.4	2.1 *	1	0535	68	72.	127.6	9.8 *	1	0855	108	68.	111
1	0220	29	31.	10.2	2.1 *	1	0540	69	72.	127.7	9.8 *	1	0900	109	68.	111
1	0225	30	31.	10.8	2.2 *	1	0545	70	72.	127.7	9.8 *	1	0905	110	68.	110
1	0230	31	31.	11.2	2.2 *	1	0550	71	72.	127.5	9.8 *	1	0910	111	68.	110
1	0235	32	31.	11.5	2.2 *	1	0555	72	72.	127.4	9.8 *	1	0915	112	67.	109
1	0240	33	31.	11.9	2.2 *	1	0600	73	72.	127.2	9.8 *	1	0920	113	67.	109
1	0245	34	31.	12.3	2.3 *	1	0605	74	72.	127.0	9.8 *	1	0925	114	67.	108
1	0250	35	32.	12.8	2.3 *	1	0610	75	72.	126.8	9.8 *	1	0930	115	67.	108
1	0255	36	32.	13.6	2.4 *	1	0615	76	72.	126.5	9.8 *	1	0935	116	67.	107
1	0300	37	32.	14.9	2.4 *	1	0620	77	72.	126.1	9.7 *	1	0940	117	67.	107
1	0305	38	33.	16.7	2.5 *	1	0625	78	71.	125.7	9.7 *	1	0945	118	67.	106
1	0310	39	33.	19.1	2.7 *	1	0630	79	71.	125.3	9.7 *	1	0950	119	67.	106
1	0315	40	34.	22.3	2.9 *	1	0635	80	71.	124.8	9.7 *	1	0955	120	66.	105

PEAK FLOW	TIME		6-HR	24-HR	72-HR	9.92-HR
(CFS)	(HR)	(CFS) □				
72.	5.67		69.	50.	50.	50.
		(INCHES)	.227	.274	.274	.274
		(AC-FT)	34.	41.	41.	41.

PEAK STORAGE	TIME		6-HR	24-HR	72-HR	9.92-HR
(AC-FT)	(HR)					
128.	5.67		115.	75.	75.	75.

PEAK STAGE	TIME		6-HR	24-HR	72-HR	9.92-HR
(FEET)	(HR)					
9.85	5.67		8.98	6.13	6.13	6.13

CUMULATIVE AREA = 2.80 SQ MI

HYDROGRAPH AT STATION MCDB  
PLAN 1, RATIO = .73

*****																					
						*							*								
DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
						*								*							
1		0000	1	0.	.0	.0	*	1		0320	41	35.	23.4	3.0	*	1		0640	81	68.	113
1		0005	2	0.	.0	.0	*	1		0325	42	36.	28.2	3.3	*	1		0645	82	68.	113
1		0010	3	0.	.0	.0	*	1		0330	43	38.	34.4	3.7	*	1		0650	83	68.	112
1		0015	4	0.	.0	.0	*	1		0335	44	41.	42.1	4.1	*	1		0655	84	68.	112
1		0020	5	0.	.0	.0	*	1		0340	45	45.	50.4	4.7	*	1		0700	85	68.	111
1		0025	6	0.	.0	.0	*	1		0345	46	49.	58.2	5.2	*	1		0705	86	68.	111
1		0030	7	0.	.1	.0	*	1		0350	47	52.	64.7	5.6	*	1		0710	87	68.	111
1		0035	8	1.	.2	.1	*	1		0355	48	55.	70.0	6.0	*	1		0715	88	68.	110
1		0040	9	2.	.4	.1	*	1		0400	49	56.	74.4	6.3	*	1		0720	89	68.	110
1		0045	10	3.	.7	.2	*	1		0405	50	58.	78.2	6.5	*	1		0725	90	67.	109
1		0050	11	3.	.9	.2	*	1		0410	51	59.	81.2	6.7	*	1		0730	91	67.	109
1		0055	12	4.	1.2	.3	*	1		0415	52	60.	83.5	6.9	*	1		0735	92	67.	108
1		0100	13	5.	1.4	.3	*	1		0420	53	60.	85.3	7.0	*	1		0740	93	67.	108

1	0105	14	6.	1.6	.4 *	1	0425	54	61.	86.9	7.1 *	1	0745	94	67.	107
1	0110	15	7.	1.8	.4 *	1	0430	55	61.	88.4	7.2 *	1	0750	95	67.	107
1	0115	16	7.	2.0	.5 *	1	0435	56	62.	90.2	7.3 *	1	0755	96	67.	106
1	0120	17	8.	2.2	.5 *	1	0440	57	62.	92.5	7.5 *	1	0800	97	67.	106
1	0125	18	9.	2.4	.6 *	1	0445	58	63.	95.5	7.7 *	1	0805	98	66.	105
1	0130	19	10.	2.7	.7 *	1	0450	59	65.	99.1	7.9 *	1	0810	99	66.	105
1	0135	20	11.	3.0	.8 *	1	0455	60	66.	103.1	8.2 *	1	0815	100	66.	105
1	0140	21	13.	3.5	.9 *	1	0500	61	67.	107.1	8.5 *	1	0820	101	66.	104
1	0145	22	15.	4.0	1.0 *	1	0505	62	68.	110.5	8.7 *	1	0825	102	66.	104
1	0150	23	17.	4.6	1.2 *	1	0510	63	68.	113.0	8.9 *	1	0830	103	66.	103
1	0155	24	20.	5.2	1.3 *	1	0515	64	69.	114.8	9.0 *	1	0835	104	66.	103
1	0200	25	22.	5.9	1.5 *	1	0520	65	69.	115.8	9.1 *	1	0840	105	66.	102
1	0205	26	25.	6.6	1.6 *	1	0525	66	69.	116.5	9.1 *	1	0845	106	66.	102
1	0210	27	28.	7.3	1.8 *	1	0530	67	69.	116.8	9.1 *	1	0850	107	65.	101
1	0215	28	30.	8.1	2.0 *	1	0535	68	69.	117.0	9.1 *	1	0855	108	65.	101
1	0220	29	30.	8.7	2.0 *	1	0540	69	69.	117.1	9.1 *	1	0900	109	65.	100
1	0225	30	30.	9.2	2.1 *	1	0545	70	69.	117.0	9.1 *	1	0905	110	65.	100
1	0230	31	30.	9.6	2.1 *	1	0550	71	69.	116.9	9.1 *	1	0910	111	65.	100
1	0235	32	31.	9.9	2.1 *	1	0555	72	69.	116.8	9.1 *	1	0915	112	65.	99
1	0240	33	31.	10.1	2.1 *	1	0600	73	69.	116.6	9.1 *	1	0920	113	65.	99
1	0245	34	31.	10.5	2.2 *	1	0605	74	69.	116.4	9.1 *	1	0925	114	65.	98
1	0250	35	31.	10.9	2.2 *	1	0610	75	69.	116.2	9.1 *	1	0930	115	64.	98
1	0255	36	31.	11.6	2.2 *	1	0615	76	69.	115.9	9.1 *	1	0935	116	64.	97
1	0300	37	31.	12.7	2.3 *	1	0620	77	69.	115.5	9.0 *	1	0940	117	64.	97
1	0305	38	32.	14.3	2.4 *	1	0625	78	69.	115.1	9.0 *	1	0945	118	64.	96
1	0310	39	33.	16.6	2.5 *	1	0630	79	69.	114.7	9.0 *	1	0950	119	64.	96
1	0315	40	34.	19.5	2.7 *	1	0635	80	69.	114.2	8.9 *	1	0955	120	64.	96

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PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	9.92-HR
		(CFS)				
69.	5.67		66.	48.	48.	48.
		(INCHES)	.219	.262	.262	.262
		(AC-FT)	33.	39.	39.	39.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	9.92-HR
117.	5.67		105.	68.	68.	68.

PEAK STAGE	TIME	MAXIMUM AVERAGE STAGE			
(FEET)	(HR)	6-HR	24-HR	72-HR	9.92-HR
9.14	5.67	8.32	5.67	5.67	5.67

CUMULATIVE AREA = 2.80 SQ MI

HYDROGRAPH AT STATION MCDB  
PLAN 1, RATIO = .69

DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	.0	*	1		0320	41	34.	21.1	2.8	*	1		0640	81	66.	106
1		0005	2	0.	.0	.0	*	1		0325	42	36.	25.6	3.1	*	1		0645	82	66.	105
1		0010	3	0.	.0	.0	*	1		0330	43	37.	31.4	3.5	*	1		0650	83	66.	105
1		0015	4	0.	.0	.0	*	1		0335	44	40.	38.6	3.9	*	1		0655	84	66.	104
1		0020	5	0.	.0	.0	*	1		0340	45	43.	46.4	4.4	*	1		0700	85	66.	104
1		0025	6	0.	.0	.0	*	1		0345	46	47.	53.8	4.9	*	1		0705	86	66.	103
1		0030	7	0.	.1	.0	*	1		0350	47	50.	59.9	5.3	*	1		0710	87	66.	103
1		0035	8	1.	.2	.0	*	1		0355	48	52.	64.9	5.7	*	1		0715	88	66.	102
1		0040	9	1.	.3	.1	*	1		0400	49	55.	69.1	5.9	*	1		0720	89	66.	102
1		0045	10	2.	.5	.1	*	1		0405	50	56.	72.6	6.2	*	1		0725	90	65.	101
1		0050	11	3.	.7	.2	*	1		0410	51	57.	75.5	6.4	*	1		0730	91	65.	101
1		0055	12	4.	.9	.2	*	1		0415	52	58.	77.6	6.5	*	1		0735	92	65.	101
1		0100	13	4.	1.1	.3	*	1		0420	53	58.	79.4	6.6	*	1		0740	93	65.	100
1		0105	14	5.	1.3	.3	*	1		0425	54	59.	80.8	6.7	*	1		0745	94	65.	100
1		0110	15	6.	1.5	.4	*	1		0430	55	59.	82.2	6.8	*	1		0750	95	65.	99
1		0115	16	6.	1.7	.4	*	1		0435	56	60.	83.9	6.9	*	1		0755	96	65.	99
1		0120	17	7.	1.8	.5	*	1		0440	57	60.	86.0	7.1	*	1		0800	97	65.	98
1		0125	18	8.	2.0	.5	*	1		0445	58	61.	88.9	7.3	*	1		0805	98	64.	98
1		0130	19	9.	2.3	.6	*	1		0450	59	62.	92.3	7.5	*	1		0810	99	64.	97



1	0135	20	10.	2.6	.7 *	1	0455	60	64.	96.1	7.7 *	1	0815	100	64.	97
1	0140	21	11.	3.0	.8 *	1	0500	61	65.	99.9	8.0 *	1	0820	101	64.	97
1	0145	22	13.	3.5	.9 *	1	0505	62	66.	103.1	8.2 *	1	0825	102	64.	96
1	0150	23	15.	4.0	1.0 *	1	0510	63	66.	105.5	8.4 *	1	0830	103	64.	96
1	0155	24	17.	4.6	1.1 *	1	0515	64	67.	107.1	8.5 *	1	0835	104	64.	95
1	0200	25	19.	5.1	1.3 *	1	0520	65	67.	108.1	8.5 *	1	0840	105	63.	95
1	0205	26	22.	5.8	1.4 *	1	0525	66	67.	108.7	8.6 *	1	0845	106	63.	94
1	0210	27	24.	6.5	1.6 *	1	0530	67	67.	109.0	8.6 *	1	0850	107	63.	94
1	0215	28	27.	7.2	1.8 *	1	0535	68	67.	109.2	8.6 *	1	0855	108	63.	93
1	0220	29	29.	7.7	1.9 *	1	0540	69	67.	109.2	8.6 *	1	0900	109	63.	93
1	0225	30	30.	8.2	2.0 *	1	0545	70	67.	109.1	8.6 *	1	0905	110	63.	93
1	0230	31	30.	8.5	2.0 *	1	0550	71	67.	109.0	8.6 *	1	0910	111	63.	92
1	0235	32	30.	8.7	2.0 *	1	0555	72	67.	108.9	8.6 *	1	0915	112	62.	92
1	0240	33	30.	8.9	2.1 *	1	0600	73	67.	108.7	8.6 *	1	0920	113	62.	91
1	0245	34	30.	9.2	2.1 *	1	0605	74	67.	108.5	8.6 *	1	0925	114	62.	91
1	0250	35	30.	9.6	2.1 *	1	0610	75	67.	108.3	8.6 *	1	0930	115	62.	90
1	0255	36	31.	10.2	2.1 *	1	0615	76	67.	108.0	8.5 *	1	0935	116	62.	90
1	0300	37	31.	11.2	2.2 *	1	0620	77	67.	107.6	8.5 *	1	0940	117	62.	90
1	0305	38	31.	12.7	2.3 *	1	0625	78	67.	107.2	8.5 *	1	0945	118	62.	89
1	0310	39	32.	14.7	2.4 *	1	0630	79	67.	106.8	8.5 *	1	0950	119	61.	89
1	0315	40	33.	17.5	2.6 *	1	0635	80	67.	106.4	8.4 *	1	0955	120	61.	88

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PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	9.92-HR
67.	5.67	(CFS)□	64.	46.	46.	46.
		(INCHES)	.212	.253	.253	.253
		(AC-FT)	32.	38.	38.	38.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	9.92-HR
109.	5.67		97.	63.	63.	63.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	9.92-HR
8.61	5.67		7.83	5.33	5.33	5.33

CUMULATIVE AREA = 2.80 SQ MI

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HYDROGRAPH AT STATION MCDB  
PLAN 1, RATIO = .64

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	.0	*	1		0320	41	33.	17.2	2.6	*	1		0640	81	62.	92
1		0005	2	0.	.0	.0	*	1		0325	42	34.	21.2	2.8	*	1		0645	82	62.	91
1		0010	3	0.	.0	.0	*	1		0330	43	36.	26.3	3.1	*	1		0650	83	62.	91
1		0015	4	0.	.0	.0	*	1		0335	44	38.	32.6	3.5	*	1		0655	84	62.	90
1		0020	5	0.	.0	.0	*	1		0340	45	40.	39.6	4.0	*	1		0700	85	62.	90
1		0025	6	0.	.0	.0	*	1		0345	46	43.	46.1	4.4	*	1		0705	86	62.	90
1		0030	7	0.	.0	.0	*	1		0350	47	46.	51.5	4.8	*	1		0710	87	62.	89
1		0035	8	0.	.1	.0	*	1		0355	48	48.	55.9	5.1	*	1		0715	88	61.	89
1		0040	9	1.	.2	.0	*	1		0400	49	50.	59.7	5.3	*	1		0720	89	61.	88
1		0045	10	1.	.3	.1	*	1		0405	50	51.	62.8	5.5	*	1		0725	90	61.	88
1		0050	11	2.	.5	.1	*	1		0410	51	53.	65.4	5.7	*	1		0730	91	61.	87
1		0055	12	2.	.6	.2	*	1		0415	52	54.	67.3	5.8	*	1		0735	92	61.	87
1		0100	13	3.	.8	.2	*	1		0420	53	54.	68.8	5.9	*	1		0740	93	61.	87
1		0105	14	3.	.9	.2	*	1		0425	54	55.	70.0	6.0	*	1		0745	94	61.	86
1		0110	15	4.	1.0	.3	*	1		0430	55	55.	71.3	6.1	*	1		0750	95	60.	86
1		0115	16	4.	1.2	.3	*	1		0435	56	56.	72.7	6.2	*	1		0755	96	60.	85
1		0120	17	5.	1.3	.3	*	1		0440	57	57.	74.7	6.3	*	1		0800	97	60.	85
1		0125	18	5.	1.5	.4	*	1		0445	58	57.	77.2	6.5	*	1		0805	98	60.	84
1		0130	19	6.	1.7	.4	*	1		0450	59	58.	80.2	6.7	*	1		0810	99	60.	84
1		0135	20	7.	1.9	.5	*	1		0455	60	60.	83.7	6.9	*	1		0815	100	60.	84
1		0140	21	8.	2.2	.6	*	1		0500	61	61.	87.0	7.1	*	1		0820	101	60.	83
1		0145	22	10.	2.6	.7	*	1		0505	62	62.	89.9	7.3	*	1		0825	102	59.	83
1		0150	23	11.	3.0	.8	*	1		0510	63	62.	92.1	7.5	*	1		0830	103	59.	82
1		0155	24	13.	3.4	.9	*	1		0515	64	63.	93.5	7.6	*	1		0835	104	59.	82
1		0200	25	15.	3.9	1.0	*	1		0520	65	63.	94.4	7.6	*	1		0840	105	59.	82

1	0205	26	17.	4.4	1.1 *	1	0525	66	63.	94.9	7.7 *	1	0845	106	59.	81
1	0210	27	19.	5.0	1.2 *	1	0530	67	63.	95.1	7.7 *	1	0850	107	59.	81
1	0215	28	21.	5.6	1.4 *	1	0535	68	63.	95.2	7.7 *	1	0855	108	59.	80
1	0220	29	23.	6.0	1.5 *	1	0540	69	63.	95.2	7.7 *	1	0900	109	58.	80
1	0225	30	24.	6.4	1.6 *	1	0545	70	63.	95.1	7.7 *	1	0905	110	58.	80
1	0230	31	25.	6.6	1.7 *	1	0550	71	63.	95.0	7.7 *	1	0910	111	58.	79
1	0235	32	26.	6.8	1.7 *	1	0555	72	63.	94.9	7.7 *	1	0915	112	58.	79
1	0240	33	26.	7.0	1.7 *	1	0600	73	63.	94.7	7.6 *	1	0920	113	58.	78
1	0245	34	27.	7.2	1.8 *	1	0605	74	63.	94.5	7.6 *	1	0925	114	58.	78
1	0250	35	28.	7.5	1.9 *	1	0610	75	63.	94.3	7.6 *	1	0930	115	58.	78
1	0255	36	30.	8.0	2.0 *	1	0615	76	63.	94.0	7.6 *	1	0935	116	58.	77
1	0300	37	30.	8.8	2.1 *	1	0620	77	63.	93.7	7.6 *	1	0940	117	57.	77
1	0305	38	31.	10.0	2.1 *	1	0625	78	63.	93.3	7.6 *	1	0945	118	57.	76
1	0310	39	31.	11.8	2.2 *	1	0630	79	63.	92.9	7.5 *	1	0950	119	57.	76
1	0315	40	32.	14.1	2.4 *	1	0635	80	62.	92.5	7.5 *	1	0955	120	57.	76

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PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	9.92-HR
63.	5.58	(CFS)□	60.	43.	43.	43.
		(INCHES)	.198	.234	.234	.234
		(AC-FT)	30.	35.	35.	35.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	9.92-HR
95.	5.58		84.	54.	54.	54.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	9.92-HR
7.68	5.58		6.96	4.72	4.72	4.72

CUMULATIVE AREA = 2.80 SQ MI

HYDROGRAPH AT STATION MCDB  
PLAN 1, RATIO = .58

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE	*	DA	MON	HRMN	ORD	OUTFLOW	STORA
1		0000	1	0.	.0	.0	*	1		0320	41	32.	13.5	2.3	*	1		0640	81	58.	77
1		0005	2	0.	.0	.0	*	1		0325	42	33.	16.9	2.6	*	1		0645	82	57.	77
1		0010	3	0.	.0	.0	*	1		0330	43	34.	21.2	2.8	*	1		0650	83	57.	76
1		0015	4	0.	.0	.0	*	1		0335	44	36.	26.6	3.2	*	1		0655	84	57.	76
1		0020	5	0.	.0	.0	*	1		0340	45	38.	32.6	3.5	*	1		0700	85	57.	76
1		0025	6	0.	.0	.0	*	1		0345	46	39.	38.2	3.9	*	1		0705	86	57.	75
1		0030	7	0.	.0	.0	*	1		0350	47	41.	42.9	4.2	*	1		0710	87	57.	75
1		0035	8	0.	.0	.0	*	1		0355	48	43.	46.7	4.4	*	1		0715	88	57.	74
1		0040	9	0.	.1	.0	*	1		0400	49	45.	50.0	4.7	*	1		0720	89	56.	74
1		0045	10	1.	.1	.0	*	1		0405	50	46.	52.7	4.8	*	1		0725	90	56.	74
1		0050	11	1.	.2	.1	*	1		0410	51	47.	54.9	5.0	*	1		0730	91	56.	73
1		0055	12	1.	.3	.1	*	1		0415	52	48.	56.5	5.1	*	1		0735	92	56.	73
1		0100	13	2.	.4	.1	*	1		0420	53	49.	57.8	5.2	*	1		0740	93	56.	72
1		0105	14	2.	.5	.1	*	1		0425	54	49.	58.9	5.3	*	1		0745	94	56.	72
1		0110	15	2.	.6	.2	*	1		0430	55	50.	59.9	5.3	*	1		0750	95	56.	72
1		0115	16	3.	.7	.2	*	1		0435	56	51.	61.2	5.4	*	1		0755	96	56.	71
1		0120	17	3.	.8	.2	*	1		0440	57	51.	62.8	5.5	*	1		0800	97	55.	71
1		0125	18	3.	.9	.2	*	1		0445	58	53.	65.0	5.7	*	1		0805	98	55.	71
1		0130	19	4.	1.1	.3	*	1		0450	59	54.	67.7	5.8	*	1		0810	99	55.	70
1		0135	20	5.	1.3	.3	*	1		0455	60	55.	70.7	6.0	*	1		0815	100	55.	70
1		0140	21	6.	1.5	.4	*	1		0500	61	56.	73.6	6.2	*	1		0820	101	55.	69
1		0145	22	7.	1.8	.5	*	1		0505	62	57.	76.2	6.4	*	1		0825	102	55.	69
1		0150	23	8.	2.1	.5	*	1		0510	63	58.	78.0	6.5	*	1		0830	103	55.	69
1		0155	24	9.	2.4	.6	*	1		0515	64	58.	79.2	6.6	*	1		0835	104	54.	68
1		0200	25	10.	2.8	.7	*	1		0520	65	58.	80.0	6.7	*	1		0840	105	54.	68
1		0205	26	12.	3.2	.8	*	1		0525	66	58.	80.4	6.7	*	1		0845	106	54.	68
1		0210	27	13.	3.6	.9	*	1		0530	67	59.	80.6	6.7	*	1		0850	107	54.	67
1		0215	28	15.	4.0	1.0	*	1		0535	68	59.	80.6	6.7	*	1		0855	108	54.	67
1		0220	29	16.	4.4	1.1	*	1		0540	69	59.	80.6	6.7	*	1		0900	109	53.	66
1		0225	30	17.	4.7	1.2	*	1		0545	70	58.	80.5	6.7	*	1		0905	110	53.	66
1		0230	31	18.	4.9	1.2	*	1		0550	71	58.	80.4	6.7	*	1		0910	111	53.	66

1	0235	32	19.	5.0	1.3 *	1	0555	72	58.	80.2	6.7 *	1	0915	112	53.	65
1	0240	33	19.	5.2	1.3 *	1	0600	73	58.	80.0	6.7 *	1	0920	113	53.	65
1	0245	34	20.	5.4	1.3 *	1	0605	74	58.	79.9	6.7 *	1	0925	114	53.	65
1	0250	35	21.	5.6	1.4 *	1	0610	75	58.	79.6	6.6 *	1	0930	115	52.	64
1	0255	36	23.	6.0	1.5 *	1	0615	76	58.	79.4	6.6 *	1	0935	116	52.	64
1	0300	37	25.	6.7	1.7 *	1	0620	77	58.	79.1	6.6 *	1	0940	117	52.	64
1	0305	38	29.	7.7	1.9 *	1	0625	78	58.	78.7	6.6 *	1	0945	118	52.	63
1	0310	39	30.	9.0	2.1 *	1	0630	79	58.	78.4	6.6 *	1	0950	119	52.	63
1	0315	40	31.	11.0	2.2 *	1	0635	80	58.	78.0	6.5 *	1	0955	120	51.	62

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PEAK FLOW (CFS)	TIME (HR)	6-HR (CFS)□	24-HR (INCHES) (AC-FT)	72-HR (INCHES) (AC-FT)	9.92-HR□ (INCHES) (AC-FT)
59.	5.58	55.	.182	.212	.212
		27.	32.	32.	32.

PEAK STORAGE (AC-FT)	TIME (HR)	6-HR	24-HR	72-HR	9.92-HR□
81.	5.58	71.	46.	46.	46.

PEAK STAGE (FEET)	TIME (HR)	6-HR	24-HR	72-HR	9.92-HR□
6.71	5.58	6.05	4.08	4.08	4.08

CUMULATIVE AREA = 2.80 SQ MI

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HYDROGRAPH AT STATION MCDB  
PLAN 1, RATIO = .53

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DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE *	DA	MON	HRMN	ORD	OUTFLOW	STORAGE	STAGE *	DA	MON	HRMN	ORD	OUTFLOW	STORA
1	0000	1	0.	.0	.0 *	1	0320	41	31.	10.8	2.2 *	1	0640	81	53.	66			
1	0005	2	0.	.0	.0 *	1	0325	42	32.	13.7	2.4 *	1	0645	82	53.	65			
1	0010	3	0.	.0	.0 *	1	0330	43	33.	17.4	2.6 *	1	0650	83	53.	65			
1	0015	4	0.	.0	.0 *	1	0335	44	34.	22.0	2.9 *	1	0655	84	52.	64			
1	0020	5	0.	.0	.0 *	1	0340	45	36.	27.2	3.2 *	1	0700	85	52.	64			
1	0025	6	0.	.0	.0 *	1	0345	46	38.	32.0	3.5 *	1	0705	86	52.	64			
1	0030	7	0.	.0	.0 *	1	0350	47	39.	36.1	3.8 *	1	0710	87	52.	63			
1	0035	8	0.	.0	.0 *	1	0355	48	40.	39.4	4.0 *	1	0715	88	52.	63			
1	0040	9	0.	.0	.0 *	1	0400	49	41.	42.3	4.2 *	1	0720	89	52.	63			
1	0045	10	0.	.1	.0 *	1	0405	50	42.	44.7	4.3 *	1	0725	90	51.	62			
1	0050	11	0.	.1	.0 *	1	0410	51	43.	46.5	4.4 *	1	0730	91	51.	62			
1	0055	12	1.	.1	.0 *	1	0415	52	44.	48.0	4.5 *	1	0735	92	51.	62			
1	0100	13	1.	.2	.1 *	1	0420	53	45.	49.0	4.6 *	1	0740	93	51.	61			
1	0105	14	1.	.3	.1 *	1	0425	54	45.	50.0	4.7 *	1	0745	94	51.	61			
1	0110	15	1.	.3	.1 *	1	0430	55	45.	50.9	4.7 *	1	0750	95	51.	61			
1	0115	16	1.	.4	.1 *	1	0435	56	46.	51.9	4.8 *	1	0755	96	50.	60			
1	0120	17	2.	.5	.1 *	1	0440	57	47.	53.4	4.9 *	1	0800	97	50.	60			
1	0125	18	2.	.6	.1 *	1	0445	58	48.	55.3	5.0 *	1	0805	98	50.	60			
1	0130	19	3.	.7	.2 *	1	0450	59	49.	57.6	5.2 *	1	0810	99	50.	59			
1	0135	20	3.	.8	.2 *	1	0455	60	50.	60.3	5.4 *	1	0815	100	50.	59			
1	0140	21	4.	1.0	.3 *	1	0500	61	51.	62.9	5.5 *	1	0820	101	49.	59			
1	0145	22	5.	1.2	.3 *	1	0505	62	53.	65.1	5.7 *	1	0825	102	49.	58			
1	0150	23	6.	1.5	.4 *	1	0510	63	53.	66.7	5.8 *	1	0830	103	49.	58			
1	0155	24	7.	1.7	.4 *	1	0515	64	54.	67.8	5.9 *	1	0835	104	49.	57			
1	0200	25	8.	2.0	.5 *	1	0520	65	54.	68.4	5.9 *	1	0840	105	49.	57			
1	0205	26	9.	2.3	.6 *	1	0525	66	54.	68.7	5.9 *	1	0845	106	49.	57			
1	0210	27	10.	2.6	.7 *	1	0530	67	54.	68.8	5.9 *	1	0850	107	48.	56			
1	0215	28	11.	3.0	.7 *	1	0535	68	54.	68.9	5.9 *	1	0855	108	48.	56			
1	0220	29	12.	3.2	.8 *	1	0540	69	54.	68.8	5.9 *	1	0900	109	48.	56			
1	0225	30	13.	3.5	.9 *	1	0545	70	54.	68.7	5.9 *	1	0905	110	48.	55			
1	0230	31	14.	3.7	.9 *	1	0550	71	54.	68.6	5.9 *	1	0910	111	48.	55			
1	0235	32	14.	3.8	1.0 *	1	0555	72	54.	68.4	5.9 *	1	0915	112	48.	55			
1	0240	33	15.	3.9	1.0 *	1	0600	73	54.	68.3	5.9 *	1	0920	113	47.	54			
1	0245	34	15.	4.1	1.0 *	1	0605	74	54.	68.1	5.9 *	1	0925	114	47.	54			
1	0250	35	16.	4.3	1.1 *	1	0610	75	54.	67.9	5.9 *	1	0930	115	47.	54			
1	0255	36	17.	4.6	1.2 *	1	0615	76	54.	67.6	5.8 *	1	0935	116	47.	54			
1	0300	37	19.	5.2	1.3 *	1	0620	77	54.	67.3	5.8 *	1	0940	117	47.	53			

1	0305	38	22.	6.0	1.5	*	1	0625	78	54.	67.0	5.8	*	1	0945	118	47.	53
1	0310	39	27.	7.1	1.8	*	1	0630	79	53.	66.7	5.8	*	1	0950	119	47.	53
1	0315	40	30.	8.7	2.0	*	1	0635	80	53.	66.3	5.8	*	1	0955	120	46.	52

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PEAK FLOW	TIME		MAXIMUM AVERAGE FLOW			
(CFS)	(HR)		6-HR	24-HR	72-HR	9.92-HR□
54.	5.58	(CFS)□	50.	35.	35.	35.
		(INCHES)	.166	.191	.191	.191
		(AC-FT)	25.	29.	29.	29.

PEAK STORAGE	TIME		MAXIMUM AVERAGE STORAGE			
(AC-FT)	(HR)		6-HR	24-HR	72-HR	9.92-HR□
69.	5.58		60.	38.	38.	38.

PEAK STAGE	TIME		MAXIMUM AVERAGE STAGE			
(FEET)	(HR)		6-HR	24-HR	72-HR	9.92-HR□
5.92	5.58		5.34	3.58	3.58	3.58

CUMULATIVE AREA = 2.80 SQ MI

□ □

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS  
 FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES  
 TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN		RATIOS APPLIED TO PRECIPITATION							
					RATIO 1 .83	RATIO 2 .80	RATIO 3 .76	RATIO 4 .73	RATIO 5 .69	RATIO 6 .64	RATIO 7 .58	RATIO 8 .54
HYDROGRAPH AT□	B13A	1.29	1	FLOW TIME	580. 4.08	551. 4.08	512. 4.08	471. 4.08	441. 4.08	387. 4.08	329. 4.08	28 4.
ROUTED TO□	RB13A	1.29	1	FLOW TIME	533. 4.75	507. 4.75	470. 4.75	433. 4.75	406. 4.75	356. 4.75	302. 4.75	25 4.
HYDROGRAPH AT□	B13B	.54	1	FLOW TIME	148. 5.08	140. 5.08	130. 5.08	120. 5.17	112. 5.17	98. 5.25	84. 5.33	7 5.
2 COMBINED AT□	PT1	1.83	1	FLOW TIME	675. 4.75	641. 4.75	595. 4.75	547. 4.75	512. 4.75	448. 4.75	380. 4.75	32 4.
ROUTED TO□	RPT1	1.83	1	FLOW TIME	658. 5.08	626. 5.08	581. 5.08	535. 5.08	501. 5.08	439. 5.08	372. 5.08	31 5.
HYDROGRAPH AT□	B17	.82	1	FLOW TIME	258. 4.33	244. 4.33	226. 4.33	207. 4.33	193. 4.42	168. 4.42	142. 4.42	12 4.
2 COMBINED AT□	PT2	2.64	1	FLOW TIME	864. 5.00	821. 5.00	762. 5.00	702. 5.00	657. 5.08	577. 5.08	490. 5.08	42 5.
DIVERSION TO□	DVBD2	2.64	1	FLOW TIME	350. 5.00	337. 5.00	319. 5.00	301. 5.00	288. 5.08	263. 5.08	237. 5.08	21 5.
HYDROGRAPH AT□	DVBD2	2.64	1	FLOW TIME	514. 5.00	484. 5.00	443. 5.00	401. 5.00	370. 5.08	314. 5.08	253. 5.08	20 5.
ROUTED TO□	RPT2	2.64	1	FLOW TIME	513. 5.08	483. 5.08	442. 5.08	400. 5.08	370. 5.08	313. 5.17	253. 5.17	20 5.
HYDROGRAPH AT□	B13C	2.01	1	FLOW TIME	893. 3.92	848. 3.92	785. 3.92	721. 3.92	674. 3.92	588. 3.92	496. 3.92	42 3.
2 COMBINED AT□	PT3	4.65	1	FLOW TIME	1052. 5.08	998. 5.08	924. 5.08	848. 5.08	792. 5.08	688. 5.08	577. 5.08	48 5.
DIVERSION TO□	DVBD3	4.65	1	FLOW TIME	273. 5.08	271. 5.08	268. 5.08	265. 5.08	263. 5.08	259. 5.08	255. 5.08	25 5.
HYDROGRAPH AT□	DVBD3	4.65	1	FLOW TIME	779. 5.08	727. 5.08	655. 5.08	583. 5.08	529. 5.08	429. 5.08	322. 5.08	23 5.
ROUTED TO□	RPT3A	4.65	1	FLOW TIME	773. 5.17	722. 5.17	653. 5.17	581. 5.17	526. 5.17	429. 5.17	322. 5.17	23 5.
DIVERSION TO□	DVBD4	4.65	1	FLOW TIME	178. 5.17	173. 5.17	165. 5.17	158. 5.17	152. 5.17	140. 5.17	126. 5.17	11 5.
HYDROGRAPH AT□	DVBD4	4.65	1	FLOW TIME	595. 5.17	549. 5.17	488. 5.17	424. 5.17	375. 5.17	289. 5.17	196. 5.17	12 5.

ROUTED TO	RPT3B	4.65	1	FLOW TIME	583. 5.25	537. 5.25	476. 5.25	412. 5.25	364. 5.25	279. 5.25	187. 5.25	11 5.
HYDROGRAPH AT	B11S	1.12	1	FLOW TIME	259. 4.00	240. 4.00	214. 4.08	189. 4.08	170. 4.08	138. 4.08	104. 4.08	8 5.
HYDROGRAPH AT	HYDRO1	.63	1	FLOW TIME	623. 4.08	623. 4.08	623. 4.08	623. 4.08	623. 4.08	623. 4.08	623. 4.08	62 4.
3 COMBINED AT	PT4	6.40	1	FLOW TIME	1428. 4.08	1359. 4.08	1265. 4.08	1171. 4.08	1099. 4.08	977. 4.08	847. 4.08	75 4.
ROUTED TO	RPT4A	6.40	1	FLOW TIME	1377. 4.33	1310. 4.33	1218. 4.33	1126. 4.33	1057. 4.33	939. 4.33	813. 4.33	72 4.
DIVERSION TO	DVBD7	6.40	1	FLOW TIME	918. 4.33	873. 4.33	812. 4.33	751. 4.33	705. 4.33	626. 4.33	542. 4.33	48 4.
HYDROGRAPH AT	DVBD7	6.40	1	FLOW TIME	459. 4.33	437. 4.33	406. 4.33	375. 4.33	352. 4.33	313. 4.33	271. 4.33	24 4.
ROUTED TO	RPT4B	6.40	1	FLOW TIME	446. 4.42	424. 4.42	394. 4.42	364. 4.50	341. 4.50	303. 4.50	262. 4.42	23 4.
HYDROGRAPH AT	B14B	.99	1	FLOW TIME	226. 3.83	209. 3.83	186. 3.83	163. 3.83	146. 3.83	118. 3.83	89. 3.92	6 3.
2 COMBINED AT	PT22W	7.39	1	FLOW TIME	563. 4.42	533. 4.42	492. 4.42	451. 4.42	421. 4.42	369. 4.42	314. 4.42	27 4.
HYDROGRAPH AT	D012A	10.21	1	FLOW TIME	2260. 5.08	2129. 5.08	1950. 5.17	1770. 5.25	1639. 5.25	1403. 5.25	1157. 5.33	96 5.
ROUTED TO	RD012A	10.21	1	FLOW TIME	2260. 5.08	2129. 5.08	1950. 5.17	1770. 5.25	1639. 5.25	1403. 5.33	1156. 5.33	96 5.
DIVERSION TO	DVI151	10.21	1	FLOW TIME	1276. 5.08	1255. 5.08	1227. 5.17	1198. 5.25	1171. 5.25	1121. 5.33	1049. 5.33	96 5.
HYDROGRAPH AT	DVI151	10.21	1	FLOW TIME	984. 5.08	874. 5.08	723. 5.17	572. 5.25	468. 5.25	282. 5.33	108. 5.33	5.
ROUTED TO	RI151	10.21	1	FLOW TIME	983. 5.17	873. 5.17	722. 5.25	571. 5.25	467. 5.33	281. 5.33	107. 5.42	5.
DIVERSION TO	DVI152	10.21	1	FLOW TIME	380. 5.17	341. 5.17	285. 5.25	229. 5.25	190. 5.33	112. 5.33	39. 5.42	5.
HYDROGRAPH AT	DVI152	10.21	1	FLOW TIME	604. 5.17	532. 5.17	437. 5.25	342. 5.25	277. 5.33	169. 5.33	68. 5.42	5.
ROUTED TO	RI152	10.21	1	FLOW TIME	601. 5.33	530. 5.33	435. 5.33	340. 5.42	275. 5.42	168. 5.50	66. 5.58	5.
HYDROGRAPH AT	D01	9.54	1	FLOW TIME	3126. 4.42	2950. 4.42	2709. 4.50	2466. 4.50	2286. 4.50	1962. 4.50	1621. 4.50	134 4.
ROUTED TO	RD01	9.54	1	FLOW TIME	2893. 5.17	2732. 5.17	2510. 5.17	2286. 5.17	2121. 5.25	1825. 5.25	1512. 5.25	126 5.

HYDROGRAPH AT□	D005	2.73	1	FLOW TIME	934. 4.17	879. 4.17	803. 4.17	727. 4.17	670. 4.17	569. 4.25	465. 4.25	38 4.
2 COMBINED AT□	PT9	12.27	1	FLOW TIME	3580. 5.17	3384. 5.17	3114. 5.25	2843. 5.25	2642. 5.25	2280. 5.25	1895. 5.25	158 5.
ROUTED TO□	RD005	12.27	1	FLOW TIME	3529. 5.42	3337. 5.42	3072. 5.42	2803. 5.42	2604. 5.42	2245. 5.42	1868. 5.50	156 5.
HYDROGRAPH AT□	D02	5.91	1	FLOW TIME	1927. 4.50	1819. 4.50	1671. 4.50	1521. 4.50	1409. 4.50	1210. 4.50	999. 4.58	83 4.
ROUTED TO□	RD02	5.91	1	FLOW TIME	1798. 5.17	1698. 5.17	1561. 5.25	1423. 5.25	1321. 5.25	1137. 5.25	943. 5.33	78 5.
HYDROGRAPH AT□	D03	11.99	1	FLOW TIME	4032. 4.42	3806. 4.42	3495. 4.42	3180. 4.42	2947. 4.42	2528. 4.42	2086. 4.42	173 4.
HYDROGRAPH AT□	D04	6.95	1	FLOW TIME	2254. 4.50	2131. 4.50	1960. 4.50	1787. 4.50	1660. 4.58	1432. 4.58	1190. 4.58	99 4.
2 COMBINED AT□	PT10	18.94	1	FLOW TIME	6270. 4.42	5920. 4.42	5438. 4.42	4950. 4.42	4589. 4.42	3945. 4.50	3266. 4.50	272 4.
ROUTED TO□	RPT10	18.94	1	FLOW TIME	5597. 5.42	5291. 5.42	4870. 5.42	4443. 5.50	4130. 5.50	3565. 5.50	2974. 5.67	250 5.
HYDROGRAPH AT□	D006	8.52	1	FLOW TIME	2804. 4.25	2643. 4.25	2421. 4.33	2199. 4.33	2034. 4.33	1739. 4.33	1428. 4.33	117 4.
3 COMBINED AT□	PT11	33.37	1	FLOW TIME	9528. 5.33	9015. 5.33	8307. 5.33	7588. 5.33	7056. 5.33	6096. 5.33	5082. 5.42	427 5.
ROUTED TO□	RPT11	33.37	1	FLOW TIME	9430. 5.50	8921. 5.50	8218. 5.50	7506. 5.50	6978. 5.50	6030. 5.58	5031. 5.58	422 5.
HYDROGRAPH AT□	D2A	.99	1	FLOW TIME	573. 3.67	541. 3.67	497. 3.67	453. 3.67	420. 3.67	361. 3.67	299. 3.67	24 3.
3 COMBINED AT□	PT12	46.63	1	FLOW TIME	13034. 5.42	12325. 5.42	11346. 5.42	10363. 5.50	9635. 5.50	8323. 5.50	6931. 5.50	581 5.
ROUTED TO□	RPT12	46.63	1	FLOW TIME	12288. 6.25	11624. 6.33	10708. 6.33	9780. 6.33	9092. 6.33	7853. 6.33	6548. 6.42	549 6.
HYDROGRAPH AT□	D2B	7.50	1	FLOW TIME	1583. 4.75	1482. 4.83	1344. 4.83	1207. 4.92	1109. 5.08	936. 5.17	760. 5.33	62 5.
2 COMBINED AT□	PT13	54.13	1	FLOW TIME	13326. 6.17	12595. 6.17	11587. 6.17	10572. 6.25	9822. 6.25	8471. 6.25	7040. 6.33	590 6.
ROUTED TO□	RPT13	54.13	1	FLOW TIME	12688. 7.08	11992. 7.08	11031. 7.17	10066. 7.17	9352. 7.17	8065. 7.17	6705. 7.25	561 7.
HYDROGRAPH AT□	D06	11.68	1	FLOW TIME	3284. 4.75	3103. 4.75	2852. 4.75	2598. 4.75	2410. 4.75	2072. 4.75	1718. 4.83	143 5.
ROUTED TO□	RD06	11.68	1	FLOW	3098.	2929.	2699.	2465.	2292.	1981.	1654.	139

				TIME	5.67	5.67	5.75	5.75	5.75	5.83	5.83	5.
HYDROGRAPH AT□	D3	6.64	1	FLOW TIME	1056. 4.83	981. 5.00	883. 5.33	790. 5.42	722. 5.42	601. 5.42	477. 5.50	38 5.
4 COMBINED AT□	PT14	82.66	1	FLOW TIME	15109. 6.75	14252. 6.83	13078. 6.83	11900. 6.83	11044. 6.92	9510. 6.92	7891. 7.00	660 7.
DIVERSION TO□	DVI153	82.66	1	FLOW TIME	9190. 6.75	8562. 6.83	7702. 6.83	6840. 6.83	6212. 6.92	5088. 6.92	4041. 7.00	326 7.
HYDROGRAPH AT□	DVI153	82.66	1	FLOW TIME	5919. 6.75	5690. 6.83	5376. 6.83	5060. 6.83	4831. 6.92	4422. 6.92	3850. 7.00	333 7.
ROUTED TO□	RPT14	82.66	1	FLOW TIME	5894. 7.25	5666. 7.25	5355. 7.33	5043. 7.33	4812. 7.33	4405. 7.42	3829. 7.42	331 7.
HYDROGRAPH AT□	D1A	2.29	1	FLOW TIME	770. 4.25	725. 4.25	665. 4.25	603. 4.25	558. 4.25	476. 4.25	391. 4.33	32 4.
ROUTED TO□	RD1A	2.29	1	FLOW TIME	758. 4.42	714. 4.42	655. 4.42	594. 4.42	549. 4.42	469. 4.50	386. 4.50	31 4.
HYDROGRAPH AT□	D1B	.32	1	FLOW TIME	89. 4.50	84. 4.50	77. 4.58	70. 4.58	65. 4.58	56. 4.58	46. 4.58	3 4.
2 COMBINED AT□	PT15	2.61	1	FLOW TIME	846. 4.42	798. 4.42	731. 4.42	663. 4.42	613. 4.50	525. 4.50	431. 4.50	35 4.
ROUTED TO□	RPT15	2.61	1	FLOW TIME	825. 4.75	777. 4.75	713. 4.83	647. 4.83	599. 4.83	513. 4.83	421. 4.83	34 4.
HYDROGRAPH AT□	D1C	.56	1	FLOW TIME	128. 4.92	120. 5.00	110. 5.08	100. 5.08	92. 5.17	79. 5.25	65. 5.25	5 5.
2 COMBINED AT□	PT16	3.17	1	FLOW TIME	951. 4.83	897. 4.83	822. 4.83	746. 4.83	690. 4.83	590. 4.83	484. 4.83	39 4.
ROUTED TO□	RPT16	3.17	1	FLOW TIME	936. 5.08	883. 5.08	809. 5.08	734. 5.08	679. 5.08	580. 5.08	476. 5.08	39 5.
HYDROGRAPH AT□	D1D	1.23	1	FLOW TIME	443. 3.92	416. 3.92	379. 3.92	341. 3.92	314. 3.92	265. 3.92	213. 3.92	17 3.
ROUTED TO□	RD1D	1.23	1	FLOW TIME	431. 4.08	405. 4.08	369. 4.08	332. 4.08	305. 4.08	257. 4.08	207. 4.08	16 4.
HYDROGRAPH AT□	D1E	.22	1	FLOW TIME	47. 4.58	44. 4.58	40. 4.58	35. 4.58	32. 4.58	27. 4.67	22. 5.33	1 5.
HYDROGRAPH AT□	D1F	.81	1	FLOW TIME	104. 5.58	97. 5.58	86. 5.67	76. 5.67	69. 5.67	56. 5.67	43. 5.75	3 5.
4 COMBINED AT□	PT17	5.43	1	FLOW TIME	1351. 5.17	1274. 5.17	1167. 5.17	1060. 5.17	981. 5.17	838. 5.17	690. 5.25	57 5.
ROUTED TO□	RPT17	5.43	1	FLOW TIME	1315. 5.67	1240. 5.67	1136. 5.67	1031. 5.67	953. 5.67	815. 5.67	670. 5.75	55 5.
HYDROGRAPH AT□												



	B18A	1.50	1	FLOW TIME	162. 5.25	150. 5.25	133. 5.25	116. 5.25	104. 5.25	82. 5.33	61. 5.33	4 5.
COMBINED AT□	PT18	89.59	1	FLOW TIME	6565. 6.92	6295. 6.92	5922. 6.92	5549. 7.00	5274. 7.00	4781. 7.08	4114. 7.25	353 7.
DIVERSION TO□	DVI154	89.59	1	FLOW TIME	1301. 6.92	1291. 6.92	1277. 6.92	1263. 7.00	1252. 7.00	1232. 7.08	1204. 7.25	115 7.
HYDROGRAPH AT□	DVI154	89.59	1	FLOW TIME	5264. 6.92	5004. 6.92	4646. 6.92	4286. 7.00	4022. 7.00	3549. 7.08	2910. 7.25	238 7.
ROUTED TO□	RPT18	89.59	1	FLOW TIME	5255. 7.00	4995. 7.08	4639. 7.08	4280. 7.08	4015. 7.08	3541. 7.17	2905. 7.33	238 7.
HYDROGRAPH AT□	B18B	1.92	1	FLOW TIME	677. 4.08	640. 4.08	589. 4.08	538. 4.08	500. 4.08	431. 4.08	358. 4.08	30 4.
HYDROGRAPH AT□	DVBD2	.00	1	FLOW TIME	350. 5.00	337. 5.00	319. 5.00	301. 5.00	288. 5.08	263. 5.08	237. 5.08	21 5.
ROUTED TO□	RDVBD2	.00	1	FLOW TIME	343. 5.58	331. 5.58	314. 5.58	296. 5.58	283. 5.58	259. 5.58	233. 5.58	21 5.
3 COMBINED AT□	PT19	91.51	1	FLOW TIME	5508. 6.92	5238. 6.92	4868. 7.00	4496. 7.00	4221. 7.00	3721. 7.08	3058. 7.25	251 7.
DIVERSION TO□	DVI155	91.51	1	FLOW TIME	2974. 6.92	2815. 6.92	2624. 7.00	2513. 7.00	2416. 7.00	2277. 7.08	2089. 7.25	183 7.
HYDROGRAPH AT□	DVI155	91.51	1	FLOW TIME	2534. 6.92	2423. 6.92	2244. 7.00	1983. 7.00	1806. 7.00	1444. 7.08	968. 7.25	68 7.
ROUTED TO□	RPT19	91.51	1	FLOW TIME	2522. 7.58	2412. 7.58	2226. 7.58	1967. 7.67	1790. 7.67	1423. 7.75	953. 7.92	67 8.
HYDROGRAPH AT□	B14A	1.35	1	FLOW TIME	670. 3.75	637. 3.75	590. 3.75	543. 3.75	507. 3.75	443. 3.75	375. 3.75	32 3.
HYDROGRAPH AT□	RDVBD3	.00	1	FLOW TIME	273. 5.08	271. 5.08	268. 5.08	265. 5.08	263. 5.08	259. 5.08	255. 5.08	25 5.
HYDROGRAPH AT□	RDVBD4	.00	1	FLOW TIME	178. 5.17	173. 5.17	165. 5.17	158. 5.17	152. 5.17	140. 5.17	126. 5.17	11 5.
2 COMBINED AT□	PT20	.00	1	FLOW TIME	450. 5.17	443. 5.17	433. 5.17	423. 5.17	415. 5.17	399. 5.17	381. 5.17	36 5.
ROUTED TO□	RDVBD4	.00	1	FLOW TIME	444. 5.58	436. 5.58	427. 5.58	416. 5.58	408. 5.58	392. 5.67	375. 5.67	36 5.
3 COMBINED AT□	PT21	92.86	1	FLOW TIME	2750. 7.00	2618. 7.00	2361. 7.17	2075. 7.25	1873. 7.42	1462. 7.67	966. 7.83	68 7.
2 COMBINED AT□	PT22	100.25	1	FLOW TIME	2808. 6.92	2668. 7.00	2404. 7.08	2115. 7.25	1910. 7.42	1496. 7.67	1000. 7.83	88 5.
ROUTED TO□	RPT22	100.25	1	FLOW TIME	2800. 7.17	2658. 7.25	2398. 7.42	2110. 7.58	1903. 7.67	1487. 7.92	994. 8.08	87 5.

HYDROGRAPH AT□	B15B	.95	1	FLOW TIME	199. 4.17	186. 4.17	168. 4.17	149. 4.17	136. 4.17	113. 4.17	90. 4.17	7 4.
COMBINED AT□	PT23	101.20	1	FLOW TIME	2809. 7.17	2664. 7.25	2402. 7.42	2113. 7.50	1905. 7.67	1488. 7.92	1063. 5.33	94 5.
HYDROGRAPH AT□	RDVI151	.00	1	FLOW TIME	1276. 5.08	1255. 5.08	1227. 5.17	1198. 5.25	1171. 5.25	1121. 5.33	1049. 5.33	96 5.
ROUTED TO□	RDVI151	.00	1	FLOW TIME	1276. 5.17	1255. 5.25	1227. 5.25	1198. 5.33	1170. 5.33	1120. 5.42	1048. 5.42	95 5.
HYDROGRAPH AT□	RDVI152	.00	1	FLOW TIME	380. 5.17	341. 5.17	285. 5.25	229. 5.25	190. 5.33	112. 5.33	39. 5.42	5.
ROUTED TO□	RDVI152	.00	1	FLOW TIME	379. 5.25	341. 5.25	285. 5.33	228. 5.33	190. 5.42	111. 5.42	39. 5.50	5.
2 COMBINED AT□	PT24	.00	1	FLOW TIME	1655. 5.25	1595. 5.25	1511. 5.33	1426. 5.33	1359. 5.33	1231. 5.42	1087. 5.50	95 5.
ROUTED TO□	RPT24	.00	1	FLOW TIME	1654. 5.33	1595. 5.33	1510. 5.42	1426. 5.42	1359. 5.42	1230. 5.50	1086. 5.58	95 5.
HYDROGRAPH AT□	D009	3.62	1	FLOW TIME	1626. 3.92	1535. 3.92	1408. 3.92	1280. 3.92	1185. 3.92	1015. 3.92	836. 3.92	69 3.
ROUTED TO□	RD009	3.62	1	FLOW TIME	1586. 4.00	1496. 4.00	1372. 4.00	1247. 4.00	1154. 4.00	988. 4.00	812. 4.00	67 4.
HYDROGRAPH AT□	D004	2.40	1	FLOW TIME	1001. 4.08	947. 4.08	873. 4.08	798. 4.08	743. 4.08	643. 4.17	537. 4.17	45 4.
2 COMBINED AT□	PT5	6.02	1	FLOW TIME	2567. 4.08	2426. 4.08	2232. 4.08	2035. 4.08	1889. 4.08	1626. 4.08	1347. 4.08	112 4.
ROUTED TO□	RPT5	6.02	1	FLOW TIME	2262. 4.50	2137. 4.50	1965. 4.50	1790. 4.50	1661. 4.50	1429. 4.50	1183. 4.50	98 4.
HYDROGRAPH AT□	D08	11.91	1	FLOW TIME	3172. 5.50	3008. 5.50	2780. 5.50	2551. 5.58	2381. 5.58	2074. 5.58	1746. 5.58	148 5.
2 COMBINED AT□	PT6	17.93	1	FLOW TIME	4808. 5.50	4564. 5.50	4228. 5.50	3885. 5.50	3630. 5.50	3169. 5.50	2677. 5.58	228 5.
ROUTED TO□	RPT6	17.93	1	FLOW TIME	4692. 6.08	4448. 6.08	4111. 6.08	3769. 6.08	3515. 6.08	3059. 6.17	2574. 6.17	218 6.
HYDROGRAPH AT□	D010	10.43	1	FLOW TIME	3045. 4.50	2873. 4.50	2636. 4.50	2397. 4.50	2220. 4.50	1905. 4.58	1573. 4.58	130 4.
HYDROGRAPH AT□	D07	4.28	1	FLOW TIME	1249. 4.58	1180. 4.58	1083. 4.58	986. 4.58	914. 4.58	785. 4.67	650. 4.67	54 4.
3 COMBINED AT□	PT7	32.65	1	FLOW TIME	8018. 5.50	7594. 5.50	7008. 5.50	6413. 5.50	5972. 5.50	5180. 5.58	4341. 5.58	366 5.
ROUTED TO□	RPT7	32.65	1	FLOW TIME	7984. 5.75	7562. 5.75	6979. 5.75	6388. 5.75	5950. 5.75	5158. 5.75	4323. 5.83	364 5.

HYDROGRAPH AT□	D012B	.57	1	FLOW TIME	165. 4.08	155. 4.08	141. 4.08	127. 4.08	117. 4.08	98. 4.08	79. 4.08	6 4.
2 COMBINED AT□	PT8	33.21	1	FLOW TIME	8053. 5.67	7625. 5.75	7037. 5.75	6442. 5.75	6000. 5.75	5202. 5.75	4355. 5.75	367 5.
DIVERSION TO□	DVPITT	33.21	1	FLOW TIME	4122. 5.67	3866. 5.75	3516. 5.75	3160. 5.75	2896. 5.75	2442. 5.75	1967. 5.75	160 5.
HYDROGRAPH AT□	DVPITT	33.21	1	FLOW TIME	3931. 5.67	3759. 5.75	3522. 5.75	3282. 5.75	3104. 5.75	2760. 5.75	2387. 5.75	207 5.
DIVERSION TO□	DVROSE	33.21	1	FLOW TIME	806. 5.67	763. 5.75	704. 5.75	644. 5.75	600. 5.75	520. 5.75	435. 5.75	39 5.
HYDROGRAPH AT□	DVROSE	33.21	1	FLOW TIME	3125. 5.67	2996. 5.75	2818. 5.75	2638. 5.75	2504. 5.75	2240. 5.75	1952. 5.75	168 5.
ROUTED TO□	RPT8	33.21	1	FLOW TIME	3105. 6.17	2976. 6.17	2799. 6.25	2620. 6.25	2487. 6.25	2224. 6.25	1938. 6.25	166 6.
HYDROGRAPH AT□	D4A	1.87	1	FLOW TIME	918. 3.67	864. 3.67	791. 3.67	717. 3.67	662. 3.67	564. 3.67	460. 3.67	37 3.
3 COMBINED AT□	PT25A	35.09	1	FLOW TIME	4682. 5.92	4482. 5.92	4243. 6.00	3982. 6.00	3783. 6.00	3397. 6.00	2973. 6.08	253 6.
HYDROGRAPH AT□	RDVI153	.00	1	FLOW TIME	9190. 6.75	8562. 6.83	7702. 6.83	6840. 6.83	6212. 6.92	5088. 6.92	4041. 7.00	326 7.
ROUTED TO□	RDVI153	.00	1	FLOW TIME	9169. 6.92	8543. 6.92	7683. 6.92	6825. 7.00	6197. 7.00	5071. 7.00	4032. 7.08	326 7.
ROUTED TO□	RPT25	.00	1	FLOW TIME	9152. 7.00	8527. 7.00	7668. 7.08	6813. 7.08	6184. 7.08	5060. 7.17	4024. 7.17	325 7.
2 COMBINED AT□	PT25B	35.09	1	FLOW TIME	13083. 6.75	12296. 6.75	11211. 6.75	10107. 6.75	9275. 6.75	7766. 6.75	6368. 6.75	523 6.
HYDROGRAPH AT□	D4B	2.03	1	FLOW TIME	1061. 3.67	1003. 3.67	923. 3.67	842. 3.67	782. 3.67	674. 3.67	560. 3.67	46 3.
2 COMBINED AT□	PT26	37.12	1	FLOW TIME	13084. 6.75	12297. 6.75	11212. 6.75	10108. 6.75	9276. 6.75	7767. 6.75	6369. 6.75	523 6.
ROUTED TO□	RPT26	37.12	1	FLOW TIME	13069. 6.83	12283. 6.83	11198. 6.83	10093. 6.83	9263. 6.83	7760. 6.83	6361. 6.83	523 6.
HYDROGRAPH AT□	RDVI154	.00	1	FLOW TIME	1301. 6.92	1291. 6.92	1277. 6.92	1263. 7.00	1252. 7.00	1232. 7.08	1204. 7.25	115 7.
ROUTED TO□	RDVI154	.00	1	FLOW TIME	1301. 6.92	1291. 7.00	1277. 7.00	1262. 7.00	1252. 7.00	1232. 7.08	1204. 7.25	115 7.
ROUTED TO□	RDVI154	.00	1	FLOW TIME	1301. 7.00	1291. 7.00	1277. 7.00	1262. 7.00	1252. 7.00	1232. 7.17	1204. 7.25	115 7.
HYDROGRAPH AT□	RDVI155	.00	1	FLOW	2974.	2815.	2624.	2513.	2416.	2277.	2089.	183

Flow from South  
of  
Cactus

Flow @ Southern Crossing  
of LV Blvd

Flow @ Northern Crossing  
of LV Blvd

Total Flow From Site

				TIME	6.92	6.92	7.00	7.00	7.00	7.08	7.25	7.
ROUTED TO	RDVI155	.00	1	FLOW TIME	2973. 6.92	2814. 7.00	2624. 7.00	2512. 7.00	2415. 7.00	2277. 7.17	2089. 7.33	183 7.
ROUTED TO	RDVI155	.00	1	FLOW TIME	2973. 7.00	2814. 7.00	2624. 7.00	2512. 7.00	2415. 7.00	2277. 7.17	2089. 7.33	183 7.
ROUTED TO	RDVI155	.00	1	FLOW TIME	2972. 7.00	2814. 7.00	2624. 7.00	2512. 7.00	2415. 7.08	2277. 7.17	2089. 7.33	183 7.
ROUTED TO	RDVI155	.00	1	FLOW TIME	2972. 7.00	2814. 7.00	2624. 7.00	2512. 7.00	2415. 7.08	2277. 7.17	2089. 7.33	183 7.
2 COMBINED AT	PT27	.00	1	FLOW TIME	4273. 7.00	4105. 7.00	3900. 7.00	3774. 7.00	3667. 7.08	3508. 7.17	3292. 7.33	298 7.
ROUTED TO	RPT27	.00	1	FLOW TIME	4273. 7.00	4104. 7.00	3900. 7.00	3774. 7.00	3667. 7.08	3508. 7.17	3292. 7.33	298 7.
ROUTED TO	RPT27	.00	1	FLOW TIME	4273. 7.00	4104. 7.00	3900. 7.00	3774. 7.08	3666. 7.08	3508. 7.17	3292. 7.33	298 7.
HYDROGRAPH AT	B18C	1.26	1	FLOW TIME	445. 3.75	419. 3.75	384. 3.75	349. 3.75	324. 3.75	278. 3.75	231. 3.75	19 3.
3 COMBINED AT	PT28	38.38	1	FLOW TIME	17333. 6.83	16376. 6.83	15092. 6.83	13857. 6.83	12917. 6.83	11242. 6.92	9593. 6.92	806 7.
ROUTED TO	LDCDB	38.38	1	FLOW TIME	11547. 8.33	11008. 8.33	10290. 8.33	9585. 8.25	9071. 8.25	8182. 8.25	7226. 8.17	632 8.
** PEAK STAGES IN FEET **												
			1	STAGE TIME	2219.28 8.33	2216.94 8.33	2213.83 8.33	2210.78 8.25	2208.56 8.25	2204.71 8.25	2200.85 8.17	2197. 8.
ROUTED TO	RLDCDB	38.38	1	FLOW TIME	11547. 8.33	11007. 8.33	10290. 8.33	9584. 8.33	9070. 8.25	8181. 8.25	7225. 8.25	632 8.
HYDROGRAPH AT	D5A	1.33	1	FLOW TIME	738. 3.75	703. 3.75	654. 3.75	605. 3.75	568. 3.75	502. 3.75	431. 3.75	37 3.
ROUTED TO	RD5A	1.33	1	FLOW TIME	702. 3.83	669. 3.83	623. 3.83	575. 3.83	540. 3.83	477. 3.83	409. 3.83	35 3.
HYDROGRAPH AT	RDVROSE	.00	1	FLOW TIME	806. 5.67	763. 5.75	704. 5.75	644. 5.75	600. 5.75	520. 5.75	435. 5.75	39 5.
ROUTED TO	RDVROSE	.00	1	FLOW TIME	799. 6.17	756. 6.17	698. 6.25	638. 6.25	594. 6.25	515. 6.25	431. 6.25	39 6.
3 COMBINED AT	PT29	39.70	1	FLOW TIME	11892. 8.17	11343. 8.17	10609. 8.17	9886. 8.08	9358. 8.08	8439. 8.08	7452. 8.00	652 8.
ROUTED TO	RPT29	39.70	1	FLOW TIME	11891. 8.25	11342. 8.17	10609. 8.17	9885. 8.17	9357. 8.08	8439. 8.08	7452. 8.08	652 8.
HYDROGRAPH AT	B16B	.70	1	FLOW TIME	348. 3.75	332. 3.75	309. 3.75	285. 3.75	268. 3.75	236. 3.75	203. 3.75	17 3.
ROUTED TO												

	RB16B	.70	1	FLOW TIME	344. 3.83	327. 3.83	305. 3.83	282. 3.83	264. 3.83	235. 3.83	202. 3.83	17 3.
COMBINED AT□	PT30	40.40	1	FLOW TIME	11891. 8.25	11342. 8.17	10609. 8.17	9885. 8.17	9357. 8.08	8439. 8.08	7452. 8.08	652 8.
ROUTED TO□	RPT30	40.40	1	FLOW TIME	11891. 8.25	11340. 8.25	10607. 8.17	9885. 8.17	9355. 8.17	8438. 8.08	7452. 8.08	652 8.
HYDROGRAPH AT□	B16A	.91	1	FLOW TIME	236. 4.17	222. 4.17	203. 4.17	184. 4.17	170. 4.17	144. 4.17	118. 4.17	9 4.
HYDROGRAPH AT□	D5B	.60	1	FLOW TIME	242. 4.08	231. 4.08	216. 4.08	201. 4.08	190. 4.08	170. 4.08	148. 4.08	13 4.
3 COMBINED AT□	PT31	41.91	1	FLOW TIME	11892. 8.25	11341. 8.25	10608. 8.17	9886. 8.17	9356. 8.08	8439. 8.08	7453. 8.08	652 8.
ROUTED TO□	RPT31	41.91	1	FLOW TIME	11891. 8.25	11340. 8.25	10607. 8.17	9885. 8.17	9356. 8.17	8439. 8.08	7452. 8.08	652 8.
2 COMBINED AT□	PT32	143.12	1	FLOW TIME	14470. 8.08	13813. 8.08	12891. 8.00	11920. 8.00	11216. 8.00	9921. 8.00	8447. 8.08	724 8.
ROUTED TO□	RPT32	143.12	1	FLOW TIME	14469. 8.08	13811. 8.08	12889. 8.08	11918. 8.00	11215. 8.00	9920. 8.08	8445. 8.08	724 8.
HYDROGRAPH AT□	HYDRO2	.38	1	FLOW TIME	36. 5.25	36. 5.25	36. 5.25	36. 5.25	36. 5.25	36. 5.25	36. 5.25	3 5.
ROUTED TO□	RHYDRO2	.38	1	FLOW TIME	32. 6.08	32. 6.08	32. 6.08	32. 6.08	32. 6.08	32. 6.08	32. 6.08	3 6.
HYDROGRAPH AT□	B12	.97	1	FLOW TIME	119. 5.00	110. 5.00	98. 5.08	86. 5.08	77. 5.08	61. 5.08	45. 5.08	3 5.
2 COMBINED AT□	PT33W	1.35	1	FLOW TIME	149. 5.08	140. 5.08	128. 5.08	115. 5.08	107. 5.08	91. 5.08	75. 5.08	6 5.
HYDROGRAPH AT□	RDVBD7	.00	1	FLOW TIME	918. 4.33	873. 4.33	812. 4.33	751. 4.33	705. 4.33	626. 4.33	542. 4.33	48 4.
ROUTED TO□	RDVBD7	.00	1	FLOW TIME	879. 4.58	836. 4.58	777. 4.58	718. 4.58	674. 4.58	597. 4.58	516. 4.58	45 4.
2 COMBINED AT□	PT33	1.35	1	FLOW TIME	956. 4.58	907. 4.58	841. 4.58	775. 4.58	725. 4.58	640. 4.58	550. 4.58	48 4.
ROUTED TO□	RPT33	1.35	1	FLOW TIME	929. 4.92	881. 4.92	815. 4.92	750. 4.92	702. 4.92	618. 4.92	530. 4.92	46 4.
HYDROGRAPH AT□	B15A	.96	1	FLOW TIME	279. 3.92	262. 3.92	238. 3.92	214. 3.92	197. 3.92	166. 3.92	134. 3.92	10 3.
2 COMBINED AT□	PT34W	2.31	1	FLOW TIME	1103. 5.00	1046. 5.00	968. 5.00	889. 5.00	831. 5.00	729. 5.00	622. 5.00	54 4.
2 COMBINED AT□	PT34	145.42	1	FLOW TIME	14546. 8.08	13888. 8.08	12967. 8.00	11996. 8.00	11293. 8.00	9997. 8.00	8521. 8.08	731 8.

HYDROGRAPH AT□	MNWB	.44	1	FLOW TIME	136. 4.17	128. 4.17	118. 4.17	107. 4.17	99. 4.17	85. 4.17	70. 4.17	5 4.
ROUTED TO□	RMNWB	.44	1	FLOW TIME	136. 4.25	128. 4.25	117. 4.25	107. 4.33	99. 4.33	85. 4.33	70. 4.33	5 4.
HYDROGRAPH AT□	MNWC	.60	1	FLOW TIME	191. 4.17	180. 4.17	166. 4.17	151. 4.17	141. 4.17	121. 4.17	101. 4.17	8 4.
2 COMBINED AT□	PT35	1.04	1	FLOW TIME	324. 4.17	306. 4.25	281. 4.25	256. 4.25	238. 4.25	205. 4.25	169. 4.25	14 4.
ROUTED TO□	RPT35	1.04	1	FLOW TIME	324. 4.25	306. 4.25	281. 4.25	256. 4.25	237. 4.25	203. 4.25	169. 4.33	14 4.
HYDROGRAPH AT□	MNWD	.08	1	FLOW TIME	39. 3.67	37. 3.67	34. 3.67	31. 3.67	29. 3.67	25. 3.67	21. 3.67	1 3.
2 COMBINED AT□	PT36	1.12	1	FLOW TIME	340. 4.25	321. 4.25	295. 4.25	269. 4.25	250. 4.25	215. 4.25	178. 4.25	14 4.
ROUTED TO□	RPT36	1.12	1	FLOW TIME	333. 4.42	315. 4.42	289. 4.42	263. 4.42	244. 4.42	210. 4.50	174. 4.50	14 4.
HYDROGRAPH AT□	MC2	.92	1	FLOW TIME	218. 4.33	206. 4.33	190. 4.33	173. 4.33	161. 4.33	140. 4.33	118. 4.33	10 4.
2 COMBINED AT□	PT37	2.03	1	FLOW TIME	548. 4.42	518. 4.42	476. 4.42	434. 4.42	403. 4.42	347. 4.42	289. 4.42	24 4.
ROUTED TO□	RPT37	2.03	1	FLOW TIME	548. 4.50	518. 4.50	476. 4.50	434. 4.50	403. 4.50	347. 4.50	289. 4.50	24 4.
HYDROGRAPH AT□	B10A	1.60	1	FLOW TIME	776. 3.58	735. 3.58	678. 3.58	621. 3.58	579. 3.58	504. 3.58	427. 3.58	36 3.
2 COMBINED AT□	PT38	3.64	1	FLOW TIME	917. 3.67	866. 3.67	795. 3.67	725. 3.67	678. 4.83	594. 4.83	504. 4.83	43 4.
ROUTED TO□	RPT38	3.64	1	FLOW TIME	907. 3.67	851. 3.67	784. 3.67	721. 4.92	674. 4.92	591. 4.92	502. 4.92	43 4.
2 COMBINED AT□	PT39	149.06	1	FLOW TIME	14559. 8.08	13901. 8.08	12983. 8.00	12011. 8.00	11308. 8.00	10011. 8.00	8531. 8.08	732 8.
ROUTED TO□	RPT39	149.06	1	FLOW TIME	14557. 8.08	13899. 8.08	12980. 8.08	12008. 8.00	11307. 8.00	10009. 8.08	8529. 8.08	732 8.
HYDROGRAPH AT□	D6	1.56	1	FLOW TIME	655. 3.83	624. 3.83	582. 3.92	539. 3.92	507. 3.92	451. 3.92	390. 3.92	34 3.
2 COMBINED AT□	PT40	150.62	1	FLOW TIME	14557. 8.08	13900. 8.08	12980. 8.08	12008. 8.00	11307. 8.00	10009. 8.08	8530. 8.08	732 8.
ROUTED TO□	RPT40	150.62	1	FLOW TIME	14553. 8.17	13895. 8.17	12975. 8.17	12004. 8.08	11303. 8.08	10005. 8.17	8527. 8.17	732 8.
HYDROGRAPH AT□	D7	.85	1	FLOW TIME	231. 4.17	218. 4.17	200. 4.17	182. 4.17	169. 4.17	145. 4.17	121. 4.17	10 4.

2 COMBINED AT□	PT41	151.47	1	FLOW TIME	14554. 8.17	13896. 8.17	12976. 8.17	12005. 8.08	11304. 8.08	10006. 8.17	8527. 8.17	732 8.
ROUTED TO□	RPT41	151.47	1	FLOW TIME	14552. 8.17	13894. 8.17	12975. 8.17	12003. 8.17	11301. 8.08	10005. 8.17	8526. 8.25	732 8.
HYDROGRAPH AT□	D8	1.26	1	FLOW TIME	661. 3.58	627. 3.58	581. 3.58	534. 3.58	500. 3.58	439. 3.58	374. 3.58	32 3.
ROUTED TO□	RD8	1.26	1	FLOW TIME	626. 3.67	595. 3.67	551. 3.67	507. 3.67	474. 3.67	416. 3.67	355. 3.67	30 3.
2 COMBINED AT□	PT42	152.73	1	FLOW TIME	14552. 8.17	13894. 8.17	12975. 8.17	12003. 8.17	11301. 8.08	10005. 8.17	8526. 8.25	732 8.
ROUTED TO□	RPT42	152.73	1	FLOW TIME	14550. 8.17	13892. 8.17	12974. 8.17	12002. 8.17	11300. 8.17	10004. 8.17	8526. 8.25	732 8.
HYDROGRAPH AT□	B10B	.85	1	FLOW TIME	284. 4.08	271. 4.08	252. 4.08	233. 4.08	220. 4.08	195. 4.08	170. 4.08	14 4.
ROUTED TO□	RB10B	.85	1	FLOW TIME	270. 4.58	257. 4.58	239. 4.58	221. 4.58	208. 4.58	185. 4.58	161. 4.58	14 4.
HYDROGRAPH AT□	D9A	.86	1	FLOW TIME	133. 4.67	124. 4.67	112. 4.67	100. 4.67	92. 4.67	77. 4.67	63. 4.67	5 5.
2 COMBINED AT□	PT43	1.71	1	FLOW TIME	402. 4.58	381. 4.58	351. 4.58	321. 4.58	300. 4.58	262. 4.58	223. 4.58	19 4.
ROUTED TO□	RPT43	1.71	1	FLOW TIME	402. 4.67	381. 4.67	351. 4.67	321. 4.67	300. 4.67	262. 4.67	223. 4.67	19 4.
2 COMBINED AT□	PT44	154.44	1	FLOW TIME	14563. 8.17	13905. 8.17	12986. 8.17	12013. 8.17	11310. 8.17	10013. 8.17	8532. 8.25	732 8.
ROUTED TO□	RPT44	154.44	1	FLOW TIME	14554. 8.33	13895. 8.33	12975. 8.25	12004. 8.25	11301. 8.25	10002. 8.25	8526. 8.33	732 8.
HYDROGRAPH AT□	D9B	1.54	1	FLOW TIME	518. 3.83	487. 3.83	445. 3.83	402. 3.83	371. 3.83	316. 3.83	258. 3.83	21 3.
2 COMBINED AT□	PT45	155.99	1	FLOW TIME	14554. 8.33	13895. 8.33	12975. 8.25	12004. 8.25	11301. 8.25	10002. 8.25	8526. 8.33	732 8.
ROUTED TO□	RPT45	155.99	1	FLOW TIME	14544. 8.50	13885. 8.50	12961. 8.42	11991. 8.42	11289. 8.42	9993. 8.50	8517. 8.50	731 8.
HYDROGRAPH AT□	D10	2.51	1	FLOW TIME	1032. 3.83	980. 3.83	908. 3.83	836. 3.83	783. 3.83	687. 3.83	586. 3.83	50 3.
2 COMBINED AT□	PT46	158.49	1	FLOW TIME	14544. 8.50	13885. 8.50	12961. 8.42	11991. 8.42	11289. 8.42	9993. 8.50	8517. 8.50	731 8.
ROUTED TO□	RPT46	158.49	1	FLOW TIME	14543. 8.50	13883. 8.50	12960. 8.50	11989. 8.50	11287. 8.50	9992. 8.50	8516. 8.58	731 8.
HYDROGRAPH AT□	MC	2.80	1	FLOW	1911.	1824.	1704.	1582.	1490.	1323.	1142.	99

				TIME	3.67	3.67	3.67	3.67	3.67	3.67	3.67	3.67
DIVERSION TO	TORAW	2.80	1	FLOW TIME	382. 3.67	365. 3.67	341. 3.67	316. 3.67	298. 3.67	265. 3.67	228. 3.67	19 3.
HYDROGRAPH AT	TORAW	2.80	1	FLOW TIME	1528. 3.67	1459. 3.67	1364. 3.67	1265. 3.67	1192. 3.67	1058. 3.67	913. 3.67	79 3.
ROUTED TO	MCDB	2.80	1	FLOW TIME	76. 5.67	75. 5.67	72. 5.67	69. 5.67	67. 5.67	63. 5.58	59. 5.58	5 5.
				** PEAK STAGES IN FEET **								
				1 STAGE TIME	11.05 5.67	10.55 5.67	9.85 5.67	9.14 5.67	8.61 5.67	7.68 5.58	6.71 5.58	5. 5.
ROUTED TO	RMCDDB	2.80	1	FLOW TIME	76. 5.75	75. 5.75	72. 5.75	69. 5.75	67. 5.75	63. 5.67	59. 5.67	5 5.
HYDROGRAPH AT	RTORAW	.00	1	FLOW TIME	382. 3.67	365. 3.67	341. 3.67	316. 3.67	298. 3.67	265. 3.67	228. 3.67	19 3.
2 COMBINED AT	PT47	2.80	1	FLOW TIME	430. 3.67	411. 3.67	384. 3.67	357. 3.67	338. 3.67	302. 3.67	264. 3.67	23 3.
ROUTED TO	RPT47	2.80	1	FLOW TIME	434. 3.83	414. 3.83	386. 3.83	358. 3.83	337. 3.83	302. 3.92	266. 3.92	23 3.
HYDROGRAPH AT	RH	1.36	1	FLOW TIME	288. 3.75	263. 3.75	231. 3.75	199. 3.75	176. 3.75	136. 3.75	107. 5.00	8 5.
2 COMBINED AT	PT48W	4.16	1	FLOW TIME	712. 3.83	669. 3.83	611. 3.83	552. 3.83	509. 3.83	433. 3.83	360. 3.92	30 3.
HYDROGRAPH AT	B20	.79	1	FLOW TIME	228. 4.17	215. 4.17	197. 4.17	178. 4.17	165. 4.17	140. 4.17	115. 4.17	9 4.
ROUTED TO	RB20	.79	1	FLOW TIME	227. 4.17	214. 4.17	196. 4.25	178. 4.25	164. 4.25	140. 4.25	115. 4.25	9 4.
3 COMBINED AT	PT48	163.44	1	FLOW TIME	14617. 8.50	13955. 8.50	13030. 8.50	12056. 8.50	11352. 8.50	10052. 8.50	8572. 8.58	736 8.
ROUTED TO	RPT48	163.44	1	FLOW TIME	14614. 8.58	13952. 8.58	13026. 8.50	12054. 8.50	11350. 8.50	10049. 8.58	8569. 8.58	736 8.
HYDROGRAPH AT	D12	.18	1	FLOW TIME	39. 4.17	36. 4.17	33. 4.17	29. 4.17	26. 4.17	22. 4.17	17. 4.25	1 4.
ROUTED TO	RD12	.18	1	FLOW TIME	39. 4.42	36. 4.42	33. 4.50	29. 4.50	26. 4.50	22. 4.50	17. 4.58	1 4.
HYDROGRAPH AT	D11A	.16	1	FLOW TIME	67. 3.58	63. 3.58	56. 3.58	50. 3.58	46. 3.58	37. 3.58	29. 3.58	2 3.
2 COMBINED AT	PT49	.33	1	FLOW TIME	72. 4.83	68. 4.83	62. 4.83	56. 4.83	52. 4.83	44. 4.83	36. 4.83	3 4.
ROUTED TO	RPT49	.33	1	FLOW TIME	71. 5.00	67. 5.00	61. 5.00	55. 5.00	51. 5.00	44. 5.00	36. 5.00	2 5.
2 COMBINED AT												



PTED TO

HYDROGRAPH AT

2 COMBINED AT

PT50	163.78	1	FLOW TIME	14615. 8.58	13953. 8.58	13027. 8.50	12055. 8.50	11351. 8.50	10050. 8.58	8570. 8.58	736 8.
RPT50	163.78	1	FLOW TIME	14610. 8.58	13949. 8.58	13024. 8.58	12052. 8.58	11350. 8.58	10049. 8.67	8570. 8.67	736 8.
D11B	.99	1	FLOW TIME	504. 3.58	476. 3.58	437. 3.58	397. 3.58	368. 3.58	316. 3.58	261. 3.58	21 3.
PT51	164.76	1	FLOW TIME	14610. 8.58	13949. 8.58	13024. 8.58	12052. 8.58	11350. 8.58	10049. 8.67	8570. 8.67	736 8.

□ □

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNGE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)	INTERPOLATED TO COMPUTATION INTERVAL			
						DT (MIN)	PEAK (CFS)	TIME TO PEAK (MIN)	VOLUME (IN)
FOR PLAN = 1	RATIO=	.00							
RPT2	MANE	4.57	513.74	306.00	.66	5.00	513.39	305.00	.66

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9296E+02 EXCESS= .0000E+00 OUTFLOW= .9298E+02 BASIN STORAGE= .3577E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT2	MANE	4.66	483.39	303.03	.62	5.00	483.38	305.00	.62

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8677E+02 EXCESS= .0000E+00 OUTFLOW= .8679E+02 BASIN STORAGE= .4353E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT2	MANE	4.81	442.44	307.67	.56	5.00	442.12	305.00	.56

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7833E+02 EXCESS= .0000E+00 OUTFLOW= .7836E+02 BASIN STORAGE= .3619E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT2	MANE	4.98	400.54	308.48	.50	5.00	400.28	305.00	.50

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6987E+02 EXCESS= .0000E+00 OUTFLOW= .6990E+02 BASIN STORAGE= .3813E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT2	MANE	5.00	369.94	305.00	.45	5.00	369.94	305.00	.45

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6367E+02 EXCESS= .0000E+00 OUTFLOW= .6370E+02 BASIN STORAGE= .3740E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT2	MANE	4.75	313.26	308.75	.37	5.00	312.61	310.00	.37

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5263E+02 EXCESS= .0000E+00 OUTFLOW= .5266E+02 BASIN STORAGE= .3643E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT2	MANE	4.25	252.88	310.25	.29	5.00	252.85	310.00	.29

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4109E+02 EXCESS= .0000E+00 OUTFLOW= .4112E+02 BASIN STORAGE= .3370E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT2	MANE	3.75	204.53	311.25	.23	5.00	204.46	310.00	.23

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3200E+02 EXCESS= .0000E+00 OUTFLOW= .3202E+02 BASIN STORAGE= .3489E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT3A	MANE	2.38	776.46	308.81	.52	5.00	773.44	310.00	.52

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1299E+03 EXCESS= .0000E+00 OUTFLOW= .1299E+03 BASIN STORAGE= .2096E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT3A	MANE	2.43	724.62	308.93	.48	5.00	721.99	310.00	.48

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1191E+03 EXCESS= .0000E+00 OUTFLOW= .1191E+03 BASIN STORAGE= .2715E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT3A	MANE	2.52	653.94	307.53	.42	5.00	653.06	310.00	.42

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1044E+03 EXCESS= .0000E+00 OUTFLOW= .1044E+03 BASIN STORAGE= .2081E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT3A	MANE	2.50	581.15	307.50	.36	5.00	581.10	310.00	.36

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8961E+02 EXCESS= .0000E+00 OUTFLOW= .8964E+02 BASIN STORAGE= .2347E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT3A	MANE	2.71	527.57	309.35	.32	5.00	526.42	310.00	.32

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7893E+02 EXCESS= .0000E+00 OUTFLOW= .7895E+02 BASIN STORAGE= .2328E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT3A	MANE	2.50	429.06	310.00	.24	5.00	429.06	310.00	.24

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5990E+02 EXCESS= .0000E+00 OUTFLOW= .5993E+02 BASIN STORAGE= .2076E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT3A	MANE	2.25	322.16	310.50	.16	5.00	322.01	310.00	.16

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4055E+02 EXCESS= .0000E+00 OUTFLOW= .4055E+02 BASIN STORAGE= .2685E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT3A	MANE	2.25	236.32	310.50	.11	5.00	236.02	310.00	.11

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2605E+02 EXCESS= .0000E+00 OUTFLOW= .2607E+02 BASIN STORAGE= .2439E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RD012A	MANE	1.48	2260.12	306.05	1.02	5.00	2260.09	305.00	1.02

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5551E+03 EXCESS= .0000E+00 OUTFLOW= .5550E+03 BASIN STORAGE= .2618E+00 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RD012A	MANE	1.51	2129.13	307.49	.96	5.00	2128.77	305.00	.96

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5217E+03 EXCESS= .0000E+00 OUTFLOW= .5215E+03 BASIN STORAGE= .2548E+00 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RD012A	MANE	1.55	1950.06	311.70	.87	5.00	1949.96	310.00	.87

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4759E+03 EXCESS= .0000E+00 OUTFLOW= .4758E+03 BASIN STORAGE= .2423E+00 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RD012A	MANE	1.60	1770.40	313.61	.79	5.00	1770.34	315.00	.79

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4301E+03 EXCESS= .0000E+00 OUTFLOW= .4299E+03 BASIN STORAGE= .2303E+00 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RD012A	MANE	1.64	1638.72	316.61	.73	5.00	1638.61	315.00	.73

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3964E+03 EXCESS= .0000E+00 OUTFLOW= .3963E+03 BASIN STORAGE= .2197E+00 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RD012A	MANE	1.73	1403.14	317.41	.62	5.00	1402.58	320.00	.62

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3367E+03 EXCESS= .0000E+00 OUTFLOW= .3366E+03 BASIN STORAGE= .2025E+00 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RD012A	MANE	1.84	1156.79	321.31	.50	5.00	1156.42	320.00	.50

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2747E+03 EXCESS= .0000E+00 OUTFLOW= .2746E+03 BASIN STORAGE= .1777E+00 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RD012A	MANE	1.95	961.15	323.63	.41	5.00	961.12	325.00	.41

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2259E+03 EXCESS= .0000E+00 OUTFLOW= .2258E+03 BASIN STORAGE= .1608E+00 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	2.64	1301.16	417.91	-1.00	5.00	1301.12	415.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	2.65	1290.94	419.04	-1.00	5.00	1290.91	420.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	2.66	1276.77	420.64	-1.00	5.00	1276.77	420.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	2.67	1262.48	419.60	-1.00	5.00	1262.47	420.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	2.68	1251.97	420.81	-1.00	5.00	1251.93	420.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	2.70	1231.74	425.87	-1.00	5.00	1231.69	425.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	2.72	1203.64	434.70	-1.00	5.00	1203.64	435.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	2.76	1151.04	441.44	-1.00	5.00	1151.02	440.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	.79	1301.12	416.32	-1.00	5.00	1301.10	420.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	.80	1290.90	420.03	-1.00	5.00	1290.90	420.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	.80	1276.75	420.29	-1.00	5.00	1276.74	420.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	.80	1262.46	420.57	-1.00	5.00	1262.44	420.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	.81	1251.93	421.18	-1.00	5.00	1251.88	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	.81	1231.69	426.37	-1.00	5.00	1231.65	430.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	.82	1203.62	436.05	-1.00	5.00	1203.56	435.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI154	MANE	.83	1150.99	440.58	-1.00	5.00	1150.96	440.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	1.46	2973.79	417.19	-1.00	5.00	2972.91	415.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	1.49	2814.95	416.92	-1.00	5.00	2814.29	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	1.52	2624.17	420.02	-1.00	5.00	2624.17	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	1.54	2512.50	421.09	-1.00	5.00	2512.35	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	1.56	2415.66	421.58	-1.00	5.00	2415.27	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	1.59	2277.15	427.79	-1.00	5.00	2277.06	430.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	1.63	2089.31	437.73	-1.00	5.00	2088.92	440.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	1.70	1836.00	442.03	-1.00	5.00	1835.04	440.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.81	2972.91	416.28	-1.00	5.00	2972.50	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.83	2814.18	419.71	-1.00	5.00	2814.15	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.85	2624.07	420.56	-1.00	5.00	2624.01	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.87	2512.26	421.01	-1.00	5.00	2512.07	420.00	-1.00



FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.88	2415.22	421.54	-1.00	5.00	2414.85	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.90	2276.98	430.70	-1.00	5.00	2276.94	430.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.93	2088.90	439.29	-1.00	5.00	2088.88	440.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.98	1835.04	441.05	-1.00	5.00	1834.57	445.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.53	2972.46	420.02	-1.00	5.00	2972.46	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.54	2814.01	420.20	-1.00	5.00	2813.98	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.55	2623.93	420.53	-1.00	5.00	2623.89	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.56	2512.01	420.63	-1.00	5.00	2511.85	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.57	2414.86	421.04	-1.00	5.00	2414.81	425.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.58	2276.87	430.48	-1.00	5.00	2276.83	430.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.60	2088.83	440.14	-1.00	5.00	2088.82	440.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.64	1834.49	445.13	-1.00	5.00	1834.48	445.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.55	2972.36	420.09	-1.00	5.00	2972.34	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.56	2813.84	420.39	-1.00	5.00	2813.77	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							
RDVI155	MANE	.58	2623.81	420.73	-1.00	5.00	2623.72	420.00	-1.00
FOR PLAN = 1	RATIO=	.00							

RDVI155	MANE	.59	2511.79	421.06	-1.00	5.00	2511.59	420.00	-1.00
FOR PLAN = 1 RATIO=									
RDVI155	MANE	.60	2414.77	424.75	-1.00	5.00	2414.76	425.00	-1.00
FOR PLAN = 1 RATIO=									
RDVI155	MANE	.61	2276.79	430.57	-1.00	5.00	2276.70	430.00	-1.00
FOR PLAN = 1 RATIO=									
RDVI155	MANE	.63	2088.75	440.60	-1.00	5.00	2088.74	440.00	-1.00
FOR PLAN = 1 RATIO=									
RDVI155	MANE	.67	1834.34	445.36	-1.00	5.00	1834.32	445.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.67	4273.25	420.32	-1.00	5.00	4273.21	420.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.68	4104.46	420.37	-1.00	5.00	4104.33	420.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.69	3900.40	420.73	-1.00	5.00	3900.24	420.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.70	3774.00	420.70	-1.00	5.00	3773.67	420.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.71	3666.57	424.88	-1.00	5.00	3666.56	425.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.72	3508.24	431.00	-1.00	5.00	3508.14	430.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.74	3292.01	440.63	-1.00	5.00	3292.00	440.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.77	2984.76	445.15	-1.00	5.00	2984.73	445.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.31	4273.08	420.08	-1.00	5.00	4273.06	420.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.32	4104.16	420.20	-1.00	5.00	4104.12	420.00	-1.00
FOR PLAN = 1 RATIO=									
RPT27	MANE	.33	3900.15	420.38	-1.00	5.00	3900.09	420.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RPT27	MANE	.33	3773.64	421.02	-1.00	5.00	3773.57	425.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RPT27	MANE	.33	3666.49	424.90	-1.00	5.00	3666.49	425.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RPT27	MANE	.34	3508.06	430.42	-1.00	5.00	3508.02	430.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RPT27	MANE	.35	3291.91	440.13	-1.00	5.00	3291.90	440.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RPT27	MANE	.36	2984.60	445.02	-1.00	5.00	2984.59	445.00	-1.00

FOR PLAN = 1	RATIO=	.00							
RLDCDB	MANE	.64	11546.51	500.36	2.11	5.00	11546.51	500.00	2.11

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4341E+04 EXCESS= .0000E+00 OUTFLOW= .4331E+04 BASIN STORAGE= .1129E+02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RLDCDB	MANE	.65	11007.49	500.18	2.01	5.00	11007.46	500.00	2.02

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4133E+04 EXCESS= .0000E+00 OUTFLOW= .4123E+04 BASIN STORAGE= .1097E+02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RLDCDB	MANE	.67	10289.53	499.48	1.88	5.00	10289.50	500.00	1.88

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3864E+04 EXCESS= .0000E+00 OUTFLOW= .3855E+04 BASIN STORAGE= .1051E+02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RLDCDB	MANE	.68	9584.50	496.33	1.75	5.00	9583.94	500.00	1.75

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3592E+04 EXCESS= .0000E+00 OUTFLOW= .3584E+04 BASIN STORAGE= .1005E+02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RLDCDB	MANE	.70	9070.50	496.07	1.65	5.00	9069.83	495.00	1.65

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3387E+04 EXCESS= .0000E+00 OUTFLOW= .3379E+04 BASIN STORAGE= .9691E+01 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RLDCDB	MANE	.73	8181.38	495.75	1.47	5.00	8181.08	495.00	1.47

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3020E+04 EXCESS= .0000E+00 OUTFLOW= .3012E+04 BASIN STORAGE= .8958E+01 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RLDCDB	MANE	.76	7225.62	491.71	1.27	5.00	7225.43	495.00	1.27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2608E+04 EXCESS= .0000E+00 OUTFLOW= .2602E+04 BASIN STORAGE= .8073E+01 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RLDCDB	MANE	.80	6326.03	490.14	1.09	5.00	6325.99	490.00	1.09

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2242E+04 EXCESS= .0000E+00 OUTFLOW= .2236E+04 BASIN STORAGE= .7203E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT29 MANE .75 11892.36 491.07 2.19 5.00 11890.97 495.00 2.19

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4661E+04 EXCESS= .0000E+00 OUTFLOW= .4648E+04 BASIN STORAGE= .1351E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT29 MANE .77 11342.60 491.31 2.09 5.00 11341.75 490.00 2.09

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4439E+04 EXCESS= .0000E+00 OUTFLOW= .4426E+04 BASIN STORAGE= .1313E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT29 MANE .78 10609.02 490.27 1.95 5.00 10608.84 490.00 1.95

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4142E+04 EXCESS= .0000E+00 OUTFLOW= .4130E+04 BASIN STORAGE= .1258E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT29 MANE .81 9885.72 486.34 1.81 5.00 9884.97 490.00 1.81

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3842E+04 EXCESS= .0000E+00 OUTFLOW= .3831E+04 BASIN STORAGE= .1204E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT29 MANE .82 9357.33 485.63 1.71 5.00 9356.84 485.00 1.71

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3632E+04 EXCESS= .0000E+00 OUTFLOW= .3621E+04 BASIN STORAGE= .1160E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT29 MANE .85 8438.53 484.92 1.52 5.00 8438.53 485.00 1.52

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3228E+04 EXCESS= .0000E+00 OUTFLOW= .3218E+04 BASIN STORAGE= .1074E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT29 MANE .89 7452.13 481.81 1.31 5.00 7451.73 485.00 1.31

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2789E+04 EXCESS= .0000E+00 OUTFLOW= .2780E+04 BASIN STORAGE= .9676E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT29 MANE .94 6527.45 480.65 1.13 5.00 6527.02 480.00 1.13

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2397E+04 EXCESS= .0000E+00 OUTFLOW= .2389E+04 BASIN STORAGE= .8649E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RB16B MANE 4.00 347.01 228.00 1.36 5.00 343.73 230.00 1.36

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5087E+02 EXCESS= .0000E+00 OUTFLOW= .5088E+02 BASIN STORAGE= .1161E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RB16B MANE 4.00 330.25 228.00 1.30 5.00 327.28 230.00 1.30

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4851E+02 EXCESS= .0000E+00 OUTFLOW= .4852E+02 BASIN STORAGE= .1127E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RB16B MANE 4.00 307.12 228.00 1.21 5.00 304.56 230.00 1.21

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4526E+02 EXCESS= .0000E+00 OUTFLOW= .4527E+02 BASIN STORAGE= .1078E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RB16B MANE 4.00 283.68 228.00 1.12 5.00 281.52 230.00 1.12

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4197E+02 EXCESS= .0000E+00 OUTFLOW= .4198E+02 BASIN STORAGE= .1028E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RB16B MANE 4.00 266.30 228.00 1.06 5.00 264.43 230.00 1.06

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3954E+02 EXCESS= .0000E+00 OUTFLOW= .3955E+02 BASIN STORAGE= .1524E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RB16B MANE 3.75 235.20 228.75 .94 5.00 234.66 230.00 .94

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3516E+02 EXCESS= .0000E+00 OUTFLOW= .3517E+02 BASIN STORAGE= .1424E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RB16B MANE 3.75 201.89 228.75 .82 5.00 201.66 230.00 .82

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3054E+02 EXCESS= .0000E+00 OUTFLOW= .3054E+02 BASIN STORAGE= .1302E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RB16B MANE 3.75 174.98 228.75 .72 5.00 174.98 230.00 .72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2682E+02 EXCESS= .0000E+00 OUTFLOW= .2682E+02 BASIN STORAGE= .1154E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT30 MANE 1.62 11891.10 493.19 2.16 5.00 11890.65 495.00 2.17

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4686E+04 EXCESS= .0000E+00 OUTFLOW= .4659E+04 BASIN STORAGE= .2782E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT30 MANE 1.64 11341.23 492.55 2.06 5.00 11339.68 495.00 2.06

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4477E+04 EXCESS= .0000E+00 OUTFLOW= .4451E+04 BASIN STORAGE= .2693E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT30 MANE 1.68 10608.14 491.52 1.92 5.00 10607.12 490.00 1.93

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4172E+04 EXCESS= .0000E+00 OUTFLOW= .4147E+04 BASIN STORAGE= .2575E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT30 MANE 1.72 9884.83 489.10 1.78 5.00 9884.64 490.00 1.79

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3873E+04 EXCESS= .0000E+00 OUTFLOW= .3850E+04 BASIN STORAGE= .2455E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT30 MANE 1.75 9356.44 487.35 1.68 5.00 9355.11 490.00 1.69

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3654E+04 EXCESS= .0000E+00 OUTFLOW= .3632E+04 BASIN STORAGE= .2360E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT30 MANE 1.81 8437.96 485.78 1.50 5.00 8437.83 485.00 1.50

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3257E+04 EXCESS= .0000E+00 OUTFLOW= .3236E+04 BASIN STORAGE= .2172E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT30 MANE 1.88 7451.61 484.96 1.29 5.00 7451.60 485.00 1.30



CONTINUITY SUMMARY (AC-FT) - INFLOW= .2811E+04 EXCESS= .0000E+00 OUTFLOW= .2793E+04 BASIN STORAGE= .1945E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT30 MANE 1.96 6526.70 482.57 1.11 5.00 6525.77 485.00 1.11

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2420E+04 EXCESS= .0000E+00 OUTFLOW= .2403E+04 BASIN STORAGE= .1725E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT31 MANE .96 11891.27 495.39 2.12 5.00 11891.07 495.00 2.12

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4759E+04 EXCESS= .0000E+00 OUTFLOW= .4743E+04 BASIN STORAGE= .1683E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT31 MANE .97 11340.54 494.70 2.02 5.00 11340.49 495.00 2.02

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4533E+04 EXCESS= .0000E+00 OUTFLOW= .4518E+04 BASIN STORAGE= .1631E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT31 MANE 1.00 10608.29 491.14 1.89 5.00 10606.96 490.00 1.89

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4238E+04 EXCESS= .0000E+00 OUTFLOW= .4224E+04 BASIN STORAGE= .1560E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT31 MANE 1.02 9885.57 490.85 1.75 5.00 9885.32 490.00 1.75

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3937E+04 EXCESS= .0000E+00 OUTFLOW= .3923E+04 BASIN STORAGE= .1489E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT31 MANE 1.04 9356.43 486.63 1.65 5.00 9356.24 490.00 1.65

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3710E+04 EXCESS= .0000E+00 OUTFLOW= .3697E+04 BASIN STORAGE= .1433E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT31 MANE 1.08 8438.89 485.73 1.47 5.00 8438.53 485.00 1.47

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3301E+04 EXCESS= .0000E+00 OUTFLOW= .3288E+04 BASIN STORAGE= .1323E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT31 MANE 1.12 7452.43 485.40 1.27 5.00 7452.31 485.00 1.27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2849E+04 EXCESS= .0000E+00 OUTFLOW= .2837E+04 BASIN STORAGE= .1189E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT31 MANE 1.18 6526.73 481.99 1.09 5.00 6526.66 485.00 1.09

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2453E+04 EXCESS= .0000E+00 OUTFLOW= .2443E+04 BASIN STORAGE= .1058E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT32 MANE 1.27 14469.61 486.25 .76 5.00 14468.71 485.00 .76

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5851E+04 EXCESS= .0000E+00 OUTFLOW= .5827E+04 BASIN STORAGE= .2655E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT32 MANE 1.29 13811.99 485.75 .72 5.00 13811.33 485.00 .73

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5560E+04 EXCESS= .0000E+00 OUTFLOW= .5536E+04 BASIN STORAGE= .2571E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT32 MANE 1.33 12890.95 481.24 .67 5.00 12889.14 485.00 .67

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5150E+04 EXCESS= .0000E+00 OUTFLOW= .5128E+04 BASIN STORAGE= .2458E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT32 MANE 1.37 11919.46 481.92 .62 5.00 11917.91 480.00 .62

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4740E+04 EXCESS= .0000E+00 OUTFLOW= .4719E+04 BASIN STORAGE= .2344E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT32 MANE 1.40 11215.25 480.47 .58 5.00 11214.61 480.00 .58

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4452E+04 EXCESS= .0000E+00 OUTFLOW= .4431E+04 BASIN STORAGE= .2253E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT32 MANE 1.46 9921.06 482.58 .51 5.00 9919.80 485.00 .51

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3893E+04 EXCESS= .0000E+00 OUTFLOW= .3873E+04 BASIN STORAGE= .2088E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT32 MANE 1.55 8446.42 486.16 .43 5.00 8445.10 485.00 .43

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3314E+04 EXCESS= .0000E+00 OUTFLOW= .3297E+04 BASIN STORAGE= .1880E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT32 MANE 1.65 7241.15 485.41 .37 5.00 7241.11 485.00 .37

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2824E+04 EXCESS= .0000E+00 OUTFLOW= .2809E+04 BASIN STORAGE= .1689E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMNWB MANE 5.00 135.71 255.00 1.05 5.00 135.71 255.00 1.05

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2476E+02 EXCESS= .0000E+00 OUTFLOW= .2477E+02 BASIN STORAGE= .2236E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMNWB MANE 5.00 127.98 255.00 .99 5.00 127.98 255.00 .99

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2335E+02 EXCESS= .0000E+00 OUTFLOW= .2335E+02 BASIN STORAGE= .2167E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMNWB MANE 5.00 117.32 255.00 .91 5.00 117.32 255.00 .91

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2141E+02 EXCESS= .0000E+00 OUTFLOW= .2141E+02 BASIN STORAGE= .1994E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMNWB MANE 5.00 106.60 260.00 .83 5.00 106.60 260.00 .83

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1946E+02 EXCESS= .0000E+00 OUTFLOW= .1946E+02 BASIN STORAGE= .1895E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMNWB MANE 5.00 98.79 260.00 .76 5.00 98.79 260.00 .76

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1802E+02 EXCESS= .0000E+00 OUTFLOW= .1802E+02 BASIN STORAGE= .1820E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RMNWB MANE 5.00 84.74 260.00 .66 5.00 84.74 260.00 .66

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1546E+02 EXCESS= .0000E+00 OUTFLOW= .1546E+02 BASIN STORAGE= .2188E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RMNWB MANE 5.00 69.85 260.00 .54 5.00 69.85 260.00 .54

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1279E+02 EXCESS= .0000E+00 OUTFLOW= .1279E+02 BASIN STORAGE= .2061E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RMNWB MANE 5.00 57.90 260.00 .45 5.00 57.90 260.00 .45

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1067E+02 EXCESS= .0000E+00 OUTFLOW= .1067E+02 BASIN STORAGE= .1794E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT37 MANE 5.00 547.98 270.00 1.07 5.00 547.98 270.00 1.07

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1157E+03 EXCESS= .0000E+00 OUTFLOW= .1157E+03 BASIN STORAGE= .1818E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT37 MANE 5.00 517.61 270.00 1.01 5.00 517.61 270.00 1.01

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1095E+03 EXCESS= .0000E+00 OUTFLOW= .1095E+03 BASIN STORAGE= .1764E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT37 MANE 5.00 475.85 270.00 .93 5.00 475.85 270.00 .93

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1011E+03 EXCESS= .0000E+00 OUTFLOW= .1011E+03 BASIN STORAGE= .1686E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT37 MANE 5.00 433.76 270.00 .85 5.00 433.76 270.00 .85

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9263E+02 EXCESS= .0000E+00 OUTFLOW= .9265E+02 BASIN STORAGE= .1606E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT37 MANE 5.00 402.79 270.00 .80 5.00 402.79 270.00 .80

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8641E+02 EXCESS= .0000E+00 OUTFLOW= .8642E+02 BASIN STORAGE= .1736E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT37 MANE 5.00 347.36 270.00 .69 5.00 347.36 270.00 .69

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7533E+02 EXCESS= .0000E+00 OUTFLOW= .7534E+02 BASIN STORAGE= .1608E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT37 MANE 5.00 289.36 270.00 .59 5.00 289.36 270.00 .59

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6377E+02 EXCESS= .0000E+00 OUTFLOW= .6378E+02 BASIN STORAGE= .1563E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT37 MANE 5.00 243.42 270.00 .50 5.00 243.42 270.00 .50

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5462E+02 EXCESS= .0000E+00 OUTFLOW= .5463E+02 BASIN STORAGE= .1433E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00

RPT38	MANE	3.39	909.98	220.30	1.08	5.00	906.65	220.00	1.08
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CONTINUITY SUMMARY (AC-FT) - INFLOW= .2098E+03 EXCESS= .0000E+00 OUTFLOW= .2098E+03 BASIN STORAGE= .1834E-01 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT38	MANE	3.46	856.55	221.16	1.03	5.00	850.67	220.00	1.03

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1991E+03 EXCESS= .0000E+00 OUTFLOW= .1991E+03 BASIN STORAGE= .1739E-01 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT38	MANE	3.50	788.85	220.50	.95	5.00	783.82	220.00	.95

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1844E+03 EXCESS= .0000E+00 OUTFLOW= .1844E+03 BASIN STORAGE= .1786E-01 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT38	MANE	3.50	722.86	294.00	.87	5.00	720.89	295.00	.87

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1697E+03 EXCESS= .0000E+00 OUTFLOW= .1697E+03 BASIN STORAGE= .1704E-01 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT38	MANE	3.50	676.04	294.00	.82	5.00	674.37	295.00	.82

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1589E+03 EXCESS= .0000E+00 OUTFLOW= .1589E+03 BASIN STORAGE= .1958E-01 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT38	MANE	3.50	591.67	294.00	.72	5.00	590.55	295.00	.72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1396E+03 EXCESS= .0000E+00 OUTFLOW= .1396E+03 BASIN STORAGE= .1823E-01 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT38	MANE	3.50	502.39	294.00	.62	5.00	501.80	295.00	.62

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1196E+03 EXCESS= .0000E+00 OUTFLOW= .1195E+03 BASIN STORAGE= .1762E-01 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT38	MANE	3.75	430.65	296.25	.53	5.00	429.53	295.00	.53

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1037E+03 EXCESS= .0000E+00 OUTFLOW= .1036E+03 BASIN STORAGE= .1753E-01 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT39	MANE	1.83	14557.38	485.61	.78	5.00	14556.94	485.00	.78

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6268E+04 EXCESS= .0000E+00 OUTFLOW= .6232E+04 BASIN STORAGE= .3875E+02 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT39	MANE	1.86	13899.61	486.64	.74	5.00	13899.41	485.00	.75

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5969E+04 EXCESS= .0000E+00 OUTFLOW= .5934E+04 BASIN STORAGE= .3748E+02 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT39	MANE	1.91	12981.92	482.05	.69	5.00	12979.80	485.00	.69

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5534E+04 EXCESS= .0000E+00 OUTFLOW= .5501E+04 BASIN STORAGE= .3584E+02 PERCENT ERR

FOR PLAN = 1		RATIO= .00							
RPT39	MANE	1.97	12009.67	482.51	.63	5.00	12008.09	480.00	.64

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5092E+04 EXCESS= .0000E+00 OUTFLOW= .5060E+04 BASIN STORAGE= .3419E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT39 MANE 2.01 11307.11 481.45 .59 5.00 11306.70 480.00 .59

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4762E+04 EXCESS= .0000E+00 OUTFLOW= .4731E+04 BASIN STORAGE= .3292E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT39 MANE 2.11 10009.85 482.86 .52 5.00 10008.78 485.00 .52

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4182E+04 EXCESS= .0000E+00 OUTFLOW= .4154E+04 BASIN STORAGE= .3049E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT39 MANE 2.24 8530.20 488.04 .44 5.00 8529.41 485.00 .44

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3541E+04 EXCESS= .0000E+00 OUTFLOW= .3515E+04 BASIN STORAGE= .2759E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT39 MANE 2.37 7326.15 483.51 .38 5.00 7325.95 485.00 .38

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3043E+04 EXCESS= .0000E+00 OUTFLOW= .3020E+04 BASIN STORAGE= .2476E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT41 MANE 1.66 14552.93 490.88 .78 5.00 14551.94 490.00 .78

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6320E+04 EXCESS= .0000E+00 OUTFLOW= .6286E+04 BASIN STORAGE= .3679E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT41 MANE 1.70 13895.47 491.57 .74 5.00 13894.21 490.00 .74

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6017E+04 EXCESS= .0000E+00 OUTFLOW= .5984E+04 BASIN STORAGE= .3563E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT41 MANE 1.74 12975.73 487.81 .68 5.00 12975.31 490.00 .68

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5564E+04 EXCESS= .0000E+00 OUTFLOW= .5533E+04 BASIN STORAGE= .3418E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT41 MANE 1.80 12004.91 487.04 .63 5.00 12002.93 490.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5131E+04 EXCESS= .0000E+00 OUTFLOW= .5102E+04 BASIN STORAGE= .3260E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT41 MANE 1.84 11302.50 487.87 .59 5.00 11300.76 485.00 .59

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4802E+04 EXCESS= .0000E+00 OUTFLOW= .4773E+04 BASIN STORAGE= .3145E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT41 MANE 1.93 10005.70 489.07 .52 5.00 10005.32 490.00 .52

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4200E+04 EXCESS= .0000E+00 OUTFLOW= .4173E+04 BASIN STORAGE= .2927E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT41 MANE 2.06 8527.25 492.51 .44 5.00 8526.27 495.00 .44

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3570E+04 EXCESS= .0000E+00 OUTFLOW= .3545E+04 BASIN STORAGE= .2662E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT41 MANE 2.19 7323.83 490.55 .37 5.00 7323.70 490.00 .38

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3057E+04 EXCESS= .0000E+00 OUTFLOW= .3035E+04 BASIN STORAGE= .2411E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT42 MANE 1.36 14551.66 491.64 .78 5.00 14549.53 490.00 .78

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6379E+04 EXCESS= .0000E+00 OUTFLOW= .6352E+04 BASIN STORAGE= .2840E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT42 MANE 1.38 13893.92 491.34 .74 5.00 13891.92 490.00 .74

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6057E+04 EXCESS= .0000E+00 OUTFLOW= .6032E+04 BASIN STORAGE= .2744E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT42 MANE 1.41 12974.48 490.47 .68 5.00 12974.05 490.00 .69

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5597E+04 EXCESS= .0000E+00 OUTFLOW= .5572E+04 BASIN STORAGE= .2627E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT42 MANE 1.45 12002.46 490.89 .63 5.00 12002.28 490.00 .63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5155E+04 EXCESS= .0000E+00 OUTFLOW= .5131E+04 BASIN STORAGE= .2495E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT42 MANE 1.48 11300.88 487.96 .59 5.00 11299.81 490.00 .59

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4835E+04 EXCESS= .0000E+00 OUTFLOW= .4813E+04 BASIN STORAGE= .2396E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT42 MANE 1.55 10004.39 490.53 .52 5.00 10004.02 490.00 .52

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4236E+04 EXCESS= .0000E+00 OUTFLOW= .4215E+04 BASIN STORAGE= .2217E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT42 MANE 1.64 8525.75 495.73 .44 5.00 8525.73 495.00 .44

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3596E+04 EXCESS= .0000E+00 OUTFLOW= .3577E+04 BASIN STORAGE= .2002E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT42 MANE 1.73 7323.54 491.64 .37 5.00 7322.41 490.00 .38

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3075E+04 EXCESS= .0000E+00 OUTFLOW= .3058E+04 BASIN STORAGE= .1800E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT43 MANE 5.00 402.37 280.00 1.03 5.00 402.37 280.00 1.03

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9403E+02 EXCESS= .0000E+00 OUTFLOW= .9402E+02 BASIN STORAGE= .3309E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT43 MANE 5.00 380.62 280.00 .98 5.00 380.62 280.00 .98

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8937E+02 EXCESS= .0000E+00 OUTFLOW= .8937E+02 BASIN STORAGE= .3173E-01 PERCENT ERR



FOR PLAN = 1 RATIO= .00  
 RPT43 MANE 5.00 350.95 280.00 .91 5.00 350.95 280.00 .91

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8301E+02 EXCESS= .0000E+00 OUTFLOW= .8301E+02 BASIN STORAGE= .2982E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT43 MANE 5.00 321.35 280.00 .84 5.00 321.35 280.00 .84

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7665E+02 EXCESS= .0000E+00 OUTFLOW= .7665E+02 BASIN STORAGE= .2786E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT43 MANE 5.00 299.78 280.00 .79 5.00 299.78 280.00 .79

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7200E+02 EXCESS= .0000E+00 OUTFLOW= .7199E+02 BASIN STORAGE= .2638E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT43 MANE 5.00 261.81 280.00 .70 5.00 261.81 280.00 .70

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6373E+02 EXCESS= .0000E+00 OUTFLOW= .6373E+02 BASIN STORAGE= .2365E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT43 MANE 5.00 223.06 280.00 .61 5.00 223.06 280.00 .61

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5515E+02 EXCESS= .0000E+00 OUTFLOW= .5515E+02 BASIN STORAGE= .2066E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT43 MANE 5.00 193.25 280.00 .53 5.00 193.25 280.00 .53

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4838E+02 EXCESS= .0000E+00 OUTFLOW= .4838E+02 BASIN STORAGE= .1816E-01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT46 MANE 1.63 14543.18 510.86 .75 5.00 14542.70 510.00 .75

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6374E+04 EXCESS= .0000E+00 OUTFLOW= .6339E+04 BASIN STORAGE= .3684E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT46 MANE 1.66 13883.90 511.55 .71 5.00 13883.35 510.00 .71

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6060E+04 EXCESS= .0000E+00 OUTFLOW= .6027E+04 BASIN STORAGE= .3569E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT46 MANE 1.70 12961.39 507.91 .66 5.00 12960.28 510.00 .66

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5619E+04 EXCESS= .0000E+00 OUTFLOW= .5588E+04 BASIN STORAGE= .3411E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT46 MANE 1.76 11991.35 507.20 .61 5.00 11989.47 510.00 .61

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5171E+04 EXCESS= .0000E+00 OUTFLOW= .5141E+04 BASIN STORAGE= .3249E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT46 MANE 1.80 11288.83 508.06 .57 5.00 11286.97 510.00 .57

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4831E+04 EXCESS= .0000E+00 OUTFLOW= .4801E+04 BASIN STORAGE= .3130E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RPT46 MANE 1.88 9992.11 511.24 .50 5.00 9991.95 510.00 .50

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4233E+04 EXCESS= .0000E+00 OUTFLOW= .4206E+04 BASIN STORAGE= .2901E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT46 MANE 2.00 8517.17 512.96 .42 5.00 8516.06 515.00 .42

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3605E+04 EXCESS= .0000E+00 OUTFLOW= .3581E+04 BASIN STORAGE= .2631E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT46 MANE 2.11 7317.89 509.26 .36 5.00 7317.87 510.00 .36

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3082E+04 EXCESS= .0000E+00 OUTFLOW= .3060E+04 BASIN STORAGE= .2391E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMCDB MANE 5.00 76.45 345.00 .29 5.00 76.45 345.00 .29

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4407E+02 EXCESS= .0000E+00 OUTFLOW= .4354E+02 BASIN STORAGE= .5432E+00 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMCDB MANE 5.00 74.54 345.00 .28 5.00 74.54 345.00 .28

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4287E+02 EXCESS= .0000E+00 OUTFLOW= .4234E+02 BASIN STORAGE= .5323E+00 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMCDB MANE 5.00 71.92 345.00 .27 5.00 71.92 345.00 .27

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4119E+02 EXCESS= .0000E+00 OUTFLOW= .4068E+02 BASIN STORAGE= .5172E+00 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMCDB MANE 5.00 69.27 345.00 .26 5.00 69.27 345.00 .26

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3945E+02 EXCESS= .0000E+00 OUTFLOW= .3895E+02 BASIN STORAGE= .4999E+00 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMCDB MANE 5.00 67.30 345.00 .25 5.00 67.30 345.00 .25

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3806E+02 EXCESS= .0000E+00 OUTFLOW= .3758E+02 BASIN STORAGE= .4847E+00 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMCDB MANE 5.00 63.41 340.00 .23 5.00 63.41 340.00 .23

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3520E+02 EXCESS= .0000E+00 OUTFLOW= .3475E+02 BASIN STORAGE= .4577E+00 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMCDB MANE 5.00 58.54 340.00 .21 5.00 58.54 340.00 .21

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3184E+02 EXCESS= .0000E+00 OUTFLOW= .3142E+02 BASIN STORAGE= .4220E+00 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RMCDB MANE 5.00 54.43 340.00 .19 5.00 54.43 340.00 .19

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2875E+02 EXCESS= .0000E+00 OUTFLOW= .2837E+02 BASIN STORAGE= .3881E+00 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT47 MANE 5.00 434.09 230.00 .59 5.00 434.09 230.00 .59

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9135E+02 EXCESS= .0000E+00 OUTFLOW= .8874E+02 BASIN STORAGE= .3006E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT47 MANE 5.00 414.04 230.00 .57 5.00 414.04 230.00 .57

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8806E+02 EXCESS= .0000E+00 OUTFLOW= .8551E+02 BASIN STORAGE= .2954E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT47 MANE 5.00 386.17 230.00 .54 5.00 386.17 230.00 .54

CONTINUITY SUMMARY (AC-FT) - INFLOW= .8347E+02 EXCESS= .0000E+00 OUTFLOW= .8099E+02 BASIN STORAGE= .2883E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT47 MANE 5.00 357.73 230.00 .51 5.00 357.73 230.00 .51

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7877E+02 EXCESS= .0000E+00 OUTFLOW= .7637E+02 BASIN STORAGE= .2804E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT47 MANE 5.00 336.61 230.00 .49 5.00 336.61 230.00 .49

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7520E+02 EXCESS= .0000E+00 OUTFLOW= .7287E+02 BASIN STORAGE= .2731E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT47 MANE 5.00 301.52 235.00 .44 5.00 301.52 235.00 .44

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6835E+02 EXCESS= .0000E+00 OUTFLOW= .6603E+02 BASIN STORAGE= .2601E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT47 MANE 5.00 265.91 235.00 .39 5.00 265.91 235.00 .39

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6062E+02 EXCESS= .0000E+00 OUTFLOW= .5847E+02 BASIN STORAGE= .2433E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT47 MANE 5.00 235.53 235.00 .35 5.00 235.53 235.00 .35

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5390E+02 EXCESS= .0000E+00 OUTFLOW= .5191E+02 BASIN STORAGE= .2266E+01 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RB20 MANE 4.83 227.91 250.97 .99 5.00 227.12 250.00 .99

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4181E+02 EXCESS= .0000E+00 OUTFLOW= .4181E+02 BASIN STORAGE= .1945E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RB20 MANE 4.91 214.51 250.52 .94 5.00 214.04 250.00 .94

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3942E+02 EXCESS= .0000E+00 OUTFLOW= .3942E+02 BASIN STORAGE= .1864E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RB20 MANE 5.00 196.29 255.00 .86 5.00 196.29 255.00 .86

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3613E+02 EXCESS= .0000E+00 OUTFLOW= .3613E+02 BASIN STORAGE= .1700E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RB20 MANE 5.00 177.97 255.00 .78 5.00 177.97 255.00 .78

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3284E+02 EXCESS= .0000E+00 OUTFLOW= .3284E+02 BASIN STORAGE= .1613E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RB20 MANE 5.00 164.47 255.00 .72 5.00 164.47 255.00 .72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3042E+02 EXCESS= .0000E+00 OUTFLOW= .3043E+02 BASIN STORAGE= .1547E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RB20 MANE 5.00 140.29 255.00 .62 5.00 140.29 255.00 .62

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2614E+02 EXCESS= .0000E+00 OUTFLOW= .2614E+02 BASIN STORAGE= .1965E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RB20 MANE 5.00 114.92 255.00 .51 5.00 114.92 255.00 .51

CONTINUITY SUMMARY (AC-FT) - INFLOW= .2168E+02 EXCESS= .0000E+00 OUTFLOW= .2168E+02 BASIN STORAGE= .1771E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RB20 MANE 5.00 94.73 255.00 .43 5.00 94.73 255.00 .43

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1817E+02 EXCESS= .0000E+00 OUTFLOW= .1817E+02 BASIN STORAGE= .1538E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RD12 MANE 5.00 38.88 265.00 .77 5.00 38.88 265.00 .77

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7236E+01 EXCESS= .0000E+00 OUTFLOW= .7240E+01 BASIN STORAGE= .6424E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RD12 MANE 5.00 36.21 265.00 .72 5.00 36.21 265.00 .72

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6774E+01 EXCESS= .0000E+00 OUTFLOW= .6779E+01 BASIN STORAGE= .6181E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RD12 MANE 5.00 32.61 270.00 .66 5.00 32.61 270.00 .66

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6147E+01 EXCESS= .0000E+00 OUTFLOW= .6151E+01 BASIN STORAGE= .7172E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RD12 MANE 5.00 29.07 270.00 .59 5.00 29.07 270.00 .59

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5524E+01 EXCESS= .0000E+00 OUTFLOW= .5528E+01 BASIN STORAGE= .6750E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RD12 MANE 5.00 26.46 270.00 .54 5.00 26.46 270.00 .54

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5071E+01 EXCESS= .0000E+00 OUTFLOW= .5075E+01 BASIN STORAGE= .6460E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RD12 MANE 5.00 21.81 270.00 .46 5.00 21.81 270.00 .46

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4275E+01 EXCESS= .0000E+00 OUTFLOW= .4279E+01 BASIN STORAGE= .7033E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
 RD12 MANE 5.00 17.16 275.00 .37 5.00 17.16 275.00 .37

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3464E+01 EXCESS= .0000E+00 OUTFLOW= .3467E+01 BASIN STORAGE= .6231E-02 PERCENT ERR

FOR PLAN = 1 RATIO= .00

RD12	MANE	5.00	13.52	280.00	.30	5.00	13.52	280.00	.30
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CONTINUITY SUMMARY (AC-FT) - INFLOW= .2839E+01 EXCESS= .0000E+00 OUTFLOW= .2842E+01 BASIN STORAGE= .6715E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT49	MANE	5.00	70.77	300.00	.75	5.00	70.77	300.00	.75

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1332E+02 EXCESS= .0000E+00 OUTFLOW= .1332E+02 BASIN STORAGE= .8174E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT49	MANE	5.00	66.72	300.00	.70	5.00	66.72	300.00	.70

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1246E+02 EXCESS= .0000E+00 OUTFLOW= .1246E+02 BASIN STORAGE= .7943E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT49	MANE	5.00	61.12	300.00	.63	5.00	61.12	300.00	.63

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1128E+02 EXCESS= .0000E+00 OUTFLOW= .1128E+02 BASIN STORAGE= .8720E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT49	MANE	5.00	55.44	300.00	.57	5.00	55.44	300.00	.57

CONTINUITY SUMMARY (AC-FT) - INFLOW= .1011E+02 EXCESS= .0000E+00 OUTFLOW= .1011E+02 BASIN STORAGE= .8331E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT49	MANE	5.00	51.24	300.00	.52	5.00	51.24	300.00	.52

CONTINUITY SUMMARY (AC-FT) - INFLOW= .9264E+01 EXCESS= .0000E+00 OUTFLOW= .9267E+01 BASIN STORAGE= .8035E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT49	MANE	5.00	43.74	300.00	.44	5.00	43.74	300.00	.44

CONTINUITY SUMMARY (AC-FT) - INFLOW= .7777E+01 EXCESS= .0000E+00 OUTFLOW= .7778E+01 BASIN STORAGE= .9167E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT49	MANE	5.00	35.76	300.00	.35	5.00	35.76	300.00	.35

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6264E+01 EXCESS= .0000E+00 OUTFLOW= .6266E+01 BASIN STORAGE= .8386E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT49	MANE	4.75	29.37	304.00	.29	5.00	29.12	305.00	.29

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5103E+01 EXCESS= .0000E+00 OUTFLOW= .5103E+01 BASIN STORAGE= .9324E-02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT50	MANE	4.46	14612.59	517.77	.73	5.00	14609.97	515.00	.73

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6468E+04 EXCESS= .0000E+00 OUTFLOW= .6373E+04 BASIN STORAGE= .1025E+03 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT50	MANE	4.54	13951.06	517.66	.69	5.00	13948.68	515.00	.69

CONTINUITY SUMMARY (AC-FT) - INFLOW= .6162E+04 EXCESS= .0000E+00 OUTFLOW= .6070E+04 BASIN STORAGE= .9918E+02 PERCENT ERR

FOR PLAN = 1	RATIO=	.00							
RPT50	MANE	4.66	13025.36	517.05	.63	5.00	13024.07	515.00	.64

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5657E+04 EXCESS= .0000E+00 OUTFLOW= .5569E+04 BASIN STORAGE= .9527E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT50 MANE 4.79 12051.94 517.77 .59 5.00 12051.65 515.00 .59

CONTINUITY SUMMARY (AC-FT) - INFLOW= .5246E+04 EXCESS= .0000E+00 OUTFLOW= .5162E+04 BASIN STORAGE= .9041E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT50 MANE 4.90 11350.34 514.75 .55 5.00 11350.11 515.00 .55

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4892E+04 EXCESS= .0000E+00 OUTFLOW= .4811E+04 BASIN STORAGE= .8716E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT50 MANE 5.00 10049.07 520.00 .48 5.00 10049.07 520.00 .48

CONTINUITY SUMMARY (AC-FT) - INFLOW= .4310E+04 EXCESS= .0000E+00 OUTFLOW= .4236E+04 BASIN STORAGE= .8054E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT50 MANE 5.00 8569.97 520.00 .41 5.00 8569.97 520.00 .41

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3660E+04 EXCESS= .0000E+00 OUTFLOW= .3593E+04 BASIN STORAGE= .7310E+02 PERCENT ERR

FOR PLAN = 1 RATIO= .00  
RPT50 MANE 5.00 7367.02 520.00 .35 5.00 7367.02 520.00 .35

CONTINUITY SUMMARY (AC-FT) - INFLOW= .3136E+04 EXCESS= .0000E+00 OUTFLOW= .3075E+04 BASIN STORAGE= .6642E+02 PERCENT ERR



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## HEC-1 INPUT

PAGE 1

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\*\*\* NORMAL END OF HEC-1 \*\*\*  
NORMAL END OF HEC-1



PB89-218424

# DESIGN OF RIPRAP REVETMENT

SUTRON CORPORATION  
HERNDON, VA

MAR 89

U.S. DEPARTMENT OF COMMERCE  
National Technical Information Service

**NTIS**

# South Box Outlet

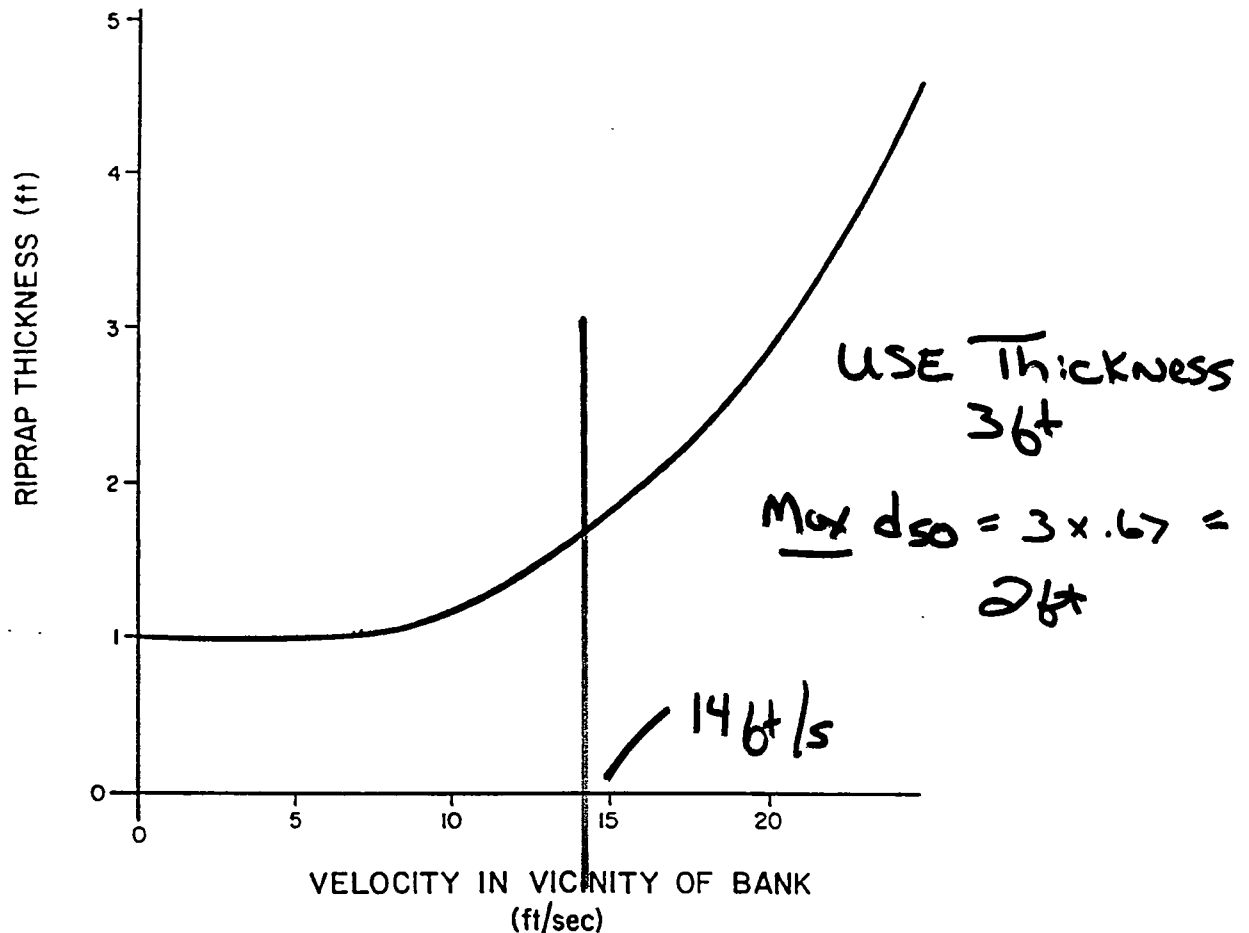


Figure 57. Required blanket thickness as a function of flow velocity.

**Rock Grading:** Table 6 provides guidelines for rock gradation in grouted riprap installations. Six size classes are listed.

**Rock Quality:** Rock used in grouted rock slope-protection is usually the same as that used in ordinary rock slope-protection. However, the specifications for specific gravity and hardness may be lowered if necessary as the rocks are protected by the surrounding grout.

In addition, the rock used in grouted riprap installations should be free of fines in order that penetration of grout may be achieved.

**Grout Quality and Characteristics:** Grout should consist of good strength concrete using a maximum aggregate size of 3/4 in and a slump of 3 to 4 in (7.6 to 10.2 cm). Sand mixes may be used where roughness of the grout surface is unnecessary, provided sufficient cement is added to give good strength and workability.

The volume of grout required will be that necessary to provide penetration to the depths shown in table 6.

The finished grout should leave face stones exposed for one-fourth to one-third their depth and the surface of the grout should expose a matrix of coarse aggregate.

# North Box Outlet

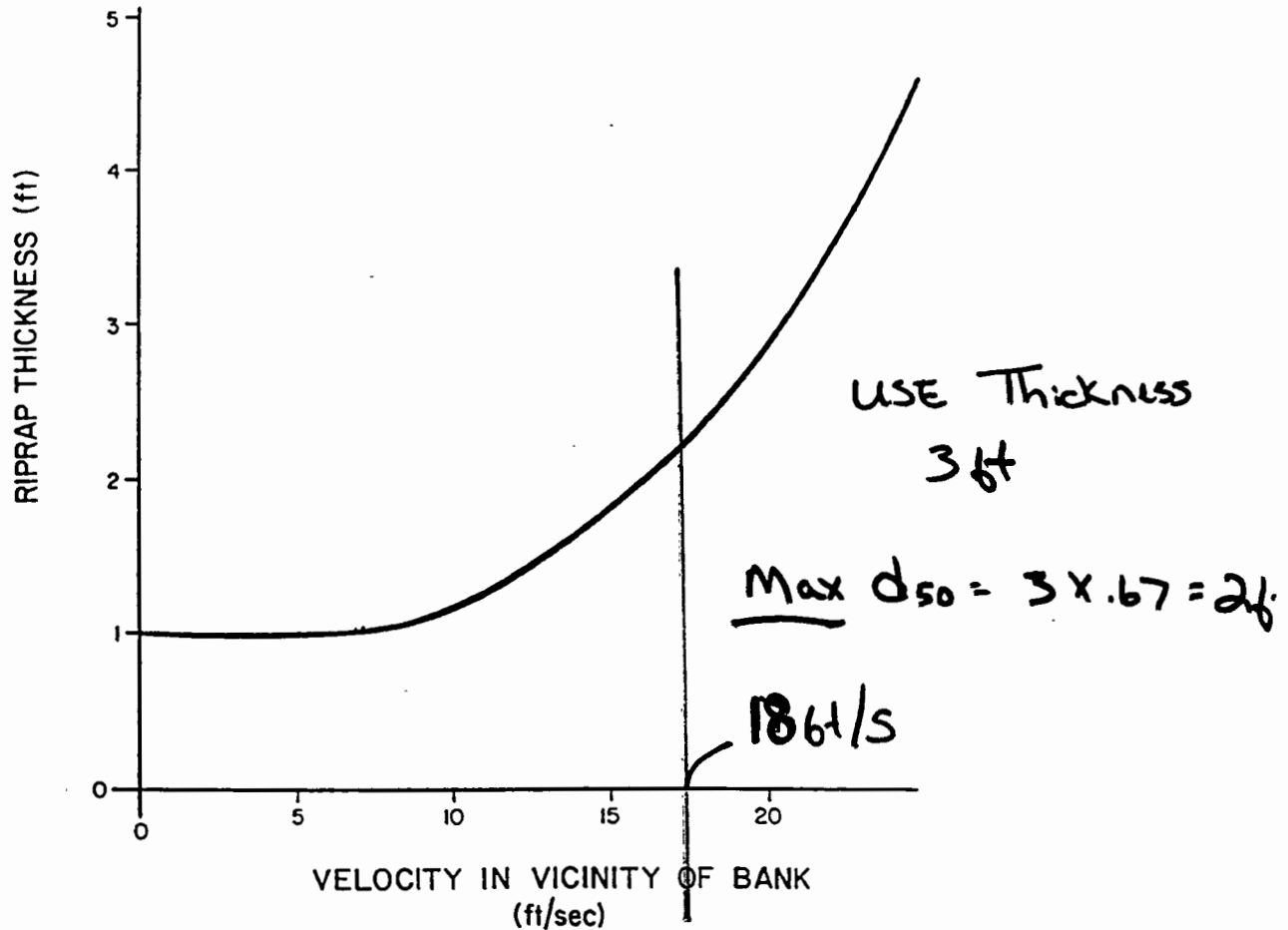


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**Rock Quality:** Rock used in grouted rock slope-protection is usually the same as that used in ordinary rock slope-protection. However, the specifications for specific gravity and hardness may be lowered if necessary as the rocks are protected by the surrounding grout.

In addition, the rock used in grouted riprap installations should be free of fines in order that penetration of grout may be achieved.

**Grout Quality and Characteristics:** Grout should consist of good strength concrete using a maximum aggregate size of 3/4 in and a slump of 3 to 4 in (7.6 to 10.2 cm). Sand mixes may be used where roughness of the grout surface is unnecessary, provided sufficient cement is added to give good strength and workability.

The volume of grout required will be that necessary to provide penetration to the depths shown in table 6.

The finished grout should leave face stones exposed for one-fourth to one-third their depth and the surface of the grout should expose a matrix of coarse aggregate.

### 6.3 GROUTED ROCK

Grouted rock revetment consists of rock slope-protection having voids filled with concrete grout to form a monolithic armor. See section 2.5 for additional descriptive information and general performance characteristics for grouted rock. Sample specifications for components of grouted rock revetments are provided in appendix A.

#### 6.3.1 Design Guidelines for Grouted Rock

Components of grouted rock riprap design include layout of a general scheme or concept, bank preparation, bank slope, rock size and blanket thickness, rock grading, rock quality, grout quality, edge treatment, filter design, and pressure relief.

**General:** Grouted riprap designs are rigid monolithic bank protection schemes. When complete, they form a continuous surface. A typical grouted riprap section is shown in figure 56.

Grouted riprap should extend from below the anticipated channel bed scour depth to the design high water level plus additional height for freeboard (see section 3.6.2). The longitudinal extent of protection should be as described in section 3.6.1.

During the design phase for a grouted riprap revetment, special attention needs to be paid to edge treatment, foundation design, and mechanisms for hydrostatic pressure relief. Each of these items is discussed below.

**Bank and Foundation Preparation:** The bank should be prepared by first clearing all trees and debris from the bank, and grading the bank surface to the desired slope. In general, the graded surface should not deviate from the specified slope line by more than six in (15.2 cm). However, local depressions larger than this can be accommodated since initial placement of filter material and/or rock for the revetment will fill these depressions.

Since grouted riprap is rigid but not extremely strong, support by the embankment must be maintained. To form a firm foundation, it is recommended that the bank surface be tamped or lightly compacted. Care must be taken during bank compaction to maintain a soil permeability similar to that of the natural, undisturbed bank material. The foundation for the grouted riprap revetment should have a bearing capacity sufficient to support either the dry weight of the revetment alone, or the submerged weight of the revetment plus the weight of the water in the wedge above the revetment for design conditions, whichever is greater.

Any large boulders or debris found buried near the edges of the revetment should be removed.

**Bank Slope:** Bank slopes for grouted riprap revetments should not exceed 1.5:1.

**Rock Size and Blanket Thickness:** Blanket thickness and rock size requirements for grouted riprap installations are interrelated. Figure 57 illustrates a relationship between the design velocity and the required riprap blanket thickness for grouted riprap designs. The median rock size in the revetment should not exceed 0.67 times the blanket thickness. The largest rock used in the revetment should not exceed the blanket thickness.

UNITED STATES DEPARTMENT OF THE INTERIOR

BUREAU OF RECLAMATION

# DESIGN OF SMALL DAMS

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to a magnitude where cavitation conditions can develop.

The negative pressure on the crest may be resolved into a system of forces acting both upward and downstream. These forces should be considered in analyzing the structural stability of the crest structure.

An approximate force diagram of the subatmospheric pressures when the design head used to determine the crest shape is 75 percent of the maximum head is shown on figure 9-29. These data are based on average results of tests made on ideally shaped weirs with negligible approach velocities. Pressures for intermediate head ratios can be assumed to vary linearly, considering that no subatmospheric pressure prevails when  $H_d/H_c = 1$ .

**9.15. Gate-Controlled Ogee Crests.**—Releases for partial gate openings for gated crests occur as orifice flow. With full head on a gate that is opened a small amount, a free discharging trajectory will follow the path of a jet issuing from an orifice. For a vertical orifice the path of the jet can be expressed by the parabolic equation:

$$-y = \frac{x^2}{4H} \quad (5)$$

where  $H$  is the head on the center of the opening. For an orifice inclined an angle  $\theta$  from the vertical, the equation is:

$$-y = x \tan \theta + \frac{x^2}{4H \cos^2 \theta} \quad (6)$$

If subatmospheric pressures are to be avoided along the crest contact, the shape of the ogee downstream from the gate sill must conform to the trajectory profile.

Gates operated with small openings under high heads produce negative pressures along the crest in the region immediately below the gate if the ogee profile drops below the trajectory profile. Tests showed the subatmospheric pressures would be equal to about one-tenth of the design head when the gate is operated at small openings and the ogee is shaped to the ideal nappe profile, equation (2), for maximum head  $H_c$ . The force diagram for this condition is shown on figure 9-30.

The adoption of a trajectory profile rather than a nappe profile downstream from the gate sill will result in a wider ogee, and reduced discharge effi-

ciency for full gate opening. Where the discharge efficiency is unimportant and where a wider ogee shape is needed for structural stability, the trajectory profile may be adopted to avoid subatmospheric pressure zones along the crest. Where the ogee is shaped to the ideal nappe profile for maximum head, the subatmospheric pressure area can be minimized by placing the gate sill downstream from the crest of the ogee. This will provide an orifice that is inclined downstream for small gate openings and will result in a steeper trajectory closer to the nappe-shaped profile.

**9.16. Discharge Over Gate-Controlled Ogee Crests.**—The discharge for a gated ogee crest at partial gate openings will be similar to flow through an orifice and may be computed by the equation:

$$Q = CDL \sqrt{2gH} \quad (7)$$

where:

$H$  = head to the center of the gate opening (including the velocity head of approach),

$D$  = shortest distance from the gate lip to the crest curve, and

$L$  = crest width.

The coefficient,  $C$ , is primarily dependent upon the characteristics of the flow lines approaching and leaving the orifice. In turn, these flow lines are dependent on the shape of the crest and the type of gate. Figure 9-31, which shows coefficients of discharge for orifice flows for different  $\theta$  angles, can be used for leaf gates or radial gates located at the crest or downstream of the crest. The  $\theta$  angle for a particular opening is that angle formed by the tangent to the gate lip and the tangent to the crest curve at the nearest point of the crest curve for radial gates. This angle is affected by the gate radius and the location of the trunnion pin. For additional information and geometric computations see [20].

**9.17. Side Channel Spillways.**—(a) *General.*—The theory of flow in a side channel spillway [21] is based principally on the law of conservation of linear momentum, assuming that the only forces producing motion in the channel result from the fall in the water surface in the direction of the axis. This premise assumes that the entire energy of the flow over the crest is dissipated through its intermingling with the channel flow and is therefore of

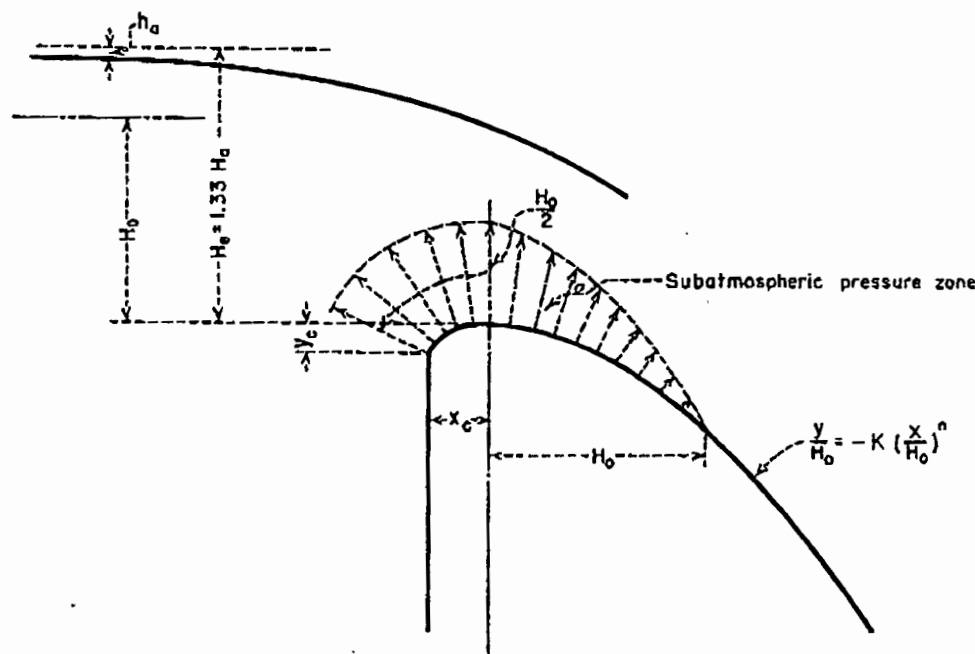
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## SPILLWAYS

377

Figure 9-29.—Subatmospheric crest pressures for  $H_c/H_0 = 0.75$ . 288-D-2415.

no assistance in moving the water along the channel. Axial velocity is produced only after the incoming water particles join the channel stream.

For any short reach of the side channel, the momentum at the beginning of the reach plus any increase in momentum from external forces must equal the momentum at the end of the reach. If a short reach,  $\Delta x$  in length, is considered and the velocity and discharge at the upstream section are  $v$  and  $Q$ , respectively, then the velocity and discharge at the downstream section will be  $v + \Delta v$  and  $Q + q(\Delta x)$ , where  $q$  is the inflow per foot of length of weir crest. Therefore, the momentum<sup>2</sup> at the upstream section will be:

$$M_u = \frac{Qv}{g} \quad (8)$$

And the momentum at the downstream section will be:

$$M_d = \frac{[Q + q(\Delta x)](v + \Delta v)}{g} \quad (9)$$

Subtracting equation (8) from equation (9):

$$\Delta M = \frac{Q(\Delta v)}{g} + \frac{q(\Delta x)}{g}(v + \Delta v) \quad (10)$$

Dividing by  $\Delta x$ :

$$\frac{\Delta M}{\Delta x} = \frac{Q(\Delta v)}{g(\Delta x)} + \frac{q}{g}(v + \Delta v) \quad (11)$$

Since the rate of change of momentum with respect to time is  $v$  times the rate of change with respect to  $x$ , and considering the average velocity =  $v + (\Delta v)/2$ , equation (11) can be written:

$$\begin{aligned} \frac{\Delta M}{\Delta t} &= \frac{Q(\Delta v)}{g(\Delta x)} \left[ v + \frac{1}{2}(\Delta v) \right] \\ &+ \frac{q}{g}(v + \Delta v) \left[ v + \frac{1}{2}(\Delta v) \right] \end{aligned} \quad (12)$$

Since  $\Delta M/\Delta t$  is the accelerating force, which is equal to the slope of the water surface,  $\Delta y/\Delta x$ , times the average discharge, equation (12) becomes:

<sup>2</sup>The weight of 1 ft<sup>3</sup> of water is taken as a unit force to eliminate the necessity of multiplying all forces and momenta by 62.4 to convert them into pounds.

$$\frac{\Delta y}{\Delta x} \left[ Q + \frac{1}{2} (\Delta Q) \right] = \frac{Q(\Delta v)}{g(\Delta x)} \left[ v + \frac{1}{2} (\Delta v) \right] + \frac{q}{g} (v + \Delta v) \left[ v + \frac{1}{2} (\Delta v) \right] \quad (13)$$

from which the change in water surface elevation is:

$$\Delta y = \frac{Q \left[ v + \frac{1}{2} (\Delta v) \right]}{g \left[ Q + \frac{1}{2} (\Delta Q) \right]} \left[ \Delta v + \frac{q(\Delta x)}{Q} (v + \Delta v) \right] \quad (14)$$

If  $Q_1$  and  $v_1$  are values at the beginning of the reach, and  $Q_2$  and  $v_2$  are the values at the end of the reach, the equation can be written:

$$\Delta y = \frac{Q_1}{g} \frac{(v_1 + v_2)}{(Q_1 + Q_2)} \left[ (v_2 - v_1) + \frac{v_2(Q_2 - Q_1)}{Q_1} \right] \quad (15)$$

Similarly, the derivation can be developed so that:

$$\Delta y = \frac{Q_2}{g} \frac{(v_1 + v_2)}{(Q_1 + Q_2)} \left[ (v_2 - v_1) + \frac{v_1(Q_2 - Q_1)}{Q_2} \right] \quad (16)$$

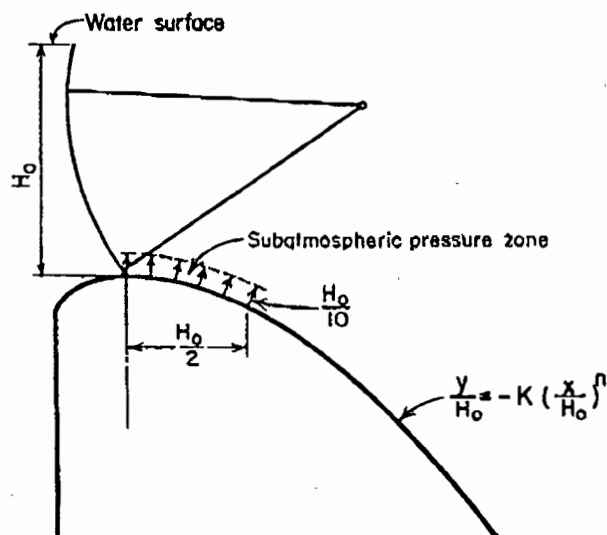


Figure 9-30.—Subatmospheric crest pressures for under-shot gate flow. 288-D-2416.

By use of equation (15) or (16), the water surface profile can be determined for any particular side channel by assuming successive short reaches of channel once a starting point is found. The solution of equation (15) or (16) is obtained by a trial-and-error procedure. For a reach of length  $\Delta x$  in a specific location,  $Q_1$  and  $Q_2$  will be known. If the depth at one end of the reach has been established, a trial depth at the other end of the reach can be found that will satisfy the indicated and computed values of  $\Delta y$ .

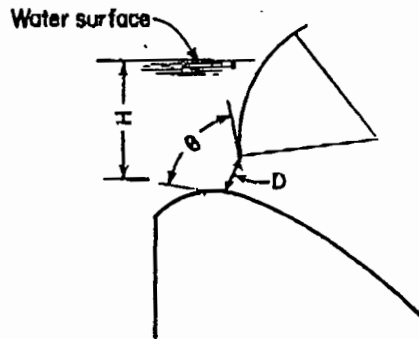
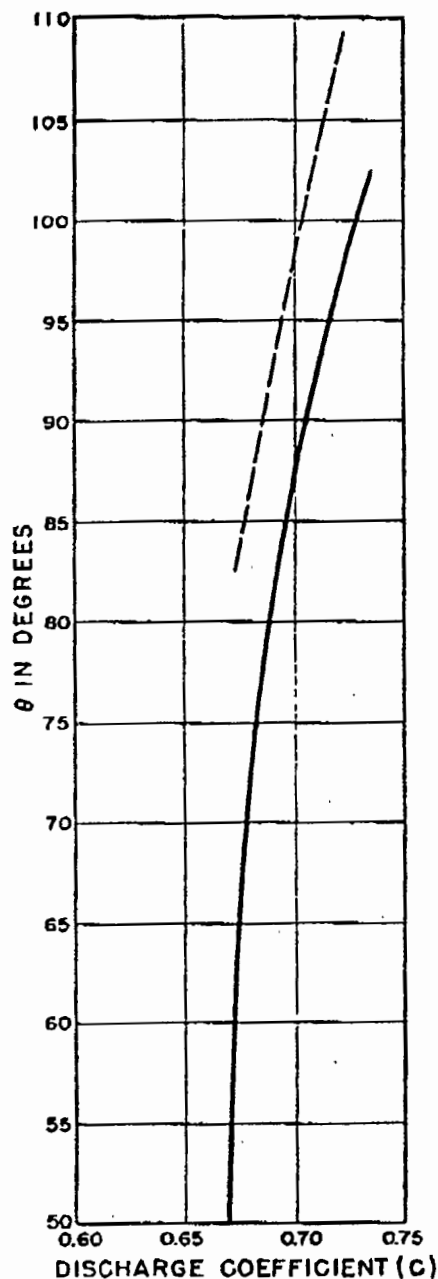
As in other water surface profile determinations, the depth of flow and the hydraulic characteristics of the flow will be affected by backwater influences from some control point or by critical conditions along the reach of the channel under consideration. The selection of a control for starting the water surface profile computations is treated in the subsequent discussion.

When the bottom of the side channel trough is selected so that its depth below the hydraulic gradient is greater than the minimum specific energy depth, flow will be either at the subcritical or supercritical stage, depending either on the relation of the bottom profile to critical slope or on the influences of a downstream control section. If the slope of the bottom is greater than critical and a control section is not established below the side channel trough, supercritical flow will prevail throughout the length of the channel. For this stage, velocities will be high and water depths will be shallow, resulting in a relatively high fall from the reservoir water level to the water surface in the trough. This flow condition is illustrated by profile B' on figure 9-32. Conversely, if a control section is established downstream from the side channel trough to increase the upstream depths, the channel can be made to flow at the subcritical stage. Velocities at this stage will be less than critical, and the greater depths will result in a smaller drop from the reservoir water surface to the side channel water surface profile. The condition of flow for subcritical depths is illustrated on figure 9-32 by water surface profile A'.

The effect of the fall distance from the reservoir to the channel water surface for each type of flow is depicted on figure 9-32(B). It can be seen that for the subcritical stage, the incoming flow will not develop high transverse velocities because of the low drop before it meets the channel flow, thus effecting a good diffusion with the water bulk in the trough.

## SPILLWAYS

379



## EQUATION FOR DISCHARGE

$$Q = CDL\sqrt{2gH}$$

D = Net gate opening

L = Crest width

H = Head to center of gate opening

For C, use dashed line when gate seats on crest and solid line when gate seats below crest.

## REFERENCE

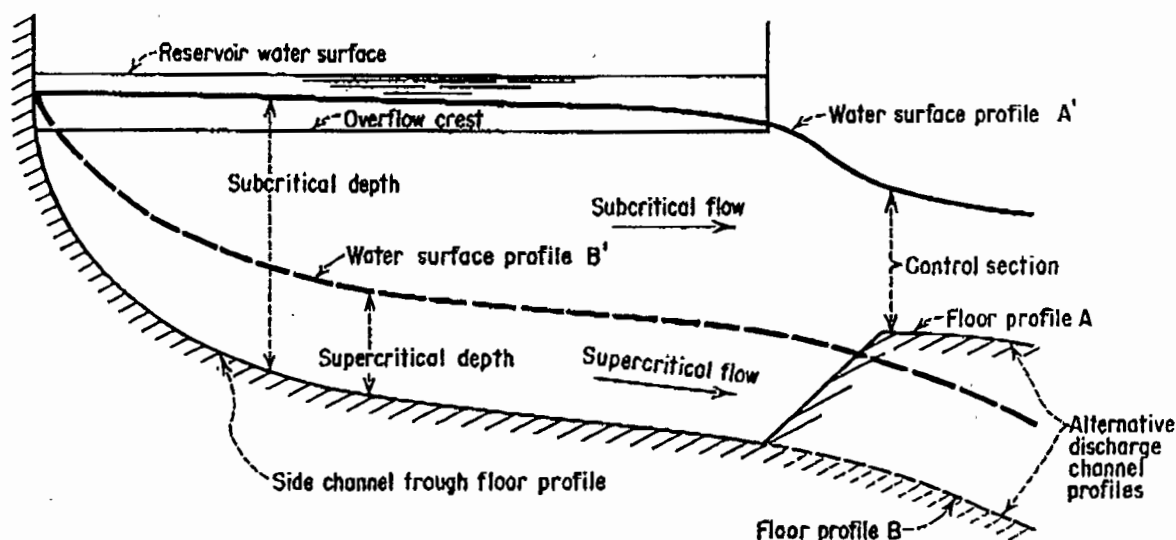
U.S. Army  
Corps Of Engineers  
Hydraulic Design Criteria  
Design Chart 311-1

Figure 9-31.—Discharge coefficient for flow under gates. 103-D-1875.

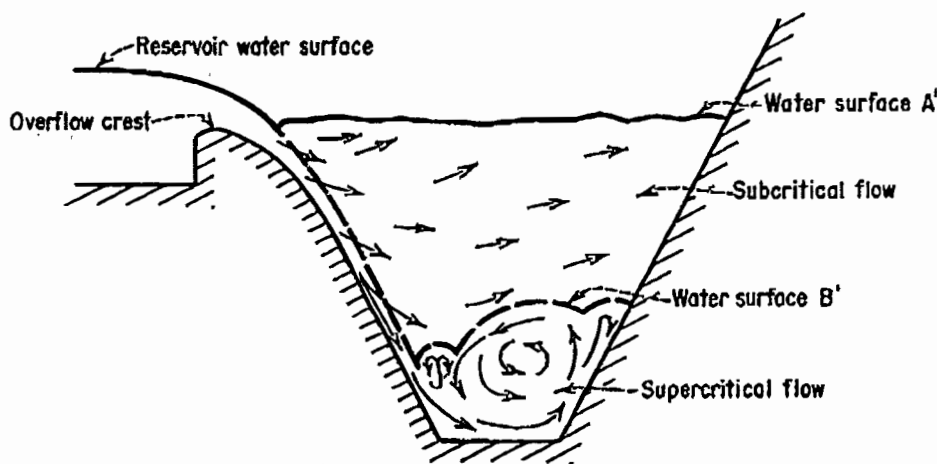
Because both the incoming velocities and the channel velocities will be relatively slow, a fairly complete intermingling of the flows will occur, thereby producing a comparatively smooth flow in the side channel. Where the channel flow is at the supercritical stage, the channel velocities will be high,

and the intermixing of the high-energy transverse flow with the channel stream will be rough and turbulent. The transverse flows will tend to sweep the channel flow to the far side of the channel, producing violent wave action with attendant vibrations. Therefore, it is evident that flows should be

## DESIGN OF SMALL DAMS



(A) SIDE CHANNEL PROFILE



(B) SIDE CHANNEL CROSS SECTION

Figure 9-32.—Side channel flow characteristics. 288-D-2418.

maintained at subcritical stage for good hydraulic performance. This can be achieved by establishing a control section downstream from the side channel trough.

The cross-sectional shape of the side channel trough will be influenced by the overflow crest on the one side and by the bank conditions on the opposite side. Because of turbulence and vibrations inherent in side channel flow, a side channel design

is ordinarily not considered except where a competent foundation such as rock exists. The channel sides will, therefore, usually be a concrete lining placed on a slope and anchored directly to the rock. A trapezoidal cross section is the one most often used for a side channel trough. The width of such a channel in relation to the depth should be considered. If the width to depth ratio is large, the depth of flow in the channel will be shallow, similar

## SPILLWAYS

381

to that depicted by the cross section *abfg* on figure 9-33. It is evident that for this condition a poor diffusion of the incoming flow with the channel flow will result. A cross section with a minimum width to depth ratio will provide the best hydraulic performance; this indicates that a cross section approaching *adj* (on fig. 9-33) would be the ideal choice both from the standpoint of hydraulics and economy. However, some bottom width is needed to avoid construction difficulties caused by confined working space. Furthermore, the stability of both the structure and the hillside, which might be jeopardized by an extremely deep cut in the abutment, must also be considered. Therefore, the minimum bottom width selected must be commensurate with both the practical and structural aspects of the problem.

A control section downstream from the side channel trough is achieved by constricting the channel sides or elevating the channel bottom to produce a point of critical flow. Flows upstream from the control will be at the subcritical stage and will provide a maximum of depth in the side channel trough. The side channel bottom and control dimensions are then selected so that flow in the trough opposite the crest will be at the greatest depth possible without submerging the flow over the crest. Flow in the discharge channel downstream from the control will be the same as that in an ordinary channel or chute spillway.

(b) *Design Example.*—A design example illustrates the procedures for determining the hydraulic design of a side channel spillway control structure. The problem is to design a side channel spillway 100 feet long (station 0+00 to station 1+00) to discharge a maximum of 2,000 ft<sup>3</sup>/s. The spillway crest is at elevation 1000.0 feet, and the discharge per foot of length  $q = 2,000/100 = 20$  ft<sup>3</sup>/s. Assume the crest coefficient  $C = 3.6$ ,  $H_c = (q/C)^{2/3} = 3.1$  feet.

For the side channel trough, assume a trapezoidal section with 1/2:1 side slopes and a bottom width of 10 feet, whose rise in bottom profile is 1.0 foot in the 100 feet of channel length. (The slope of the channel profile is arbitrary; however, a relatively flat slope will provide greater depths and lower velocities and, consequently, will ensure better intermingling of flows at the upstream end of the channel and avoid the possibility of accelerating or supercritical flows occurring in the channel for smaller discharges.) Furthermore, assume that a control section is placed downstream from the side

channel trough with its bottom at the same elevation as the bottom of the side channel floor at the downstream end. Assume that a transition is made from the 1/2:1 slopes of the trough section to a rectangular section at the control. Arbitrarily assume a datum for the control section bottom at elevation 100.0.

Therefore, the critical depth for flow at the control is  $d_c = (q_1^2/g)^{1/3}$ .

For this example:

$$q_1 = \frac{2,000}{10} = 200 \text{ ft}^3/\text{s per foot of width}$$

$$d_c = \left( \frac{200^2}{32.2} \right)^{1/3} = 10.75 \text{ feet}$$

$$v_c = \frac{q_1}{d_c} = \frac{200}{10.75} = 18.6 \text{ ft/s}$$

$$h_{v_c} = \frac{v_c^2}{2g} = \frac{18.6^2}{64.4} = 5.37 \text{ feet}$$

Assume a transition loss from the end of the side channel trough to the control section (to provide for losses caused by contraction, by diffusion of the flows not affected in the side channel proper, and by friction losses) equal to 0.2 of the difference in velocity heads between the ends of the transition. The flow characteristics at the downstream end of the side channel can be obtained from Bernoulli's equation (app. B). For figure 9-34, Bernoulli's equation may be written as follows:

$$d_{(1+00)} + h_{v_{(1+00)}} = d_c + h_{v_c} + 0.2(h_{v_c} - h_{v_{(1+00)}})$$

This expression must be solved by trial and error. First, assume a value of  $d_{(1+00)}$ , and solve for  $h_{v_{(1+00)}}$ . If the use of these values does not result in a balanced equation, a new value must be assumed for  $d_{(1+00)}$  and the process repeated. A value of 16.34 feet for  $d_{(1+00)}$  was found to satisfy the equation as follows:

For  $d_{(1+00)} = 16.34$ , the area of flow at station 1+00 in the trapezoidal cross section with 10-foot bottom width and 1/2:1 side slopes = 297 ft<sup>2</sup>.

$$v_{(1+00)} = \frac{2,000}{297} = 6.73 \text{ ft/s}$$

$$h_{v_{(1+00)}} = \frac{6.73^2}{64.4} = 0.70 \text{ foot}$$

$$0.2(h_{v_c} - h_{v_{(1+00)}}) = 0.2(5.37 - 0.70) = 0.93 \text{ foot}$$



## DESIGN OF SMALL DAMS

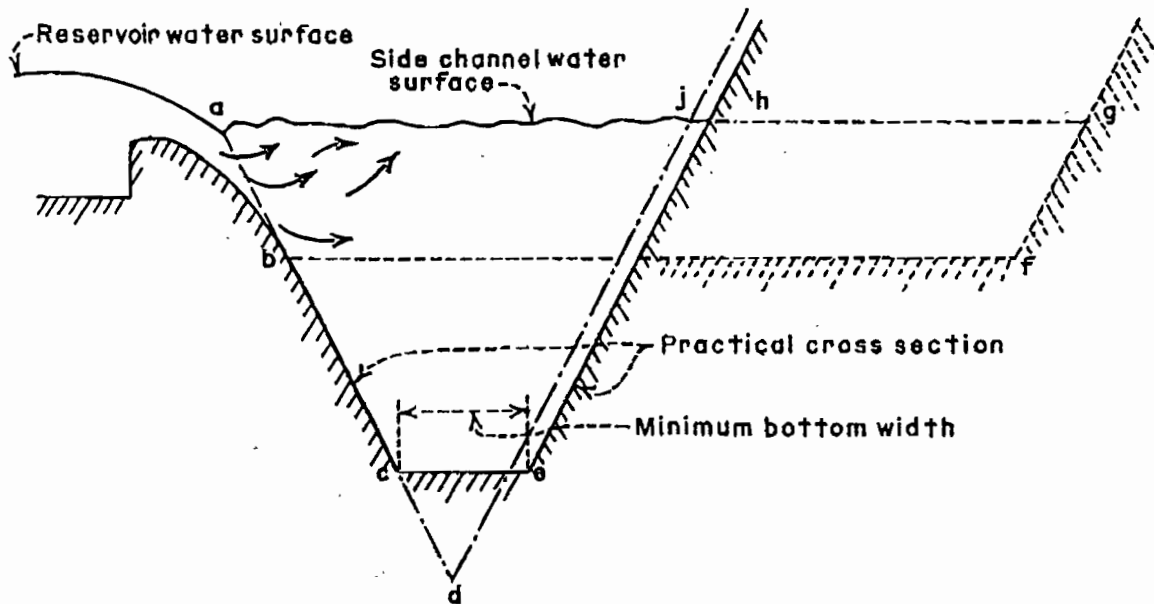


Figure 9-33.—Comparison of side channel cross sections. 288-D-2419.

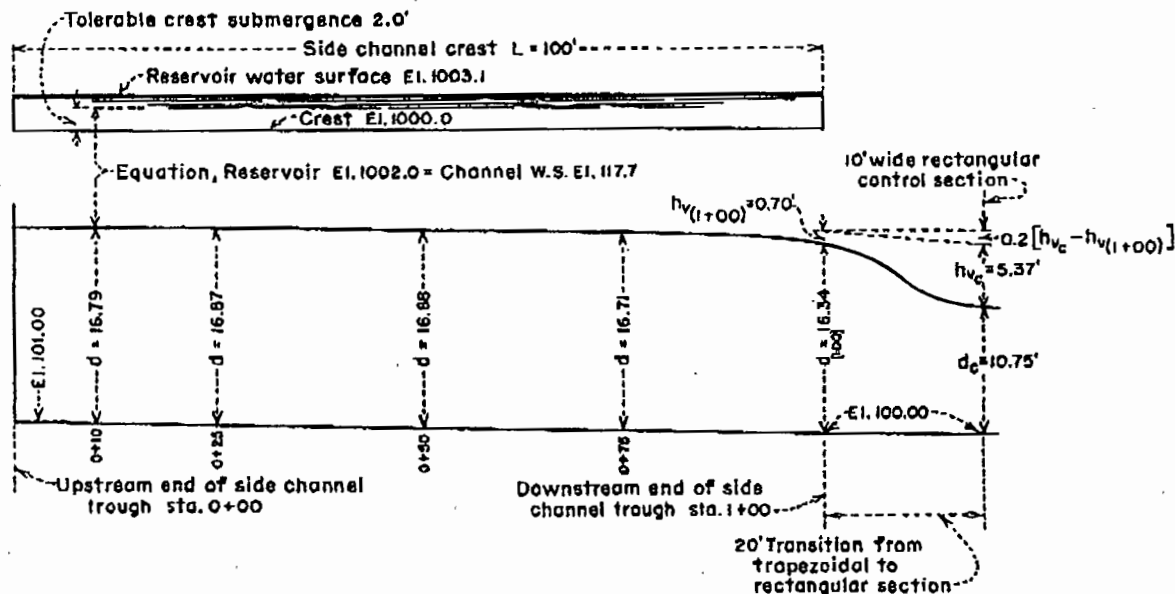


Figure 9-34.—Example of hydraulic design for side channel spillway. 288-D-2420.

Substituting the values in Bernoulli's equation:

$$16.34 + 0.70 = 10.75 + 5.37 + 0.93$$

$$17.04 = 17.05 \text{ (A satisfactory check)}$$

With the hydraulic properties of the side channel at station 1+00 determined, the water surface pro-

file along the side channel trough can be determined from equation (15). The trial-and-error computations are shown in table 9-3. The resulting water surface profile is shown on figure 9-34.

Next, the channel profile is fitted to the crest datum by relating the water surface profile to the

## SPILLWAYS

383

Table 9-3.—Side channel spillway computations. Using eq(15) for design example in section 9.17(b); given  $Q = 2,000 \text{ ft}^3/\text{s}$ , bottom width = 10 feet, side slopes =  $\frac{1}{2}:1$ , and bottom slope = 1 foot in 100 feet.

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)	(13)	(14)	(15)	(16)	(17)	(18)	(19)
Station	$\Delta x$	Elevation bottom	Trial $\Delta y$	Water surface elevation	$d$	$A$	$Q$	$v$	$Q_1 + Q_2$	$\frac{Q_1}{Q_1 + Q_2}$	$v_1 + v_2$	$v_2 - v_1$	$Q_2 - Q_1$	$\frac{Q_2 - Q_1}{Q_1}$	$\frac{v_2(Q_2 - Q_1)}{Q_1}$	(15)+(16)	$\Delta y = \frac{(11)}{x(12) \times (17)}$	Remarks
1+00	-	100.0	-	116.84	16.34	297	2,000	6.73	-	-	-	-	-	-	-	-	-	-
0+75	25	100.25	1.00	117.34	17.09	317	1,500	4.73	3,500	0.01332	11.46	2.00	500	0.333	2.34	4.34	0.64	Too low
			.62	116.96	16.71	307	-	4.89	-	-	11.62	1.84	-	-	-	4.08	.63	OK
0+50	25	100.50	.50	117.46	16.96	313	1,000	3.19	2,500	.01244	8.08	1.70	500	.60	2.44	4.14	.42	Too low
			.42	117.38	16.88	311	-	3.22	-	-	8.11	1.67	-	-	-	4.11	.41	OK
0+25	25	100.75	.30	117.68	16.93	313	500	1.60	1,500	.01038	4.82	1.62	500	1.00	3.22	4.84	.24	Too low
			.24	117.62	16.87	311	-	1.61	-	-	4.83	1.61	-	-	-	4.83	.24	OK
0+00	15	100.90	.10	117.72	16.82	310	200	.64	700	.00885	2.25	.97	300	1.50	2.41	3.38	.07	Too low
			.07	117.69	16.79	309	-	.65	-	-	2.26	.96	-	-	-	3.37	.07	OK

reservoir water level. To obtain the assumed crest coefficient value of 3.6, excessive submergence of the overflow must be avoided. If it is assumed that a maximum of two-thirds submergence at the upstream end of the channel can be tolerated, the maximum water surface level in the channel will be  $\frac{2}{3}H_c$  above the crest, or elevation 1002.0. Then at station 0+10, the channel datum water surface level elevation 117.7 will become elevation 1002.0, placing the channel floor level for station 0+00 at approximately elevation 985.3, and for station 1+00 at ap-

proximately elevation 984.3.

The design of the side channel control structure would be completed by designing the uncontrolled ogee crest by the methods shown in section 9.13, to obtain the crest coefficient value of 3.6 that was assumed.

Variations in the design can be made by assuming different bottom widths, different channel slopes, and varying control sections. A proper and economical design can usually be achieved after comparing several alternatives.

## D. HYDRAULICS OF FREE-FLOW DISCHARGE CHANNELS

**9.18. General.**—Discharge generally passes through the critical stage in the spillway control structure and enters the discharge channel as supercritical or shooting flow. To avoid a hydraulic jump below the control, the flow must remain at the supercritical stage throughout the length of the channel. The flow in the channel may be uniform or it may be accelerated or decelerated, depending on the slopes and dimensions of the channel and on the total drop. Where it is desired to minimize the grade to reduce excavation at the upstream end of a channel, the flow might be uniform or decelerating, followed by accelerating flow in the steep drop leading to the downstream river level. Flow at any point along the channel will depend upon the specific energy,  $d + h_v$ , available at that point. This energy will equal the total drop from the reservoir water level to the floor of the channel at the point under consideration, less the head losses accumulated to that point. The velocities and depths of flow along the channel can be fixed by selecting the

grade and the cross-sectional dimensions of the channel.

The velocities and depths of free surface flow in a channel, whether it be an open channel, a conduit, or a tunnel, conform to the principle of the conservation of energy as expressed by Bernoulli's theorem, which states "the absolute energy of flow at any cross section is equal to the absolute energy at a downstream section plus intervening losses of energy." As applied to figure 9-35 this relationship can be expressed as follows:

$$\Delta Z + d_1 + h_{v_1} = d_2 + h_{v_2} + \Delta h_L \quad (17)$$

When the channel grades are not too steep, for practical purposes the normal depth,  $d_n$ , can be considered equal to the vertical depth  $d$ . The term  $\Delta h_L$  includes all losses that occur in the reach of channel, such as friction, turbulence, impact, and transition losses. Because changes in most channels are made gradually, all losses except those from friction

# Spillway Channel Design SOUTH

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19
Station	dX	Channel bottom	trial dY	WSE	d	A	Q	v	Q1+Q2	$\frac{Q1}{g(Q1+Q2)}$	v1+v2	v2-v1	Q2-Q1	$\frac{Q2-Q1}{Q1}$	$\frac{v2(Q2-Q1)}{Q1}$	(13) + (16)	$\frac{dy}{(11)*(12)*(17)}$	Notes
2000.00	x	53.40	x	63.25	9.85	343.27	2496.20	7.27	x	x	x	x	x	x	x	x	x	
2100.00	100.00	54.50	1.02	64.27	9.77	339.70	1528.00	4.50	4024.20	0.02	11.77	2.77	968.20	0.39	1.74	4.52	1.02	ok
2150.00	50.00	55.30	0.35	64.62	9.32	319.86	990.90	3.10	2518.90	0.02	7.60	1.40	537.10	0.35	1.09	2.49	0.36	ok
2200.00	50.00	56.00	0.20	64.82	8.82	298.29	576.00	1.93	1566.90	0.02	5.03	1.17	414.90	0.42	0.81	1.98	0.20	ok
2250.00	50.00	56.70	0.08	64.90	8.20	272.24	296.00	1.09	872.00	0.02	3.02	0.84	280.00	0.49	0.53	1.37	0.08	ok
2300.00	50.00	57.40	0.03	64.93	7.53	244.95	132.00	0.54	428.00	0.02	1.63	0.55	164.00	0.55	0.30	0.85	0.03	ok
2350.00	50.00	58.10	0.01	64.94	6.84	217.79	44.00	0.20	176.00	0.02	0.74	0.34	88.00	0.67	0.13	0.47	0.01	ok

# Spillway Channel Design NORTH

1	2	3	4	5	6	7	8	9	10	11	12	13	14	15	16	17	18	19
Station	dX	Channel bottom	trial dY	WSE	d	A	Q	v	Q1+Q2	$\frac{Q1}{g(Q1+Q2)}$	v1+v2	v2-v1	Q2-Q1	$\frac{Q2-Q1}{Q1}$	$\frac{v2(Q2-Q1)}{Q1}$	(13) + (16)	$\frac{dy}{(11)*(12)*(17)}$	Notes
2000.00	x	53.40	x	63.50	10.10	354.51	2588.00	7.30	x	x	x	x	x	x	x	x	x	
2100.00	100.00	54.50	0.92	64.42	9.92	346.41	1717.00	4.96	4305.00	0.02	12.26	2.34	871.00	0.34	1.67	4.01	0.92	ok
2150.00	50.00	55.30	0.30	64.72	9.42	324.24	1311.00	4.04	3028.00	0.02	9.00	0.91	406.00	0.24	0.96	1.87	0.30	ok
2200.00	50.00	55.80	0.22	64.94	9.14	312.04	968.00	3.10	2279.00	0.02	7.15	0.94	343.00	0.26	0.81	1.75	0.22	ok
2250.00	50.00	56.50	0.14	65.08	8.58	288.12	684.00	2.37	1652.00	0.02	5.48	0.73	284.00	0.29	0.70	1.42	0.14	ok
2300.00	50.00	57.10	0.09	65.17	8.07	266.87	454.00	1.70	1138.00	0.02	4.08	0.67	230.00	0.34	0.57	1.24	0.09	ok
2350.00	50.00	57.80	0.06	65.23	7.43	240.95	274.00	1.14	728.00	0.02	2.84	0.56	180.00	0.40	0.45	1.01	0.06	ok

Station	South WSE	v	Freeboard	Parvin Weir Ht	Min TW	TW*
2000.00	63.25	7.27	1.32 ft	66.22	64.57	65.00
2100.00	64.27	4.50	1.00 ft	66.22	65.27	66.00
2150.00	64.62	3.10	1.00 ft	66.22	65.62	66.00
2200.00	64.82	1.93	1.00 ft	66.22	65.82	66.00
2250.00	64.90	1.09	1.00 ft	66.22	65.90	66.00
2300.00	64.93	0.54	1.00 ft	66.22	65.93	66.00
2350.00	64.94	0.20	1.00 ft	66.22	65.94	66.00

Station	North WSE	v	Freeboard	Parvin Weir Ht	Min TW	TW*
2000.00	63.50	7.30	1.33 ft	66.22	64.83	65.00
2100.00	64.42	4.96	1.00 ft	66.22	65.42	66.00
2150.00	64.72	4.04	1.00 ft	66.22	65.72	66.00
2200.00	64.94	3.10	1.00 ft	66.22	65.94	66.00
2250.00	65.08	2.37	1.00 ft	66.22	66.08	66.10
2300.00	65.17	1.70	1.00 ft	66.22	66.17	66.20
2350.00	65.23	1.14	1.00 ft	66.22	66.23	66.30

\* Wall is proposed to be this high or greater

**South**

Station	elevation bottom	Q	Bottom	Depth	q1	dc	vc	hvc	v	area	hv	Check # Depth	Check # Balance
20-00	53.40	2496.20	25.00	9.85	99.85	6.77	14.76	3.38	7.27	343.27	0.82	10.67	10.66

**North**

Station	elevation bottom	Q	Bottom	Depth	q1	dc	vc	hvc	v	area	hv	Check # Depth	Check # Balance
20-00	53.40	2587.90	25.00	10.10	103.52	6.93	14.94	3.46	7.30	354.51	0.83	10.93	10.92

Calculation to determine starting WSE

*NORTH BOX  
WSPGW MODEL*

T1 Green Park

0

T2 Alternative 1 for North Channel

T3 1/25/02

SO	631.900	47.200	9						50.000											
TS	632.000	47.200	9	.030																.000
TS	652.000	46.800	8	.030																.000
TS	675.000	46.300	7	.013																.000
TS	689.900	46.000	6	.013																.000
R	690.000	46.000	5	.013																.000
R	815.000	46.400	5	.013																.000
	.000	0																		
TS	865.000	46.600	2	.013																.000
R	1477.000	48.200	2	.013																.000
	.000	0																		
WE	1477.000	48.200	2	.500																
TS	1502.000	48.890	4	.013																.000
TS	1502.100	48.900	3	.013																.000
R	1578.000	50.790	3	.013																.000
SH	1578.000	50.790	3						58.060											
CD	1	2	0	.000	7.000	91.000	.000	.000	.00											
CD	3	2	0	.000	10.000	50.000	.000	.000	.00											
CD	5	3	5	.830	5.000	64.150	.000	.000	.00											
CD	6	2	0	.000	7.000	65.000	.000	.000	.00											
CD	7	2	0	.000	7.000	90.000	.000	.000	.00											
CD	8	2	0	.000	7.000	160.000	.000	.000	.00											
CD	9	2	0	.000	7.000	300.000	.000	.000	.00											
CD	2	6	0	.000		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
	.00	.00	.00																	
CD	4	6	0	.000		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
	.00	.00	.00																	
PTS	235	.000	.000	48.000	.000	47.750	1.400	47.000	3.200	46.000	4.530									
PTS		44.000	6.030	42.000	6.740	40.000	7.160	38.000	7.420	37.000	7.480									
PTS		36.000	7.500	35.000	7.480	34.000	7.420	32.000	7.160	30.000	6.740									
PTS		28.000	6.030	26.000	4.530	25.000	3.200	24.000	.010	23.000	3.200									
PTS		22.000	4.530	20.000	6.030	18.000	6.740	16.000	7.160	14.000	7.420									
PTS		13.000	7.480	12.000	7.500	11.000	7.480	10.000	7.420	8.000	7.160									
PTS		6.000	6.740	4.000	6.030	2.000	4.530	1.000	3.200	.250	1.400									
PTS	435	.000	.000	48.000	.000	47.750	3.400	47.000	5.200	46.000	6.530									
PTS		44.000	8.030	42.000	8.740	40.000	9.160	38.000	9.420	37.000	9.480									
PTS		36.000	9.500	35.000	9.480	34.000	9.420	32.000	9.160	30.000	9.740									
PTS		28.000	8.030	26.000	7.530	25.000	5.200	24.000	.010	23.000	5.200									
PTS		22.000	6.530	20.000	8.030	18.000	8.740	16.000	9.160	14.000	9.420									
PTS		13.000	9.480	12.000	9.500	11.000	9.480	10.000	9.420	8.000	9.160									
PTS		6.000	8.740	4.000	8.030	2.000	6.530	1.000	5.200	.250	3.400									
Q		5080.000	.0																	



FILE: ngreenalt3.WSW

W S P G W - CIVILDESIGN Version 12.91  
 For: PENTACORE Engineering, Las Vegas, Nevada - S/N 791  
 WATER SURFACE PROFILE LISTING

Green Park  
 Alternative 1 for North Channel  
 1/25/02

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Wid
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm
631.900	47.200	1.216	48.416	5080.00	13.93	3.01	51.43	.00	2.07	300.
TRANS STR	.0000					.0619	.01	1.22	2.23	
632.000	47.200	1.213	48.413	5080.00	13.96	3.03	51.44	.00	2.07	300.
TRANS STR	-.0200					.0725	1.45	1.21	2.23	
652.000	46.800	1.630	48.430	5080.00	19.48	5.89	54.32	.00	3.15	160.
TRANS STR	-.0217					.0111	.25	1.63	2.69	
675.000	46.300	3.033	49.333	5080.00	18.61	5.38	54.71	.00	4.63	90.
TRANS STR	-.0201					.0050	.07	3.03	1.88	
689.900	46.000	4.653	50.653	5080.00	18.20	5.14	55.79	.00	5.00	64.
.100	.0000					.0078	.00	4.65	1.54	
----- WARNING - Flow depth near top of box conduit -----										
690.000	46.000	4.652	50.652	5080.00	18.20	5.14	55.80	.00	5.00	64.
11.167	.0032					.0079	.09	4.65	1.54	5.
----- WARNING - Flow depth near top of box conduit -----										
701.167	46.036	4.609	50.645	5080.00	18.37	5.24	55.88	.00	5.00	64.
57.340	.0032					.0086	.49	4.61	1.56	5.
----- WARNING - Flow depth near top of box conduit -----										

FILE: ngreenalt3.WSW

W S P G W - CIVILDESIGN Version 12.91  
 For: PENTACORE Engineering, Las Vegas, Nevada - S/N 791  
 WATER SURFACE PROFILE LISTING

Green Park  
 Alternative 1 for North Channel  
 1/25/02

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Wid
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm
758.508	46.219	4.395	50.614	5080.00	19.26	5.76	56.38	.00	5.00	64.
56.492	.0032					.0098	.55	4.39	1.67	5.
815.000	46.400	4.190	50.590	5080.00	20.21	6.34	56.93	.00	5.00	64.
TRANS STR	.0040					.0067	.33	4.19	1.80	
865.000	46.600	6.833	53.433	5080.00	16.70	4.33	57.77	.00	6.83	35.
276.010	.0026					.0027	.75	6.83	1.00	7.4
1141.010	47.322	7.284	54.606	5080.00	15.93	3.94	58.54	.00	6.83	30.
335.990	.0026					.0027	.92	7.28	.86	7.4
1477.000	48.200	7.416	55.616	5080.00	15.74	3.85	59.46	.00	6.83	28.
HYDRAULIC JUMP										
1477.000	48.200	6.289	54.489	5080.00	17.89	4.97	59.46	.00	6.83	38.
HYDRAULIC DROP										
1502.000	48.890	6.960	55.850	5080.00	15.63	3.79	59.64	.00	6.96	42.
TRANS STR	.1000							6.960	1.000	
1502.100	48.900	4.751	53.651	5080.00	21.38	7.10	60.75	.00	6.85	50.
14.290	.0249					.0053	.08	4.75	1.73	2.

FILE: ngreenalt3.WSW

W S P G W - CIVILDESIGN Version 12.91  
 For: PENTACORE Engineering, Las Vegas, Nevada - S/N 791  
 WATER SURFACE PROFILE LISTING

Green Park  
 Alternative 1 for North Channel  
 1/25/02

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Wid
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm
1516.390	49.256	4.903	54.159	5080.00	20.72	6.67	60.83	.00	6.85	50.
18.132	.0249					.0047	.08	4.90	1.65	2.
1534.522	49.707	5.142	54.849	5080.00	19.76	6.06	60.91	.00	6.85	50.
14.375	.0249					.0040	.06	5.14	1.54	2.
1548.897	50.065	5.393	55.458	5080.00	18.84	5.51	60.97	.00	6.85	50.
11.095	.0249					.0035	.04	5.39	1.43	2.
1559.992	50.342	5.656	55.998	5080.00	17.96	5.01	61.01	.00	6.85	50.
8.189	.0249					.0030	.02	5.66	1.33	2.
1568.181	50.545	5.932	56.478	5080.00	17.13	4.55	61.03	.00	6.85	50.
5.579	.0249					.0026	.01	5.93	1.24	2.
1573.760	50.684	6.222	56.906	5080.00	16.33	4.14	61.05	.00	6.85	50.
3.209	.0249					.0022	.01	6.22	1.15	2.
1576.969	50.764	6.526	57.290	5080.00	15.57	3.76	61.05	.00	6.85	50.
1.031	.0249					.0019	.00	6.53	1.07	2.
1578.000	50.790	6.845	57.635	5080.00	14.84	3.42	61.06	.00	6.85	50.

*COLLECTER SYSTEM  
HEC-RAS MODEL*

HEC-RAS Plan: Plan 01

Reach	River Sta	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Crit W.S. (ft)	E.G. Elev (ft)	E.G. Slope (ft/ft)	Vel Chnl (ft/s)	Flow Area (sq ft)	Top Width (ft)	Froude # Chl
South	2600	44.00	63.30	64.40	64.40	64.86	0.002739	5.69	8.30	9.19	0.96
South	2550	44.00	63.04	63.96	63.53	64.02	0.000384	2.06	21.98	25.76	0.38
South	2500	44.00	60.60	64.00		64.00	0.000004	0.49	93.39	30.93	0.05
South	2450	44.00	59.50	64.00		64.00	0.000003	0.44	105.78	28.00	0.04
South	2400	44.00	58.80	64.00		64.00	0.000001	0.36	132.40	30.91	0.03
South	2350	44.00	58.10	64.00		64.00	0.000001	0.30	159.28	32.97	0.02
South	2300	132.00	57.40	63.99		64.00	0.000005	0.75	187.99	35.02	0.05
South	2250	296.00	56.70	63.97		64.00	0.000018	1.51	210.51	35.94	0.10
South	2200	576.00	56.00	63.89		63.99	0.000050	2.62	236.04	36.81	0.16
South	2150	991.00	55.30	63.72		63.97	0.000116	4.16	255.09	36.58	0.25
South	2100	1528.00	54.50	63.46		63.93	0.000204	5.75	291.03	43.11	0.34
South	2050	2496.00	53.80	63.10		63.84	0.000326	7.45	373.28	56.58	0.43
South	2000	2496.00	53.40	61.95		63.70	0.000942	10.62	234.99	27.50	0.64
North	2550	274.00	63.50	65.39	65.39	66.06	0.002052	7.23	43.54	32.46	0.93
North	2500	274.00	60.80	61.77	62.66	65.28	0.020372	15.38	18.57	21.38	2.76
North	2450	274.00	59.10	63.99		64.07	0.000065	2.27	131.58	34.77	0.18
North	2400	274.00	58.70	64.01		64.06	0.000036	1.81	164.09	38.80	0.14
North	2350	274.00	57.80	64.02		64.05	0.000017	1.38	212.34	41.26	0.10
North	2300	454.00	57.10	63.99		64.04	0.000032	2.02	240.69	41.91	0.14
North	2250	684.00	56.50	63.92		64.03	0.000058	2.84	257.35	41.38	0.18
North	2200	968.00	55.80	63.83		64.02	0.000089	3.66	283.23	42.56	0.23
North	2150	1311.00	55.30	63.69		64.00	0.000136	4.65	301.02	42.79	0.28
North	2100	1717.00	54.50	63.53		63.96	0.000174	5.49	333.76	43.93	0.32
North	2050	2588.00	53.80	63.38		63.89	0.000204	6.16	464.71	66.01	0.35
North	2000	2588.00	53.40	61.74		63.72	0.001085	11.29	229.26	27.50	0.69
East System	2000	5080.00	53.40	61.80		63.68	0.000772	10.99	462.26	55.00	0.67
East System	1950	5080.00	53.20	60.02	60.02	63.47	0.001808	14.89	341.24	50.00	1.00
East System	1778	5080.00	52.29	58.72	59.11	62.60	0.002172	15.81	321.32	50.00	1.10
East System	1578	5080.00	50.79	57.63	57.63	61.06	0.001793	14.85	342.18	50.00	1.00

HEC-RAS September 1998 Version 2.2  
 U.S. Army Corp of Engineers  
 Hydrologic Engineering Center  
 609 Second Street, Suite D  
 Davis, California 95616-4687  
 (916) 756-1104

```

X   X   XXXXXX   XXXX   XXXX   XX   XXXX
X   X   X       X   X   X   X   X   X
X   X   X       X   X   X   X   X   X
XXXXXXXX XXXX   X   XXX XXXX   XXXXXX   XXXX
X   X   X   X   X   X   X   X   X   X
X   X   X   X   X   X   X   X   X   X
X   X   XXXXXX   XXXX   X   X   X   X   XXXXX

```

## PROJECT DATA

Project Title: Greenpark Junction  
 Project File : gpl.prj  
 Run Date and Time: 3/15/2002 10:16:30 AM

Project in English units

## PLAN DATA

Plan Title: Plan 01  
 Plan File : w:\HYDRO\Greenpark HEC-RAS\gpl.p01

Geometry Title: Geom 01  
 Geometry File : w:\HYDRO\Greenpark HEC-RAS\gpl.g01

Flow Title : Flow 01  
 Flow File : w:\HYDRO\Greenpark HEC-RAS\gpl.f01

## Plan Summary Information:

Number of: Cross Sections =	66	Multiple Openings =	0
Culverts =	0	Inline Weirs =	0
Bridges =	0		

## Computational Information

Water surface calculation tolerance =	0.01
Critical depth calculation tolerance =	0.01
Maximum number of iterations =	20
Maximum difference tolerance =	0.3
Flow tolerance factor =	0.001

## Computation Options

Critical depth computed only where necessary	
Conveyance Calculation Method:	At breaks in n values only
Friction Slope Method:	Average Conveyance
Computational Flow Regime:	Mixed Flow

## FLOW DATA

Flow Title: Flow 01  
 Flow File : w:\HYDRO\Greenpark HEC-RAS\gpl.f01

## Flow Data (cfs)

River	Reach	RS	PF 1
Collector SystemSouth		2600	44
Collector SystemSouth		2350	44
Collector SystemSouth		2300	132
Collector SystemSouth		2250	296
Collector SystemSouth		2200	576
Collector SystemSouth		2150	991
Collector SystemSouth		2100	1528
Collector SystemSouth		2050	2496
Collector SystemNorth		2550	274
Collector SystemNorth		2350	274

Collector SystemNorth	2300	454
Collector SystemNorth	2250	684
Collector SystemNorth	2200	968
Collector SystemNorth	2150	1311
Collector SystemNorth	2100	1717
Collector SystemNorth	2050	2588
Collector SystemEast System	2000	5080

## Boundary Conditions

River	Reach	Profile	Upstream	Downstre
Collector SystemSouth		PF 1	Critical	
Collector SystemNorth		PF 1	Critical	
Collector SystemEast System		PF 1		Critic

## GEOMETRY DATA

Geometry Title: Geom 01  
 Geometry File : w:\HYDRO\Greenpark HEC-RAS\gpl.g01

## Reach Connection Table

River	Reach	Upstream Boundary	Downstream Boundary
Collector System South			Jct1
Collector System North			Jct1
Collector System East System		Jct1	

CROSS SECTION RIVER: Collector System  
 REACH: South RS: 2600

## INPUT

## Description:

Station Elevation Data		num=	8						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
68	67.07	74	67	80	66	96	65	99	64
101	63.3	107	63.3	107	68				

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
68	.013	101	.013	107	.013

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff Contr.	Expan.
101	107	25	25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: South RS: 2575.4

## INPUT

## Description:

Station Elevation Data		num=	10						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
68.5	66.93	73.41	66.71	78.32	66.02	78.79	65.98	85.21	65.24
91.41	64.41	93.86	63.68	95.5	63.17	109.5	63.17	109.5	68

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
68.5	.013	95.5	.013	109.5	.013

Bank Sta: Left	Right	Lengths: Left	Channel	Right	Coeff Contr.	Expan.
95.5	109.5	25	25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: South RS: 2550

## INPUT

## Description:

Station Elevation Data		num=	6						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
69	66.79	77	66	82	65	90	63.04	112	63.04
112	68								

Manning's n Values num= 3



Sta	n Val	Sta	n Val	Sta	n Val		
69	.013	90	.013	112	.013		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	90	112		25 25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: South RS: 2525.\*

## INPUT

## Description:

Station	Elevation	Data	num=	7					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
68	66.65	75.43	66.01	75.58	65.98	80.07	64.48	87.5	61.82
110.5	61.82	110.5	68						

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
68	.013	87.5	.013	110.5	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	87.5	110.5		25 25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: South RS: 2500

## INPUT

## Description:

Station	Elevation	Data	num=	5					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	66.51	74	66	85	60.6	109	60.6	109	68

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
67	.013	85	.013	109	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	85	109		25 25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: South RS: 2475.\*

## INPUT

## Description:

Station	Elevation	Data	num=	6					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	66.37	74.43	66.01	74.58	65.96	86.5	60.05	108	60.05
108	68								

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
67	.013	86.5	.013	108	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	86.5	108		25 25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: South RS: 2450

## INPUT

## Description:

Station	Elevation	Data	num=	5					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	66.23	75	66	88	59.5	107	59.5	107	68

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	107	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	88	107		25 25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: South RS: 2425.\*

## INPUT

## Description:

Station	Elevation	Data	num=	5					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev

67	66.09	75	65.5	88	59.15	107.5	59.15	107.5	68
----	-------	----	------	----	-------	-------	-------	-------	----

Manning's n Values      num=      3

Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	107.5	.013

Bank Sta: Left      Right      Lengths: Left Channel      Right      Coeff Contr.      Expan.

88	107.5	25	25	25	.1	.3
----	-------	----	----	----	----	----

CROSS SECTION      RIVER: Collector System  
 REACH: South      RS: 2400

## INPUT

## Description:

Station Elevation Data		num=	5						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	65.95	75	65	88	58.8	108	58.8	108	68

Manning's n Values      num=      3

Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	108	.013

Bank Sta: Left      Right      Lengths: Left Channel      Right      Coeff Contr.      Expan.

88	108	25	25	25	.1	.3
----	-----	----	----	----	----	----

CROSS SECTION      RIVER: Collector System  
 REACH: South      RS: 2375.4

## INPUT

## Description:

Station Elevation Data		num=	6						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66	65.82	74.38	65.01	74.61	64.95	88	58.45	108.5	58.45
108.5	68								

Manning's n Values      num=      3

Sta	n Val	Sta	n Val	Sta	n Val
66	.013	88	.013	108.5	.013

Bank Sta: Left      Right      Lengths: Left Channel      Right      Coeff Contr.      Expan.

88	108.5	25	25	25	.1	.3
----	-------	----	----	----	----	----

CROSS SECTION      RIVER: Collector System  
 REACH: South      RS: 2350

## INPUT

## Description:

Station Elevation Data		num=	5						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
65	65.68	74	65	88	58.1	109	58.1	109	68

Manning's n Values      num=      3

Sta	n Val	Sta	n Val	Sta	n Val
65	.013	88	.013	109	.013

Bank Sta: Left      Right      Lengths: Left Channel      Right      Coeff Contr.      Expan.

88	109	25	25	25	.1	.3
----	-----	----	----	----	----	----

CROSS SECTION      RIVER: Collector System  
 REACH: South      RS: 2325.4

## INPUT

## Description:

Station Elevation Data		num=	6						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66	65.54	72.29	65.09	74.61	64.44	88	57.75	109.5	57.75
109.5	68								

Manning's n Values      num=      3

Sta	n Val	Sta	n Val	Sta	n Val
66	.013	88	.013	109.5	.013

Bank Sta: Left      Right      Lengths: Left Channel      Right      Coeff Contr.      Expan.

88	109.5	25	25	25	.1	.3
----	-------	----	----	----	----	----

CROSS SECTION      RIVER: Collector System  
 REACH: South      RS: 2300

## INPUT

## Description:

Station Elevation Data		num= 5		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	65.4	73	65	88	57.4	110	57.4	110	68

## Manning's n Values

num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	110	.013

Bank Sta: Left		Right	Lengths: Left Channel		Right	Coeff Contr.	Expan.
Sta	Elev	Sta	Length	Length	Sta	Coeff	Expan
88	110	25	25	25	25	.1	.3

CROSS SECTION  
REACH: SouthRIVER: Collector System  
RS: 2275.\*

## INPUT

## Description:

Station Elevation Data		num= 6		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	65.18	73	64.57	74	64.25	88	57.05	110	57.05
110	67								

## Manning's n Values

num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	110	.013

Bank Sta: Left		Right	Lengths: Left Channel		Right	Coeff Contr.	Expan.
Sta	Elev	Sta	Length	Length	Sta	Coeff	Expan
88	110	25	25	25	25	.1	.3

CROSS SECTION  
REACH: SouthRIVER: Collector System  
RS: 2250

## INPUT

## Description:

Station Elevation Data		num= 5		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	64.96	74	64	88	56.7	110	56.7	110	66

## Manning's n Values:

num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	110	.013

Bank Sta: Left		Right	Lengths: Left Channel		Right	Coeff Contr.	Expan.
Sta	Elev	Sta	Length	Length	Sta	Coeff	Expan
88	110	25	25	25	25	.1	.3

CROSS SECTION  
REACH: SouthRIVER: Collector System  
RS: 2225.\*

## INPUT

## Description:

Station Elevation Data		num= 5		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	64.74	74	64	88	56.35	110.5	56.35	110.5	66

## Manning's n Values:

num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	110.5	.013

Bank Sta: Left		Right	Lengths: Left Channel		Right	Coeff Contr.	Expan.
Sta	Elev	Sta	Length	Length	Sta	Coeff	Expan
88	110.5	25	25	25	25	.1	.3

CROSS SECTION  
REACH: SouthRIVER: Collector System  
RS: 2200

## INPUT

## Description:

Station Elevation Data		num= 5		Sta Elev		Sta Elev		Sta Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	64.51	74	64	88	56	111	56	111	66

## Manning's n Values:

num= 3		Sta n Val		Sta n Val	
Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	111	.013

Bank Sta: Left		Right	Lengths: Left Channel		Right	Coeff Contr.	Expan.
Sta	Elev	Sta	Length	Length	Sta	Coeff	Expan
88	111	25	25	25	25	.1	.3

CROSS SECTION  
REACH: SouthRIVER: Collector System  
RS: 2175.\*

## INPUT

## Description:

Station Elevation Data		num=	6						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	64.29	74	64	75	63.71	88	55.65	111.5	55.65
111.5	65.5								

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	111.5	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	88	111.5		25	25		.1	.3

CROSS SECTION  
REACH: SouthRIVER: Collector System  
RS: 2150

## INPUT

## Description:

Station Elevation Data		num=	5						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	64.07	75	64	88	55.3	112	55.3	112	65

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	112	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	88	112		25	25		.1	.3

CROSS SECTION  
REACH: SouthRIVER: Collector System  
RS: 2125.\*

## INPUT

## Description:

Station Elevation Data		num=	6						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	63.93	74	63.5	75	63.2	88	54.9	112.5	54.9
112.5	65								

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	112.5	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	88	112.5		25	25		.1	.3

CROSS SECTION  
REACH: SouthRIVER: Collector System  
RS: 2100

## INPUT

## Description:

Station Elevation Data		num=	5						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	63.79	74	63	88	54.5	113	54.5	113	65

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
67	.013	88	.013	113	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	88	113		37.5	25		.1	.3

CROSS SECTION  
REACH: SouthRIVER: Collector System  
RS: 2075.\*

## INPUT

## Description:

Station Elevation Data		num=	6						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
58.5	63.58	66.26	63.08	68.33	62.56	88	54.15	113.5	54.15
113.5	64.5								

## Manning's n Values

Sta	n Val	Sta	n Val	Sta	n Val
58.5	.013	88	.013	113.5	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.

88 113.5 37.5 25 20 .1 .3

CROSS SECTION RIVER: Collector System  
REACH: South RS: 2050

## INPUT

Description:

Station Elevation Data		num=	5						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
50	63.37	60	63	88	53.8	114	53.8	114	64

Manning's n Values		num=	3						
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
50	.013	88	.013	114	.013				

Bank Sta: Left		Right	Lengths: Left Channel		Right	Coeff	Contr.	Expan.
88	114		40.01	25	20	.1	.3	

CROSS SECTION RIVER: Collector System  
REACH: South RS: 2025.\*

## INPUT

Description:

Station Elevation Data		num=	6						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
69	62.93	74	62.75	88	58.15	88	53.6	114.75	53.6
114.75	63.5								

Manning's n Values		num=	3						
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
69	.013	88	.013	114.75	.013				

Bank Sta: Left		Right	Lengths: Left Channel		Right	Coeff	Contr.	Expan.
88	114.75		40.01	25	20	.1	.3	

CROSS SECTION RIVER: Collector System  
REACH: South RS: 2000

## INPUT

Description:

Station Elevation Data		num=	4						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
88	62.5	88	53.4	115.5	53.4	115.5	63		

Manning's n Values		num=	3						
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
88	.013	88	.013	115.5	.013				

Bank Sta: Left		Right	Lengths: Left Channel		Right	Coeff	Contr.	Expan.
88	115.5		0	0	0	.1	.3	

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2550

## INPUT

Description:

Station Elevation Data		num=	5						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
69	66.02	78	65	94	63.5	107	63.5	107	65

Manning's n Values		num=	3						
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
69	.013	94	.013	107	.013				

Bank Sta: Left		Right	Lengths: Left Channel		Right	Coeff	Contr.	Expan.
94	107		25	25	25	.1	.3	

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2525.\*

## INPUT

Description:

Station Elevation Data		num=	6						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
68.5	65.92	71.7	65.32	76.96	64.46	92	62.15	107	62.15
107	65								

Manning's n Values		num=	3						
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val

68.5 .013 92 .013 107 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
92 107 25 25 25 .1 .3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2500

INPUT  
Description:  
Station Elevation Data num= 5  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
68 65.82 71 65 90 60.8 107 60.8 107 65

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
68 .013 90 .013 107 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
90 107 25 25 25 .1 .3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2475.\*

INPUT  
Description:  
Station Elevation Data num= 5  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
67.5 65.72 70.43 64.87 89 59.95 107 59.95 107 65.5

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
67.5 .013 89 .013 107 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
89 107 25 25 25 .1 .3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2450

INPUT  
Description:  
Station Elevation Data num= 4  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
67 65.62 88 59.1 107 59.1 107 66

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
67 .013 88 .013 107 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
88 107 25 25 25 .1 .3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2425.\*

INPUT  
Description:  
Station Elevation Data num= 4  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
67 65.52 87.5 58.9 108.5 58.9 108.5 65.5

Manning's n Values num= 3  
Sta n Val Sta n Val Sta n Val  
67 .013 87.5 .013 108.5 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
87.5 108.5 25 25 25 .1 .3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2400

INPUT  
Description:  
Station Elevation Data num= 4  
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
67 65.42 87 58.7 110 58.7 110 65

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val		
67	.013	87	.013	110	.013		
Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	87	110		25	25		.1 .3
CROSS SECTION		RIVER: Collector System					
REACH: North		RS: 2375.*					
INPUT							
Description:							
Station Elevation Data		num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66.5	65.32	85	58.25	110	58.25	110	65
Manning's n Values		num=	3				
Sta	n Val	Sta	n Val	Sta	n Val		
66.5	.013	85	.013	110	.013		
Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	85	110		25	25		.1 .3
CROSS SECTION		RIVER: Collector System					
REACH: North		RS: 2350					
INPUT							
Description:							
Station Elevation Data		num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66	65.22	83	57.8	110	57.8	110	65
Manning's n Values		num=	3				
Sta	n Val	Sta	n Val	Sta	n Val		
66	.013	83	.013	110	.013		
Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	83	110		25	25		.1 .3
CROSS SECTION		RIVER: Collector System					
REACH: North		RS: 2325.*					
INPUT							
Description:							
Station Elevation Data		num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66	65.12	82.5	57.45	110	57.45	110	65
Manning's n Values		num=	3				
Sta	n Val	Sta	n Val	Sta	n Val		
66	.013	82.5	.013	110	.013		
Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	82.5	110		25	25		.1 .3
CROSS SECTION		RIVER: Collector System					
REACH: North		RS: 2300					
INPUT							
Description:							
Station Elevation Data		num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66	65.02	82	57.1	110	57.1	110	65
Manning's n Values		num=	3				
Sta	n Val	Sta	n Val	Sta	n Val		
66	.013	82	.013	110	.013		
Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	82	110		25	25		.1 .3
CROSS SECTION		RIVER: Collector System					
REACH: North		RS: 2275.*					
INPUT							
Description:							
Station Elevation Data		num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66.5	64.92	82	56.8	110	56.8	110	65



Manning's n Values		num= 3					
Sta	n Val	Sta	n Val	Sta	n Val		
66.5	.013	82	.013	110	.013		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	82	110		25	25	25		.1	.3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2250

INPUT  
Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	64.82	82	56.5	110	56.5	110	65

Manning's n Values		num= 3					
Sta	n Val	Sta	n Val	Sta	n Val		
67	.013	82	.013	110	.013		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	82	110		25	25	25		.1	.3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2225.\*

INPUT  
Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66.5	64.72	82	56.15	110	56.15	110	65

Manning's n Values		num= 3					
Sta	n Val	Sta	n Val	Sta	n Val		
66.5	.013	82	.013	110	.013		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	82	110		25	25	25		.1	.3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2200

INPUT  
Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66	64.62	82	55.8	110	55.8	110	65

Manning's n Values		num= 3					
Sta	n Val	Sta	n Val	Sta	n Val		
66	.013	82	.013	110	.013		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	82	110		25	25	25		.1	.3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2175.'

INPUT  
Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
66.5	64.52	82	55.55	110.5	55.55	110.5	64.5

Manning's n Values		num= 3					
Sta	n Val	Sta	n Val	Sta	n Val		
66.5	.013	82	.013	110.5	.013		

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	82	110.5		25	25	25		.1	.3

CROSS SECTION RIVER: Collector System  
REACH: North RS: 2150

INPUT  
Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
67	64.42	82	55.3	111	55.3	111	64

Manning's n Values      num=      3  
 Sta    n Val      Sta    n Val      Sta    n Val  
   67    .013      82    .013      111    .013

Bank Sta: Left    Right    Lengths: Left Channel    Right    Coeff Contr.    Expan.  
                  82      111               22.5      25      32.5               .1               .3

CROSS SECTION               RIVER: Collector System  
 REACH: North               RS: 2125.4

## INPUT

Description:

Station Elevation Data      num=      4  
 Sta    Elev      Sta    Elev      Sta    Elev      Sta    Elev  
   67    64.32      82    54.9      111.5    54.9      111.5    64

Manning's n Values      num=      3  
 Sta    n Val      Sta    n Val      Sta    n Val  
   67    .013      82    .013      111.5    .013

Bank Sta: Left    Right    Lengths: Left Channel    Right    Coeff Contr.    Expan.  
                  82      111.5               22.5      25      32.5               .1               .3

CROSS SECTION               RIVER: Collector System  
 REACH: North               RS: 2100

## INPUT

Description:

Station Elevation Data      num=      4  
 Sta    Elev      Sta    Elev      Sta    Elev      Sta    Elev  
   67    64.22      82    54.5      112    54.5      112    64

Manning's n Values      num=      3  
 Sta    n Val      Sta    n Val      Sta    n Val  
   67    .013      82    .013      112    .013

Bank Sta: Left    Right    Lengths: Left Channel    Right    Coeff Contr.    Expan.  
                  82      112               20      25      30               .1               .3

CROSS SECTION               RIVER: Collector System  
 REACH: North               RS: 2075.4

## INPUT

Description:

Station Elevation Data      num=      4  
 Sta    Elev      Sta    Elev      Sta    Elev      Sta    Elev  
   58.5    64.21      85    54.15      115.5    54.15      115.5    64

Manning's n Values      num=      3  
 Sta    n Val      Sta    n Val      Sta    n Val  
   58.5    .013      85    .013      115.5    .013

Bank Sta: Left    Right    Lengths: Left Channel    Right    Coeff Contr.    Expan.  
                  85      115.5               20      25      30               .1               .3

CROSS SECTION               RIVER: Collector System  
 REACH: North               RS: 2050

## INPUT

Description:

Station Elevation Data      num=      4  
 Sta    Elev      Sta    Elev      Sta    Elev      Sta    Elev  
   50    64.2      88    53.8      119    53.8      119    64

Manning's n Values      num=      3  
 Sta    n Val      Sta    n Val      Sta    n Val  
   50    .013      88    .013      119    .013

Bank Sta: Left    Right    Lengths: Left Channel    Right    Coeff Contr.    Expan.  
                  88      119               17.5      25      40               .1               .3

CROSS SECTION               RIVER: Collector System  
 REACH: North               RS: 2025.4

## INPUT

Description:

Station Elevation Data      num=      5  
 Sta    Elev      Sta    Elev      Sta    Elev      Sta    Elev      Sta    Elev

69	63.35	88	58.15	88	53.6	117.25	53.6	117.25	63.5
----	-------	----	-------	----	------	--------	------	--------	------

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 69 .013 88 .013 117.25 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 88 117.25 17.5 25 40 .1 .3

CROSS SECTION RIVER: Collector System  
 REACH: North RS: 2000

INPUT  
 Description:  
 Station Elevation Data num= 4  
 Sta Elev Sta Elev Sta Elev Sta Elev  
 88 62.5 88 53.4 115.5 53.4 115.5 63

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 88 .013 88 .013 115.5 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 88 115.5 0 0 0 .1 .3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 2000

INPUT  
 Description:  
 Station Elevation Data num= 5  
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
 72.5 63 72.5 53.4 100 53.4 127.5 53.4 127.5 63.1

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 72.5 .013 72.5 .013 127.5 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 72.5 127.5 25 25 25 .1 .3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1975.\*

INPUT  
 Description:  
 Station Elevation Data num= 5  
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
 73.75 62.6 73.75 53.3 100 53.3 126.25 53.3 126.25 62.65

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 73.75 .013 73.75 .013 126.25 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 73.75 126.25 25 25 25 .1 .3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1950

INPUT  
 Description:  
 Station Elevation Data num= 5  
 Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev  
 75 62.2 75 53.2 100 53.2 125 53.2 125 62.2

Manning's n Values num= 3  
 Sta n Val Sta n Val Sta n Val  
 75 .013 75 .013 125 .013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 75 125 24.57 24.57 24.57 .1 .3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1925.42\*

INPUT  
 Description:  
 Station Elevation Data num= 5

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	62.03	75	53.07	100	53.07	125	53.07	125	62.03

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Sta	Left	Right	Lengths	Left	Channel	Right	Coeff	Contr.	Expan.
75		125	24.57	24.57	24.57		.1	.3	

CROSS SECTION RIVER: Collector System  
REACH: East System RS: 1900.85\*

INPUT  
Description:

Station	Elevation	Data	num=	5					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	61.86	75	52.94	100	52.94	125	52.94	125	61.86

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Sta	Left	Right	Lengths	Left	Channel	Right	Coeff	Contr.	Expan.
75		125	24.57	24.57	24.57		.1	.3	

CROSS SECTION RIVER: Collector System  
REACH: East System RS: 1876.28\*

INPUT  
Description:

Station	Elevation	Data	num=	5					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	61.69	75	52.81	100	52.81	125	52.81	125	61.69

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Sta	Left	Right	Lengths	Left	Channel	Right	Coeff	Contr.	Expan.
75		125	24.57	24.57	24.57		.1	.3	

CROSS SECTION RIVER: Collector System  
REACH: East System RS: 1851.71\*

INPUT  
Description:

Station	Elevation	Data	num=	5					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	61.51	75	52.68	100	52.68	125	52.68	125	61.51

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Sta	Left	Right	Lengths	Left	Channel	Right	Coeff	Contr.	Expan.
75		125	24.57	24.57	24.57		.1	.3	

CROSS SECTION RIVER: Collector System  
REACH: East System RS: 1827.14\*

INPUT  
Description:

Station	Elevation	Data	num=	5					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	61.44	75	52.55	100	52.55	125	52.55	125	61.44

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Sta	Left	Right	Lengths	Left	Channel	Right	Coeff	Contr.	Expan.
75		125	24.57	24.57	24.57		.1	.3	

CROSS SECTION RIVER: Collector System  
REACH: East System RS: 1802.57\*

INPUT  
Description:

Station Elevation Data		num=	5						
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	61.17	75	52.42	100	52.42	125	52.42	125	61.17

Manning's n Values		num=	3						
Sta	n Val	Sta	n Val	Sta	n Val				
75	.013	75	.013	125	.013				

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	75	125		24.57 24.57	24.57	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1778

INPUT  
 Description:

Station Elevation Data		num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	61	75	52.29	125	52.29	125	61

Manning's n Values		num=	3				
Sta	n Val	Sta	n Val	Sta	n Val		
75	.013	75	.013	125	.013		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	75	125		25 25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1753.\*

INPUT  
 Description:

Station Elevation Data		num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	60.88	75	52.1	125	52.1	125	60.88

Manning's n Values		num=	3				
Sta	n Val	Sta	n Val	Sta	n Val		
75	.013	75	.013	125	.013		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	75	125		25 25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1728.\*

INPUT  
 Description:

Station Elevation Data		num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	60.75	75	51.92	125	51.92	125	60.75

Manning's n Values		num=	3				
Sta	n Val	Sta	n Val	Sta	n Val		
75	.013	75	.013	125	.013		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	75	125		25 25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1703.\*

INPUT  
 Description:

Station Elevation Data		num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	60.62	75	51.73	125	51.73	125	60.62

Manning's n Values		num=	3				
Sta	n Val	Sta	n Val	Sta	n Val		
75	.013	75	.013	125	.013		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	75	125		25 25	25	.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1678.\*

INPUT

## Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	60.5	75	51.54	125	51.54	125	60.5

## Manning's n Values

num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	75	125		25	25		.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1653.\*

## INPUT

## Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	60.38	75	51.35	125	51.35	125	60.38

## Manning's n Values

num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	75	125		25	25		.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1628.\*

## INPUT

## Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	60.25	75	51.17	125	51.17	125	60.25

## Manning's n Values

num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	75	125		25	25		.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1603.\*

## INPUT

## Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	60.12	75	50.98	125	50.98	125	60.12

## Manning's n Values

num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	75	125		25	25		.1	.3

CROSS SECTION RIVER: Collector System  
 REACH: East System RS: 1578

## INPUT

## Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
75	60	75	50.79	125	50.79	125	60

## Manning's n Values

num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
75	.013	75	.013	125	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	75	125		0	0		.1	.3

## SUMMARY OF MANNING'S N VALUES

## River:Collector System

Reach	River Sta.	n1	n2	n3
South	2600	.013	.013	.013
South	2575.*	.013	.013	.013
South	2550	.013	.013	.013
South	2525.*	.013	.013	.013
South	2500	.013	.013	.013
South	2475.*	.013	.013	.013
South	2450	.013	.013	.013
South	2425.*	.013	.013	.013
South	2400	.013	.013	.013
South	2375.*	.013	.013	.013
South	2350	.013	.013	.013
South	2325.*	.013	.013	.013
South	2300	.013	.013	.013
South	2275.*	.013	.013	.013
South	2250	.013	.013	.013
South	2225.*	.013	.013	.013
South	2200	.013	.013	.013
South	2175.*	.013	.013	.013
South	2150	.013	.013	.013
South	2125.*	.013	.013	.013
South	2100	.013	.013	.013
South	2075.*	.013	.013	.013
South	2050	.013	.013	.013
South	2025.*	.013	.013	.013
South	2000	.013	.013	.013
North	2550	.013	.013	.013
North	2525.*	.013	.013	.013
North	2500	.013	.013	.013
North	2475.*	.013	.013	.013
North	2450	.013	.013	.013
North	2425.*	.013	.013	.013
North	2400	.013	.013	.013
North	2375.*	.013	.013	.013
North	2350	.013	.013	.013
North	2325.*	.013	.013	.013
North	2300	.013	.013	.013
North	2275.*	.013	.013	.013
North	2250	.013	.013	.013
North	2225.*	.013	.013	.013
North	2200	.013	.013	.013
North	2175.*	.013	.013	.013
North	2150	.013	.013	.013
North	2125.*	.013	.013	.013
North	2100	.013	.013	.013
North	2075.*	.013	.013	.013
North	2050	.013	.013	.013
North	2025.*	.013	.013	.013
North	2000	.013	.013	.013
East System	2000	.013	.013	.013
East System	1975.*	.013	.013	.013
East System	1950	.013	.013	.013
East System	1925.42*	.013	.013	.013
East System	1900.85*	.013	.013	.013
East System	1876.28*	.013	.013	.013
East System	1851.71*	.013	.013	.013
East System	1827.14*	.013	.013	.013
East System	1802.57*	.013	.013	.013
East System	1778	.013	.013	.013
East System	1753.*	.013	.013	.013
East System	1728.*	.013	.013	.013
East System	1703.*	.013	.013	.013
East System	1678.*	.013	.013	.013
East System	1653.*	.013	.013	.013
East System	1628.*	.013	.013	.013
East System	1603.*	.013	.013	.013
East System	1578	.013	.013	.013

## SUMMARY OF REACH LENGTHS

River: Collector System



Reach	River Sta.	Left	Channel	Right
South	2600	25	25	25
South	2575.*	25	25	25
South	2550	25	25	25
South	2525.*	25	25	25
South	2500	25	25	25
South	2475.*	25	25	25
South	2450	25	25	25
South	2425.*	25	25	25
South	2400	25	25	25
South	2375.*	25	25	25
South	2350	25	25	25
South	2325.*	25	25	25
South	2300	25	25	25
South	2275.*	25	25	25
South	2250	25	25	25
South	2225.*	25	25	25
South	2200	25	25	25
South	2175.*	25	25	25
South	2150	25	25	25
South	2125.*	25	25	25
South	2100	37.5	25	20
South	2075.*	37.5	25	20
South	2050	40.01	25	20
South	2025.*	40.01	25	20
South	2000	0	0	0
North	2550	25	25	25
North	2525.*	25	25	25
North	2500	25	25	25
North	2475.*	25	25	25
North	2450	25	25	25
North	2425.*	25	25	25
North	2400	25	25	25
North	2375.*	25	25	25
North	2350	25	25	25
North	2325.*	25	25	25
North	2300	25	25	25
North	2275.*	25	25	25
North	2250	25	25	25
North	2225.*	25	25	25
North	2200	25	25	25
North	2175.*	25	25	25
North	2150	22.5	25	32.5
North	2125.*	22.5	25	32.5
North	2100	20	25	30
North	2075.*	20	25	30
North	2050	17.5	25	40
North	2025.*	17.5	25	40
North	2000	0	0	0
East System	2000	25	25	25
East System	1975.*	25	25	25
East System	1950	24.57	24.57	24.57
East System	1925.42*	24.57	24.57	24.57
East System	1900.85*	24.57	24.57	24.57
East System	1876.28*	24.57	24.57	24.57
East System	1851.71*	24.57	24.57	24.57
East System	1827.14*	24.57	24.57	24.57
East System	1802.57*	24.57	24.57	24.57
East System	1778	25	25	25
East System	1753.*	25	25	25
East System	1728.*	25	25	25
East System	1703.*	25	25	25
East System	1678.*	25	25	25
East System	1653.*	25	25	25
East System	1628.*	25	25	25
East System	1603.*	25	25	25
East System	1578	0	0	0

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS  
River: Collector System

Reach	River Sta.	Contr.	Expan.
South	2600	.1	.3

South	2575.*	.1	.3
South	2550	.1	.3
South	2525.*	.1	.3
South	2500	.1	.3
South	2475.*	.1	.3
South	2450	.1	.3
South	2425.*	.1	.3
South	2400	.1	.3
South	2375.*	.1	.3
South	2350	.1	.3
South	2325.*	.1	.3
South	2300	.1	.3
South	2275.*	.1	.3
South	2250	.1	.3
South	2225.*	.1	.3
South	2200	.1	.3
South	2175.*	.1	.3
South	2150	.1	.3
South	2125.*	.1	.3
South	2100	.1	.3
South	2075.*	.1	.3
South	2050	.1	.3
South	2025.*	.1	.3
South	2000	.1	.3
North	2550	.1	.3
North	2525.*	.1	.3
North	2500	.1	.3
North	2475.*	.1	.3
North	2450	.1	.3
North	2425.*	.1	.3
North	2400	.1	.3
North	2375.*	.1	.3
North	2350	.1	.3
North	2325.*	.1	.3
North	2300	.1	.3
North	2275.*	.1	.3
North	2250	.1	.3
North	2225.*	.1	.3
North	2200	.1	.3
North	2175.*	.1	.3
North	2150	.1	.3
North	2125.*	.1	.3
North	2100	.1	.3
North	2075.*	.1	.3
North	2050	.1	.3
North	2025.*	.1	.3
North	2000	.1	.3
East System	2000	.1	.3
East System	1975.*	.1	.3
East System	1950	.1	.3
East System	1925.42*	.1	.3
East System	1900.85*	.1	.3
East System	1876.28*	.1	.3
East System	1851.71*	.1	.3
East System	1827.14*	.1	.3
East System	1802.57*	.1	.3
East System	1778	.1	.3
East System	1753.*	.1	.3
East System	1728.*	.1	.3
East System	1703.*	.1	.3
East System	1678.*	.1	.3
East System	1653.*	.1	.3
East System	1628.*	.1	.3
East System	1603.*	.1	.3
East System	1578	.1	.3

*FREEBOARD  
TABLE*

# FREEBOARD TABLE NORTH BOX

Reach	River Sta	Q Total (cfs)	Min Ch El (ft)	W.S. Elev (ft)	Depth (ft)	E.G. Elev (ft)	Vel Chnl (ft/s)	Froude #	Chl Freeboard	Flow Type	Minimum Top of Channel (ft)
South	2600	44	63.3	64.4	1.1	64.86	5.69	0.96	1.00 ft	Sub	65.4
South	2550	44	63.04	63.96	0.92	64.02	2.06	0.38	1.00 ft	Sub	65.0
South	2500	44	60.6	64	3.4	64	0.49	0.05	1.00 ft	Sub	65.0
South	2450	44	59.5	64	4.5	64	0.44	0.04	1.00 ft	Sub	65.0
South	2400	44	58.8	64	5.2	64	0.36	0.03	1.00 ft	Sub	65.0
South	2350	44	58.1	64	5.9	64	0.3	0.02	1.04 ft	Sub	65.0
South	2300	132	57.4	63.99	6.59	64	0.75	0.05	1.17 ft	Sub	65.2
South	2250	296	56.7	63.97	7.27	64	1.51	0.1	1.32 ft	Sub	65.3
South	2200	576	56	63.89	7.89	63.99	2.62	0.16	1.47 ft	Sub	65.4
South	2150	991	55.3	63.72	8.42	63.97	4.16	0.25	1.60 ft	Sub	65.3
South	2100	1528	54.5	63.46	8.96	63.93	5.75	0.34	1.75 ft	Sub	65.2
South	2050	2496	53.8	63.1	9.3	63.84	7.45	0.43	1.84 ft	Sub	64.9
South	2000	2496	53.4	61.95	8.55	63.7	10.62	0.64	1.64 ft	Sub	63.6
North	2550	274	63.5	65.39	1.89	66.06	7.23	0.93	1.00 ft	Sub	66.4
North	2500	274	60.8	61.77	0.97	65.28	15.38	2.76	1.06 ft	Super	62.8
North	2450	274	59.1	63.99	4.89	64.07	2.27	0.18	1.00 ft	Sub	65.0
North	2400	274	58.7	64.01	5.31	64.06	1.81	0.14	1.00 ft	Sub	65.0
North	2350	274	57.8	64.02	6.22	64.05	1.38	0.1	1.10 ft	Sub	65.1
North	2300	454	57.1	63.99	6.89	64.04	2.02	0.14	1.24 ft	Sub	65.2
North	2250	684	56.5	63.92	7.42	64.03	2.84	0.18	1.35 ft	Sub	65.3
North	2200	968	55.8	63.83	8.03	64.02	3.66	0.23	1.50 ft	Sub	65.3
North	2150	1311	55.3	63.69	8.39	64	4.65	0.28	1.59 ft	Sub	65.3
North	2100	1717	54.5	63.53	9.03	63.96	5.49	0.32	1.77 ft	Sub	65.3
North	2050	2588	53.8	63.38	9.58	63.89	6.16	0.35	1.93 ft	Sub	65.3
North	2000	2588	53.4	61.74	8.34	63.72	11.29	0.69	1.58 ft	Sub	63.3
East System	2000	5080	53.4	61.8	8.4	63.68	10.99	0.67	1.60 ft	Sub	63.4
East System	1950	5080	53.2	60.02	6.82	63.47	14.89	1	1.42 ft	Super	61.4
East System	1778	5080	52.29	58.72	6.43	62.6	15.81	1.1	1.40 ft	Super	60.1
East System	1578	5080	50.79	57.63	6.84	61.06	14.85	1	1.42 ft	Super	59.1

Supercritical Flow (Froude >1)

$$F_b = 1 + 0.025 * V * d^{(1/3)}$$

Subcritical Flow (Froude <1)

$$F_b = .5 + ((V^2)/2g)$$

*OUTLET SYSTEM  
HEC-RAS MODEL*

HEC-RAS Plan: Plan 01 River: North Box outlet Reach: 1

Reach	River Sta	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
1	690	5080.00	46.00	50.65	51.80	55.17	0.003439	17.06	297.73	64.00	1.39
1	675	5080.00	45.90	48.52	50.38	54.83	0.042696	20.36	257.84	116.82	2.22
1	652	5080.00	46.20	52.64	49.68	52.68	0.000130	2.01	3187.83	783.00	0.14
1	632	5080.00	47.00	52.64	49.26	52.67	0.000097	1.66	3630.15	833.00	0.12
1	256	5834.00	50.37	51.94	51.94	52.60	0.023201	6.79	915.85	680.25	0.96
1	239	6470.00	44.37	48.32		48.44	0.008406	3.53	2399.09	1317.79	0.55
1	238	6470.00	42.37	44.30	44.26	44.89	0.021798	7.56	1071.17	833.06	0.96

HEC-RAS September 1998 Version 2.2  
 U.S. Army Corp of Engineers  
 Hydrologic Engineering Center  
 609 Second Street, Suite D  
 Davis, California 95616-4687  
 (916) 756-1104

```

X   X   XXXXXX   XXXX   XXXX   XX   XXXX
X   X   X   X   X   X   X   X   X
X   X   X   X   X   X   X   X   X
XXXXXXX XXXX   X   XXX XXXX   XXXXXX   XXXX
X   X   X   X   X   X   X   X   X
X   X   X   X   X   X   X   X   X
X   X   XXXXXX   XXXX   X   X   X   X   XXXXX

```

## PROJECT DATA

Project Title: Northbox Outlet  
 Project File : nbox.prj  
 Run Date and Time: 3/19/2002 9:27:29 AM

Project in English units

## PLAN DATA

Plan Title: Plan 01  
 Plan File : C:\HEC\RAS\nbox.p01

Geometry Title: Geom 01  
 Geometry File : C:\HEC\RAS\nbox.g01

Flow Title : Flow 01  
 Flow File : C:\HEC\RAS\nbox.f01

## Plan Summary Information:

Number of:	Cross Sections =	21	Multitple Openings =	0
	Culverts =	0	Inline Weirs =	0
	Bridges =	0		

## Computational Information

Water surface calculation tolerance =	0.01
Critical depth calculation tolerance =	0.01
Maximum number of iterations =	20
Maximum difference tolerance =	0.3
Flow tolerance factor =	0.001

## Computation Options

Critical depth computed only where necessary
Conveyance Calculation Method: At breaks in n values only
Friction Slope Method: Average Conveyance
Computational Flow Regime: Mixed Flow

## FLOW DATA

Flow Title: Flow 01  
 Flow File : C:\HEC\RAS\nbox.f01

## Flow Data (cfs)

River	Reach	RS	PF 1
North Box outlet1		690	5080
North Box outlet1		256	5834
North Box outlet1		239	6470

## Boundary Conditions

River	Reach	Profile	Upstream	Downstre
North Box outlet1		PF 1	Known WS = 50.652	Known WS =



## GEOMETRY DATA

Geometry Title: Geom 01

Geometry File : C:\HEC\RAS\nbox.g01

CROSS SECTION RIVER: North Box outlet  
 REACH: 1 RS: 690

## INPUT

Description:

Station Elevation Data		num= 4					
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
968	51	968	46	1032	46	1032	51

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
968	.013	968	.013	1032	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	968	1032		15	15		.1	.3

CROSS SECTION RIVER: North Box outlet  
 REACH: 1 RS: 675

## INPUT

Description:

Station Elevation Data		num= 6							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
910	51	953	48	953	45.9	1046	45.9	1046	47
1100	52								

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
910	.028	953	.028	1046	.028

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	953	1046		23.02	23.02		.1	.3

CROSS SECTION RIVER: North Box outlet  
 REACH: 1 RS: 652

## INPUT

Description:

Station Elevation Data		num= 7							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
677	51	885	48.5	922	47	1000	46.2	1084	47
1134	48.5	1460	49.5						

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
677	.028	922	.028	1084	.028

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	922	1084		20	20		.1	.3

Ineffective Flow		num= 2			
Sta L	Sta R	Elev	Sta L	Sta R	Elev
677	885	48.5	1134	1460	48.5

CROSS SECTION RIVER: North Box outlet  
 REACH: 1 RS: 632

## INPUT

Description:

Station Elevation Data		num= 8							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
677	50	887	48.1	867	48.1	960	47	1000	47
1019	47	1157	48.1	1510	50				

Manning's n Values		num= 3			
Sta	n Val	Sta	n Val	Sta	n Val
677	.028	960	.028	1019	.028

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff	Contr.	Expan.
	960	1019		20	20		.1	.3

Ineffective Flow		num= 2			

Sta L	Sta R	Elev	Sta L	Sta R	Elev
677	867	48.1	1157	1510	48.1

CROSS SECTION RIVER: North Box outlet  
REACH: 1 RS: 256

## INPUT

Description:

Station	Elevation	Data	num=	4
Sta	Elev	Sta	Elev	Sta
750	51.77	910.51	50.37	1380

Manning's n	Values	num=	3
Sta	n Val	Sta	n Val
750	.045	910.51	.045

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	910.51	1380		45	45	45		.1	.3

CROSS SECTION RIVER: North Box outlet  
REACH: 1 RS: 251.75\*

## INPUT

Description:

Station	Elevation	Data	num=	16
Sta	Elev	Sta	Elev	Sta
747.5	51.42	788.76	50.32	809.04
1043.31	49.68	1054.5	49.66	1057.29
1174.38	49.37	1178.38	48.87	1530.5
1695.05	53.32			

Manning's n	Values	num=	4
Sta	n Val	Sta	n Val
747.5	.054	1169.88	.045

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	1169.88	1599		45	45	45		.1	.3

CROSS SECTION RIVER: North Box outlet  
REACH: 1 RS: 247.5\*

## INPUT

Description:

Station	Elevation	Data	num=	16
Sta	Elev	Sta	Elev	Sta
745	51.07	811.84	49	844.69
1224.21	48.58	1242.33	48.56	1246.86
1438.26	48.37	1446.26	47.37	1681
1882.03	52.27			

Manning's n	Values	num=	4
Sta	n Val	Sta	n Val
745	.062	1429.26	.045

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	1429.26	1818		45	45	45		.1	.3

CROSS SECTION RIVER: North Box outlet  
REACH: 1 RS: 243.25\*

## INPUT

Description:

Station	Elevation	Data	num=	16
Sta	Elev	Sta	Elev	Sta
742.5	50.72	834.92	47.69	880.35
1405.1	47.47	1430.17	47.47	1436.43
1702.13	47.37	1714.13	45.87	1831.5
2069.02	51.22			

Manning's n	Values	num=	4
Sta	n Val	Sta	n Val
742.5	.071	1688.63	.045

Bank Sta:	Left	Right	Lengths:	Left	Channel	Right	Coeff	Contr.	Expan.
	1688.63	2037		45	45	45		.1	.3

CROSS SECTION RIVER: North Box outlet  
REACH: 1 RS: 239

## INPUT

## Description:

Station Elevation Data		num= 14		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
740	50.37	858	46.37	916	46.37	1473	46.37	1486	46.37		
1586	46.37	1618	46.37	1626	46.37	1948	46.37	1949	50.37		
1966	46.37	1982	44.37	1992	46.37	2256	50.17				

## Manning's n Values

num= 3

Sta	n Val	Sta	n Val	Sta	n Val
740	.08	1948	.045	2256	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1948	2256		31.76	46.22	.1	.3

## CROSS SECTION

RIVER: North Box outlet

REACH: 1

RS: 238.916\*

## INPUT

## Description:

Station Elevation Data		num= 22		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
725	50.04	814.22	46.97	837.69	46.2	893.08	46.19	1015.49	46.16		
1425.03	46.05	1437.45	46.05	1488.58	46.04	1532.95	46.04	1563.51	46.04		
1571.15	46.04	1878.67	46.04	1879.58	49.7	1895.17	46.04	1909.83	44.2		
1914.42	44.2	1915.25	44.2	1918.42	44.2	1927.58	46.04	2169.58	49.52		
2189.5	49.69	2232.83	49.77								

## Manning's n Values

num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
725	.077	1878.67	.045	2169.58	.04	2232.83	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1878.67	2169.58		31.76	46.22	.1	.3

## CROSS SECTION

RIVER: North Box outlet

REACH: 1

RS: 238.833\*

## INPUT

## Description:

Station Elevation Data		num= 22		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
710	49.7	795.02	46.73	817.39	46.03	870.17	46	986.81	45.95		
1377.06	45.74	1388.89	45.73	1437.62	45.7	1479.9	45.7	1509.02	45.7		
1516.3	45.7	1809.33	45.7	1810.17	49.04	1824.33	45.7	1837.67	44.04		
1846.83	44.04	1848.5	44.04	1854.83	44.04	1863.17	45.7	2083.17	48.87		
2123	49.2	2209.67	49.37								

## Manning's n Values

num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
710	.074	1809.33	.045	2083.17	.04	2209.67	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1809.33	2083.17		31.76	46.22	.1	.3

## CROSS SECTION

RIVER: North Box outlet

REACH: 1

RS: 238.75\*

## INPUT

## Description:

Station Elevation Data		num= 22		Sta		Elev		Sta		Elev	
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
695	49.37	775.82	46.49	797.08	45.86	847.25	45.82	958.13	45.74		
1329.09	45.42	1340.34	45.41	1386.65	45.37	1426.85	45.37	1454.53	45.37		
1461.45	45.37	1740	45.37	1740.75	48.37	1753.5	45.37	1765.5	43.87		
1779.25	43.87	1781.75	43.87	1791.25	43.87	1798.75	45.37	1996.75	48.22		
2056.5	48.72	2186.5	48.97								

## Manning's n Values

num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
695	.071	1740	.045	1996.75	.04	2186.5	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1740	1996.75		31.76	46.22	.1	.3

## CROSS SECTION

RIVER: North Box outlet

REACH: 1

RS: 238.666\*

## INPUT

## Description:

Station Elevation Data									
num= 22									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
680	49.04	756.62	46.26	776.77	45.68	824.34	45.64	929.45	45.54
1281.12	45.1	1291.79	45.09	1335.69	45.04	1373.79	45.04	1400.04	45.04
1406.6	45.04	1670.67	45.04	1671.33	47.7	1682.67	45.04	1693.33	43.7
1711.67	43.7	1715	43.7	1727.67	43.7	1734.33	45.04	1910.33	47.57
1990	48.24	2163.33	48.57						

## Manning's n Values

num= 4									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
680	.068	1670.67	.045	1910.33	.04	2163.33	.04		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1670.67	1910.33		31.76	46.22	45.24	.1 .3

## CROSS SECTION

RIVER: North Box outlet

REACH: 1

RS: 238.583\*

## INPUT

## Description:

Station Elevation Data									
num= 22									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
665	48.7	737.41	46.02	756.46	45.51	801.42	45.46	900.77	45.33
1233.16	44.79	1243.23	44.77	1284.73	44.7	1320.74	44.7	1345.55	44.7
1351.75	44.7	1601.33	44.7	1601.92	47.04	1611.83	44.7	1621.17	43.54
1644.08	43.54	1648.25	43.54	1664.08	43.54	1669.92	44.7	1823.92	46.92
1923.5	47.75	2140.17	48.17						

## Manning's n Values

num= 4									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
665	.065	1601.33	.045	1823.92	.04	2140.17	.04		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1601.33	1823.92		31.76	46.22	45.24	.1 .3

## CROSS SECTION

RIVER: North Box outlet

REACH: 1

RS: 238.5\*

## INPUT

## Description:

Station Elevation Data									
num= 22									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
650	48.37	718.21	45.79	736.16	45.34	778.5	45.27	872.09	45.12
1185.19	44.47	1194.68	44.45	1233.77	44.37	1267.69	44.37	1291.06	44.37
1296.9	44.37	1532	44.37	1532.5	46.37	1541	44.37	1549	43.37
1576.5	43.37	1581.5	43.37	1600.5	43.37	1605.5	44.37	1737.5	46.27
1857	47.27	2117	47.77						

## Manning's n Values

num= 4									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
650	.062	1532	.045	1737.5	.04	2117	.04		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1532	1737.5		31.76	46.22	45.24	.1 .3

## CROSS SECTION

RIVER: North Box outlet

REACH: 1

RS: 238.416\*

## INPUT

## Description:

Station Elevation Data									
num= 22									
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
635	48.04	699.01	45.55	715.85	45.17	755.59	45.09	843.41	44.91
1137.22	44.15	1146.13	44.13	1182.81	44.04	1214.64	44.04	1236.57	44.04
1242.05	44.04	1462.67	44.04	1463.08	45.7	1470.17	44.04	1476.83	43.2
1508.92	43.2	1514.75	43.2	1536.92	43.2	1541.08	44.04	1651.08	45.62
1790.5	46.79	2093.83	47.37						

## Manning's n Values

num= 4									
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
635	.06	1462.67	.045	1651.08	.04	2093.83	.04		

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1462.67	1651.08		31.76	46.22	45.24	.1 .3

## CROSS SECTION

RIVER: North Box outlet

REACH: 1

RS: 238.333\*

## INPUT

## Description:

Station Elevation Data			num= 22			Elev Sta Elev Sta Elev Sta		
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta
620	47.7	679.81	45.31	695.54	45	732.67	44.91	814.72
1089.25	43.84	1097.57	43.81	1131.85	43.7	1161.59	43.7	1182.08
1187.2	43.7	1393.33	43.7	1393.67	45.04	1399.33	43.7	1404.67
1441.33	43.04	1448	43.04	1473.33	43.04	1476.67	43.7	1564.67
1724	46.3	2070.67	46.97					44.97

Manning's n Values			num= 4			Sta n Val Sta n Val Sta n Val		
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta
620	.057	1393.33	.045	1564.67	.04	2070.67	.04	

Bank Sta: Left Right		Lengths: Left Channel Right		Coeff Contr. Expan.	
1393.33	1564.67	31.76	46.22	45.24	.1 .3

CROSS SECTION  
REACH: 1

RIVER: North Box outlet  
RS: 238.25\*

## INPUT

## Description:

Station Elevation Data			num= 22			Elev Sta Elev Sta Elev Sta		
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta
605	47.37	660.61	45.08	675.23	44.83	709.76	44.72	786.04
1041.28	43.52	1049.02	43.49	1080.88	43.37	1108.54	43.37	1127.58
1132.35	43.37	1324	43.37	1324.25	44.37	1328.5	43.37	1332.5
1373.75	42.87	1381.25	42.87	1409.75	42.87	1412.25	43.37	1478.25
1657.5	45.82	2047.5	46.57					44.32

Manning's n Values			num= 4			Sta n Val Sta n Val Sta n Val		
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta
605	.054	1324	.045	1478.25	.04	2047.5	.04	

Bank Sta: Left Right		Lengths: Left Channel Right		Coeff Contr. Expan.	
1324	1478.25	31.76	46.22	45.24	.1 .3

CROSS SECTION  
REACH: 1

RIVER: North Box outlet  
RS: 238.166\*

## INPUT

## Description:

Station Elevation Data			num= 22			Elev Sta Elev Sta Elev Sta		
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta
590	47.04	641.4	44.84	654.93	44.65	686.84	44.54	757.36
993.31	43.2	1000.46	43.17	1029.92	43.04	1055.49	43.04	1073.09
1077.5	43.04	1254.67	43.04	1254.83	43.7	1257.67	43.04	1260.33
1306.17	42.7	1314.5	42.7	1346.17	42.7	1347.83	43.04	1391.83
1591	45.34	2024.33	46.17					43.67

Manning's n Values			num= 4			Sta n Val Sta n Val Sta n Val		
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta
590	.051	1254.67	.045	1391.83	.04	2024.33	.04	

Bank Sta: Left Right		Lengths: Left Channel Right		Coeff Contr. Expan.	
1254.67	1391.83	31.76	46.22	45.24	.1 .3

CROSS SECTION  
REACH: 1

RIVER: North Box outlet  
RS: 238.083\*

## INPUT

## Description:

Station Elevation Data			num= 22			Elev Sta Elev Sta Elev Sta		
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta
575	46.7	622.2	44.61	634.62	44.48	663.92	44.36	728.68
945.34	42.89	951.91	42.85	978.96	42.7	1002.44	42.7	1018.6
1022.65	42.7	1185.33	42.7	1185.42	43.04	1186.83	42.7	1188.17
1238.58	42.54	1247.75	42.54	1282.58	42.54	1283.42	42.7	1305.42
1524.5	44.85	2001.17	45.77					43.02

Manning's n Values			num= 4			Sta n Val Sta n Val Sta n Val		
Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val	Sta
575	.048	1185.33	.045	1305.42	.04	2001.17	.04	

Bank Sta: Left Right		Lengths: Left Channel Right		Coeff Contr. Expan.	
1185.33	1305.42	31.76	46.22	45.24	.1 .3

CROSS SECTION  
REACH: 1

RIVER: North Box outlet  
RS: 238

## INPUT

## Description:

Station Elevation Data		num= 10							
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
560	46.37	603	44.37	700	43.87	928	42.37	1116	42.37
1171	42.37	1181	42.37	1219	42.37	1458	44.37	1978	45.37

## Manning's n Values

num= 3					
Sta	n Val	Sta	n Val	Sta	n Val
560	.045	1116	.045	1219	.04

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1116	1219		0	0	.1	.3

## SUMMARY OF MANNING'S N VALUES

River:North Box outlet

Reach	River Sta.	n1	n2	n3	n4
1	690	.013	.013	.013	
1	675	.028	.028	.028	
1	652	.028	.028	.028	
1	632	.028	.028	.028	
1	256	.045	.045	.04	
1	251.75*	.054	.045	.04	.04
1	247.5*	.062	.045	.04	.04
1	243.25*	.071	.045	.04	.04
1	239	.08	.045	.04	
1	238.916*	.077	.045	.04	.04
1	238.833*	.074	.045	.04	.04
1	238.75*	.071	.045	.04	.04
1	238.666*	.068	.045	.04	.04
1	238.583*	.065	.045	.04	.04
1	238.5*	.062	.045	.04	.04
1	238.416*	.06	.045	.04	.04
1	238.333*	.057	.045	.04	.04
1	238.25*	.054	.045	.04	.04
1	238.166*	.051	.045	.04	.04
1	238.083*	.048	.045	.04	.04
1	238	.045	.045	.04	

## SUMMARY OF REACH LENGTHS

River: North Box outlet

Reach	River Sta.	Left	Channel	Right
1	690	15	15	15
1	675	23.02	23.02	23.02
1	652	20	20	20
1	632	20	20	20
1	256	45	45	45
1	251.75*	45	45	45
1	247.5*	45	45	45
1	243.25*	45	45	45
1	239	31.76	46.22	45.24
1	238.916*	31.76	46.22	45.24
1	238.833*	31.76	46.22	45.24
1	238.75*	31.76	46.22	45.24
1	238.666*	31.76	46.22	45.24
1	238.583*	31.76	46.22	45.24
1	238.5*	31.76	46.22	45.24
1	238.416*	31.76	46.22	45.24
1	238.333*	31.76	46.22	45.24
1	238.25*	31.76	46.22	45.24
1	238.166*	31.76	46.22	45.24
1	238.083*	31.76	46.22	45.24
1	238	0	0	0

SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS  
 River: North Box outlet

Reach	River Sta.	Contr.	Expan.
1	690	.1	.3
1	675	.1	.3
1	652	.1	.3
1	632	.1	.3
1	256	.1	.3
1	251.75*	.1	.3
1	247.5*	.1	.3
1	243.25*	.1	.3
1	239	.1	.3
1	238.916*	.1	.3
1	238.833*	.1	.3
1	238.75*	.1	.3
1	238.666*	.1	.3
1	238.583*	.1	.3
1	238.5*	.1	.3
1	238.416*	.1	.3
1	238.333*	.1	.3
1	238.25*	.1	.3
1	238.166*	.1	.3
1	238.083*	.1	.3
1	238	.1	.3

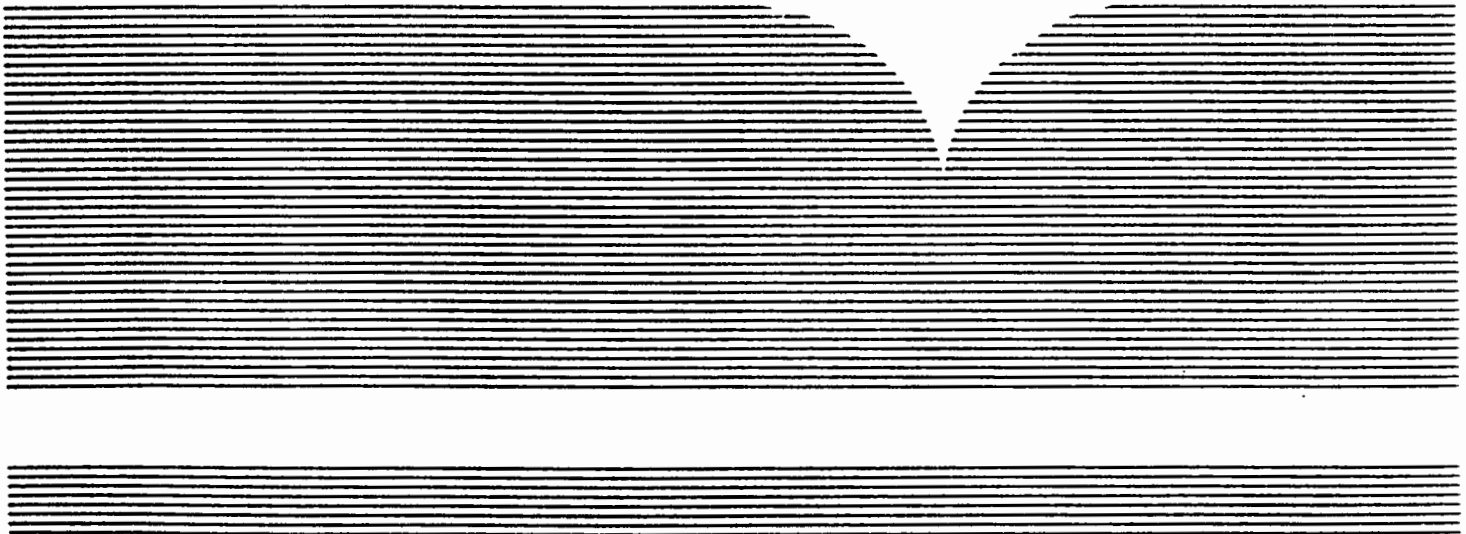


PB89-218424

# DESIGN OF RIPRAP REVETMENT

SUTRON CORPORATION  
HERNDON, VA

MAR 89



U.S. DEPARTMENT OF COMMERCE  
National Technical Information Service

**NTIS**



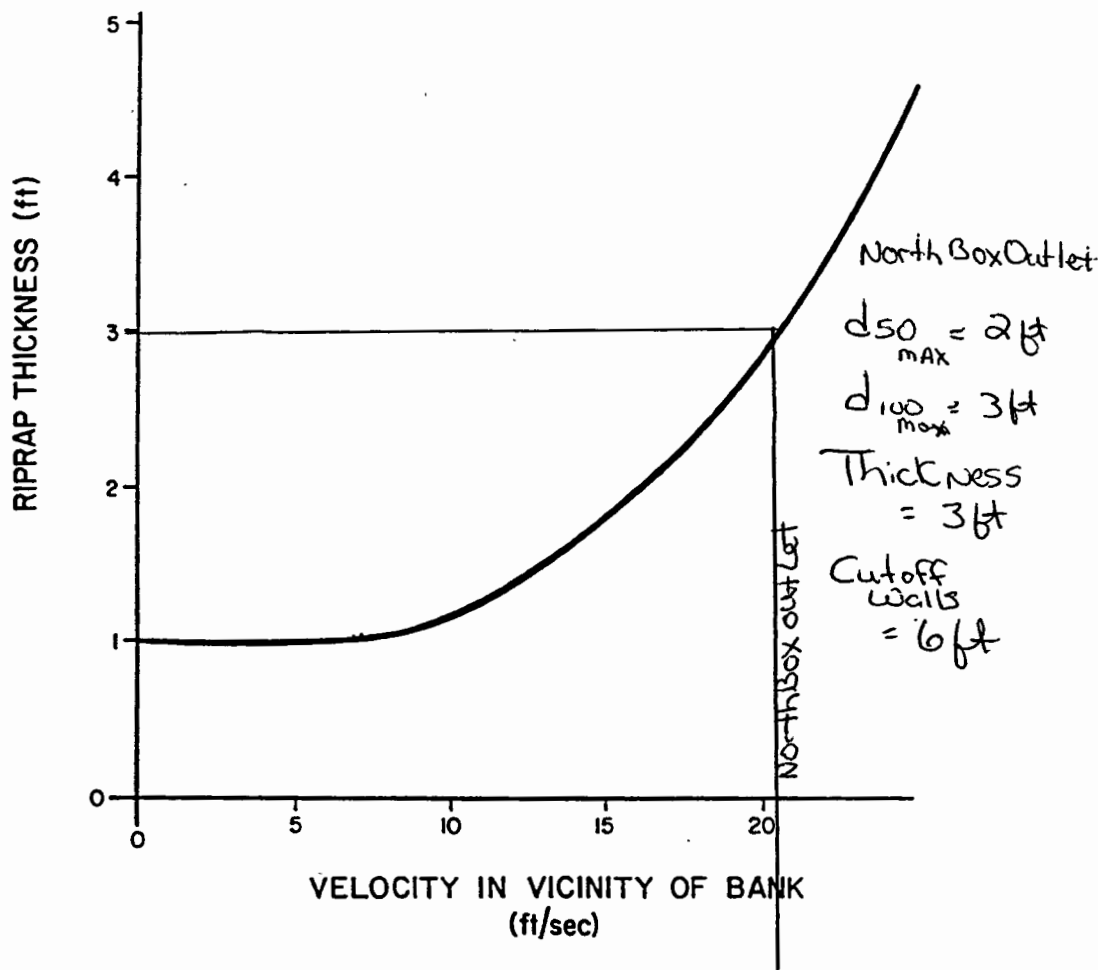


Figure 57. Required blanket thickness as a function of flow velocity.

**Rock Grading:** Table 6 provides guidelines for rock gradation in grouted riprap installations. Six size classes are listed.

**Rock Quality:** Rock used in grouted rock slope-protection is usually the same as that used in ordinary rock slope-protection. However, the specifications for specific gravity and hardness may be lowered if necessary as the rocks are protected by the surrounding grout.

In addition, the rock used in grouted riprap installations should be free of fines in order that penetration of grout may be achieved.

**Grout Quality and Characteristics:** Grout should consist of good strength concrete using a maximum aggregate size of 3/4 in and a slump of 3 to 4 in (7.6 to 10.2 cm). Sand mixes may be used where roughness of the grout surface is unnecessary, provided sufficient cement is added to give good strength and workability.

The volume of grout required will be that necessary to provide penetration to the depths shown in table 6.

The finished grout should leave face stones exposed for one-fourth to one-third their depth and the surface of the grout should expose a matrix of coarse aggregate.

## 6.3 GROUTED ROCK

Grouted rock revetment consists of rock slope-protection having voids filled with concrete grout to form a monolithic armor. See section 2.5 for additional descriptive information and general performance characteristics for grouted rock. Sample specifications for components of grouted rock revetments are provided in appendix A.

### 6.3.1 Design Guidelines for Grouted Rock

Components of grouted rock riprap design include layout of a general scheme or concept, bank preparation, bank slope, rock size and blanket thickness, rock grading, rock quality, grout quality, edge treatment, filter design, and pressure relief.

**General:** Grouted riprap designs are rigid monolithic bank protection schemes. When complete, they form a continuous surface. A typical grouted riprap section is shown in figure 56.

Grouted riprap should extend from below the anticipated channel bed scour depth to the design high water level plus additional height for freeboard (see section 3.6.2). The longitudinal extent of protection should be as described in section 3.6.1.

During the design phase for a grouted riprap revetment, special attention needs to be paid to edge treatment, foundation design, and mechanisms for hydrostatic pressure relief. Each of these items is discussed below.

**Bank and Foundation Preparation:** The bank should be prepared by first clearing all trees and debris from the bank, and grading the bank surface to the desired slope. In general, the graded surface should not deviate from the specified slope line by more than six in (15.2 cm). However, local depressions larger than this can be accommodated since initial placement of filter material and/or rock for the revetment will fill these depressions.

Since grouted riprap is rigid but not extremely strong, support by the embankment must be maintained. To form a firm foundation, it is recommended that the bank surface be tamped or lightly compacted. Care must be taken during bank compaction to maintain a soil permeability similar to that of the natural, undisturbed bank material. The foundation for the grouted riprap revetment should have a bearing capacity sufficient to support either the dry weight of the revetment alone, or the submerged weight of the revetment plus the weight of the water in the wedge above the revetment for design conditions, whichever is greater.

Any large boulders or debris found buried near the edges of the revetment should be removed.

**Bank Slope:** Bank slopes for grouted riprap revetments should not exceed 1.5:1.

**Rock Size and Blanket Thickness:** Blanket thickness and rock size requirements for grouted riprap installations are interrelated. Figure 57 illustrates a relationship between the design velocity and the required riprap blanket thickness for grouted riprap designs. The median rock size in the revetment should not exceed 0.67 times the blanket thickness. The largest rock used in the revetment should not exceed the blanket thickness.

T1 Green Park Holdings, LLC  
 T2 Culvert from I-15 South to Main South Box, 482cfs  
 T3 File: gpsb588 By Alex Kingston  
 SO 1000.0002265.540 1 2275.735  
 1147.4402266.500 1 .013 80.454 .000 2  
 1950.5702269.710 1 .013 0.000 .000 0  
 R 2100.0002270.310 1 .013 0.000 .000 0  
 R 2118.6302270.880 1 .013 -55.024 .000 1  
 TS 2230.4502274.310 2 .013 -36.188  
 R 2337.9302277.600 2 .013 0.000 .000 0  
 R 2530.8202279.460 2 .013 0.000 .000 0  
 WE 2530.8202279.460 3 .500  
 R 2560.8202279.960 3 .025 0.000 .000 0  
 SH 2560.8202279.960 3 2283.960  
 CD 2 3 1 .500 4.000 24.500 .000 .000 .00  
 CD 3 1 0 .000 6.000 30.000 .330 .330 .00  
 CD 1 6 0 .000 .00 .00 .00 .00 .00 .00 .00 .00 .00  
 PTS 121 .000 .000 10.000 .000 10.000 4.000 9.820 4.530 9.490 5.000  
 PTS 9.000 5.390 8.500 5.590 8.000 5.730 7.520 5.830 7.000 5.930  
 PTS 6.000 6.050 5.000 6.090 4.000 6.050 3.000 5.930 2.480 5.830  
 PTS 2.000 5.730 1.500 5.590 1.000 5.390 .510 5.000 .180 4.530  
 PTS .000 4.000  
 Q 482.000 .0

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PAGE 1

Date: 3-29-2002 Time: 11: 1:58

Green Park Holdings, LLC

Culvert from I-15 South to Main South Box, 482cfs

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Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.	Base Wt I.D.	No Wth ZL	Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1000.000	2265.540	10.195	2275.735	482.00	8.50	1.12	2276.86	.00	4.15	.00			1	0 .0
147.440	.0065					.0023	.34	.00	.00	3.304	.013			IR-COV
1147.440	2266.500	9.904	2276.404	482.00	8.50	1.12	2277.53	.00	4.15	.00			1	0 .0
803.130	.0040					.0023	1.87	9.90	.00	3.939	.013			IR-COV
1950.570	2269.710	8.568	2278.278	482.00	8.50	1.12	2279.40	.00	4.15	.00			1	0 .0
149.430	.0040					.0023	.35	8.57	.00	3.932	.013			IR-COV
2100.000	2270.310	8.316	2278.626	482.00	8.50	1.12	2279.75	.00	4.15	.00			1	0 .0
18.630	.0306					.0023	.04	.00	.00	1.932	.013			IR-COV
2118.630	2270.880	8.022	2278.901	482.00	8.50	1.12	2280.02	.00	4.15	.00			1	0 .0
TRANS STR	.0307					.0023	.26	.00	.00		.013			IR-COV
2230.450	2274.310	5.601	2279.911	482.00	5.02	.39	2280.30	.00	2.32	24.50	4.000	24.500	.00	1 .5
54.305	.0306					.0011	.06	5.60	.45	1.07	.013	.00	.00	BOX
2284.755	2275.972	4.000	2279.972	482.00	5.02	.39	2280.36	.00	2.32	24.50	4.000	24.500	.00	1 .5
4.903	.0306					.0006	.00	4.00	.45	1.07	.013	.00	.00	BOX
2289.658	2276.123	3.813	2279.936	482.00	5.27	.43	2280.37	.00	2.32	24.50	4.000	24.500	.00	1 .5
2.391	.0306					.0007	.00	3.81	.48	1.07	.013	.00	.00	BOX

----- WARNING - Flow depth near top of box conduit -----

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 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 2

Date: 3-29-2002 Time: 11: 1:58

Green Park Holdings, LLC  
 Culvert from I-15 South to Main South Box, 482cfs  
 File: gpsb588 By Alex Kingston

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt I.D.	No Wth ZL	Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
2292.049	2276.196	3.721	2279.917	482.00	5.40	.45	2280.37	.00	2.32	24.50	4.000	24.500	.00	1 .5
HYDRAULIC JUMP														
----- WARNING - Flow depth near top of box conduit -----														
2292.049	2276.196	1.329	2277.524	482.00	15.12	3.55	2281.07	.00	2.32	24.50	4.000	24.500	.00	1 .5
.542	.0306					.0155	.01	1.33	2.33	1.07	.013	.00	.00	BOX
2292.590	2276.212	1.334	2277.546	482.00	15.05	3.52	2281.06	.00	2.32	24.50	4.000	24.500	.00	1 .5
15.689	.0306					.0144	.23	1.33	2.32	1.07	.013	.00	.00	BOX
2308.279	2276.692	1.399	2278.092	482.00	14.35	3.20	2281.29	.00	2.32	24.50	4.000	24.500	.00	1 .5
12.224	.0306					.0124	.15	1.40	2.16	1.07	.013	.00	.00	BOX
2320.503	2277.067	1.468	2278.534	482.00	13.68	2.91	2281.44	.00	2.32	24.50	4.000	24.500	.00	1 .5
9.689	.0306					.0107	.10	1.47	2.01	1.07	.013	.00	.00	BOX
2330.192	2277.363	1.539	2278.903	482.00	13.05	2.64	2281.55	.00	2.32	24.50	4.000	24.500	.00	1 .5
7.738	.0306					.0093	.07	1.54	1.87	1.07	.013	.00	.00	BOX
2337.930	2277.600	1.614	2279.214	482.00	12.44	2.40	2281.62	.00	2.32	24.50	4.000	24.500	.00	1 .5
63.092	.0096					.0082	.52	1.61	1.74	1.55	.013	.00	.00	BOX
2401.022	2278.208	1.663	2279.871	482.00	12.08	2.27	2282.14	.00	2.32	24.50	4.000	24.500	.00	1 .5
53.848	.0096					.0073	.39	1.66	1.67	1.55	.013	.00	.00	BOX

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 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 3

Date: 3-29-2002 Time: 11: 1:58

Green Park Holdings, LLC  
 Culvert from I-15 South to Main South Box, 482cfs  
 File: gpsb588 By Alex Kingston

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top	Height/Dia.	Base Wt	No Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR
													Type Ch
2454.870	2278.727	1.744	2280.471	482.00	11.52	2.06	2282.53	.00	2.32	24.50	4.000	24.500	.00
30.910	.0096					.0063	.20	1.74	1.55	1.55	.013	.00	.00
2485.780	2279.025	1.829	2280.854	482.00	10.98	1.87	2282.73	.00	2.32	24.50	4.000	24.500	.00
19.485	.0096					.0055	.11	1.83	1.45	1.55	.013	.00	.00
2505.265	2279.213	1.918	2281.132	482.00	10.47	1.70	2282.83	.00	2.32	24.50	4.000	24.500	.00
12.502	.0096					.0048	.06	1.92	1.35	1.55	.013	.00	.00
2517.768	2279.334	2.012	2281.346	482.00	9.98	1.55	2282.89	.00	2.32	24.50	4.000	24.500	.00
7.692	.0096					.0041	.03	2.01	1.25	1.55	.013	.00	.00
2525.460	2279.408	2.110	2281.518	482.00	9.52	1.41	2282.93	.00	2.32	24.50	4.000	24.500	.00
4.101	.0096					.0036	.01	2.11	1.17	1.55	.013	.00	.00
2529.561	2279.448	2.213	2281.661	482.00	9.07	1.28	2282.94	.00	2.32	24.50	4.000	24.500	.00
1.259	.0096					.0031	.00	2.21	1.09	1.55	.013	.00	.00
2530.820	2279.460	2.322	2281.782	482.00	8.65	1.16	2282.94	.00	2.32	24.50	4.000	24.500	.00
WALL ENTRANCE													
2530.820	2279.460	3.444	2282.904	482.00	4.49	.31	2283.22	.00	1.99	32.27	6.000	30.000	.33
8.147	.0167					.0015	.01	3.44	.43	1.60	.025	.00	.33
2538.967	2279.596	3.289	2282.885	482.00	4.71	.35	2283.23	.00	1.99	32.17	6.000	30.000	.33
7.615	.0167					.0017	.01	3.29	.47	1.60	.025	.00	.33

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PAGE 4

Program Package Serial Number: 1439

WATER SURFACE PROFILE LISTING

Date: 3-29-2002 Time: 11: 1:58

Green Park Holdings, LLC

Culvert from I-15 South to Main South Box, 482cfs

File: gpsb588 By Alex Kingston

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wtl	Ino Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR
													Type Ch
2546.583	2279.723	3.141	2282.864	482.00	4.94	.38	2283.24	.00	1.99	32.07	6.000	30.000	.33 0 .0
7.083	.0167					.0020	.01	3.14	.50	1.60	.025	.00	.33 TRAP
2553.665	2279.841	2.999	2282.840	482.00	5.19	.42	2283.26	.00	1.99	31.98	6.000	30.000	.33 0 .0
6.544	.0167					.0023	.02	3.00	.54	1.60	.025	.00	.33 TRAP
2560.209	2279.950	2.864	2282.814	482.00	5.44	.46	2283.27	.00	1.99	31.89	6.000	30.000	.33 0 .0
.611	.0167					.0025	.00	2.86	.57	1.60	.025	.00	.33 TRAP
2560.820	2279.960	2.852	2282.812	482.00	5.46	.46	2283.28	.00	1.99	31.88	6.000	30.000	.33 0 .0

T1 Green Park Holdings, LLC  
 T2 Parallel System to Main Culvert  
 T3 File: gpsb2543 By: Alex Kingston

0

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	1000.1002250.200	1	.028					.000											
	1050.3402250.400	1	.028					.000											
	1050.3402250.400	2																	
R	1237.8902251.150	2	.013					.000	.000	0									
R	1290.8302251.360	2	.013					-32.268	.000	0									
TS	1348.9302251.600	3	.013					-38.708											
R	2939.3702257.960	3	.013					.000	.000	6									
TS	2939.3802257.970	4	.013					.000	.000	0									
R	3007.0002260.575	4	.013					25.000											
SH	3007.0002260.575	4				2266.050													
CD	2 3 2		.500	5.000	31.000	.000	.000	.00											
CD	4 2 0		.000	14.000	20.000	.000	.000	.00											
CD	1 5 0		.000		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
CD	3 6 0		.000		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
CD	5 5 0		.000		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
CD	6 5 0		.000		.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
CD	7 4 0		.000	3.000	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00	.00
PTS	1 4	.000	5.000	10.000	.000	90.000	.000	100.000	5.000										
PTS	329	.000	.000	24.000	.000	23.750	1.900	23.500	2.660	23.000	3.700								
PTS		22.000	5.030	21.000	5.920	20.000	6.530	19.000	6.950	18.000	7.240								
PTS		17.000	7.470	16.000	7.660	15.000	7.810	14.000	7.920	13.000	7.980								
PTS		12.000	8.000	11.000	7.980	10.000	7.920	9.000	7.810	8.000	7.660								
PTS		7.000	7.470	6.000	7.240	5.000	6.950	4.000	6.530	3.000	5.920								
PTS		2.000	5.030	1.000	3.700	.500	2.660	.250	1.900										
PTS	5 5	.000	7.500	.100	.000	20.000	.000	35.000	7.500	40.000	7.600								
PTS	6 5	.000	7.500	.100	.000	15.000	.000	35.000	.000	50.000	7.500								
Q			2543.000	.0															



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PAGE 1

Program Package Serial Number: 1439

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Green Park Holdings, LLC

Parallel System to Main Culvert

File: gpsb2543 By: Alex Kingston

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top	Height/Dia.	Base Wt	No Wth
L/Elem	Ch Slope				SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1000.000	2250.200	2.063	2252.263	2543.00	14.65	3.33	2255.60	.00	3.07	88.25		1	0 .0
.100	.0000					.0314	.00	2.06	1.84	.000	.028		IR-OPEN
1000.100	2250.200	2.062	2252.262	2543.00	14.66	3.34	2255.60	.00	3.07	88.25		1	0 .0
7.294	.0040					.0338	.25	2.06	1.84	3.806	.028		IR-OPEN
1007.394	2250.229	1.979	2252.208	2543.00	15.30	3.64	2255.84	.00	3.07	87.92		1	0 .0
7.857	.0040					.0391	.31	1.98	1.96	3.806	.028		IR-OPEN
1015.251	2250.260	1.891	2252.151	2543.00	16.05	4.00	2256.15	.00	3.07	87.56		1	0 .0
7.583	.0040					.0455	.35	1.89	2.10	3.806	.028		IR-OPEN
1022.834	2250.290	1.806	2252.096	2543.00	16.83	4.40	2256.50	.00	3.07	87.22		1	0 .0
7.335	.0040					.0531	.39	1.81	2.25	3.806	.028		IR-OPEN
1030.169	2250.320	1.726	2252.046	2543.00	17.65	4.84	2256.89	.00	3.07	86.90		1	0 .0
7.014	.0040					.0619	.43	1.73	2.42	3.806	.028		IR-OPEN
1037.182	2250.347	1.648	2251.996	2543.00	18.52	5.32	2257.32	.00	3.07	86.59		1	0 .0
6.725	.0040					.0721	.49	1.65	2.59	3.806	.028		IR-OPEN
1043.908	2250.374	1.574	2251.948	2543.00	19.42	5.86	2257.80	.00	3.07	86.30		1	0 .0
6.432	.0040					.0841	.54	1.57	2.78	3.806	.028		IR-OPEN
1050.340	2250.400	1.504	2251.904	2543.00	20.37	6.44	2258.35	.00	3.07	86.02		1	0 .0

WALL EXIT

FILE: gpsb2543.WSW

W S P G W - CIVILDESIGN Version 14.05

PAGE 2

Program Package Serial Number: 1439

WATER SURFACE PROFILE LISTING

Date: 3-25-2002 Time: 3:59: 0

Green Park Holdings, LLC

Parallel System to Main Culvert

File: gpsb2543 By: Alex Kingston

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/ID	Base Wt	INo	Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1050.340	2250.400	4.827	2255.227	2543.00	17.56	4.79	2260.02	.00	5.00	31.00	5.000	31.000	.00	2 .5
54.802	.0040					.0075	.41	4.83	1.43	5.00	.013	.00	.00	BOX
----- WARNING - Flow depth near top of box conduit -----														
1105.142	2250.619	4.653	2255.272	2543.00	18.22	5.15	2260.43	.00	5.00	31.00	5.000	31.000	.00	2 .5
67.896	.0040					.0084	.57	4.65	1.51	5.00	.013	.00	.00	BOX
----- WARNING - Flow depth near top of box conduit -----														
1173.038	2250.891	4.436	2255.327	2543.00	19.11	5.67	2261.00	.00	5.00	31.00	5.000	31.000	.00	2 .5
64.852	.0040					.0096	.62	4.44	1.63	5.00	.013	.00	.00	BOX
1237.890	2251.150	4.230	2255.380	2543.00	20.04	6.24	2261.62	5.00	5.00	31.00	5.000	31.000	.00	2 .5
52.940	.0040					.0108	.57	5.00	1.75	5.00	.013	.00	.00	BOX
1290.830	2251.360	4.061	2255.421	2543.00	20.87	6.77	2262.19	5.00	5.00	31.00	5.000	31.000	.00	2 .5
TRANS STR	.0041					.0079	.46	5.00	1.86		.013	.00	.00	BOX
1348.930	2251.600	6.732	2258.332	2543.00	17.59	4.81	2263.14	.00	6.73	15.04		3	0	.0
257.777	.0040					.0042	1.08	6.73	1.00	7.300	.013			IR-COV
1606.707	2252.631	7.250	2259.881	2543.00	16.77	4.37	2264.25	.00	6.73	11.91		3	0	.0
86.610	.0040					.0042	.36	7.25	.83	7.300	.013			IR-COV

FILE: gpsb2543.WSW

W S P G W - CIVILDESIGN Version 14.05  
 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 3

Date: 3-25-2002 Time: 3:59: 0

Green Park Holdings, LLC  
 Parallel System to Main Culvert  
 File: gpsb2543 By: Alex Kingston

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt. I.D.	No Wth ZL	Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1693.318	2252.977	7.300	2260.277	2543.00	16.71	4.34	2264.61	.00	6.73	11.48		3	0	.0
327.406	.0040					.0042	1.37	7.30	.81	7.300	.013			IR-COV
2020.724	2254.287	7.300	2261.587	2543.00	16.71	4.34	2265.92	.00	6.73	11.48		3	0	.0
HYDRAULIC JUMP														
2020.724	2254.287	6.171	2260.458	2543.00	18.77	5.47	2265.93	.00	6.73	17.18		3	0	.0
131.320	.0040					.0054	.70	6.17	1.18	7.300	.013			IR-COV
2152.044	2254.812	5.838	2260.650	2543.00	19.63	5.98	2266.63	.00	6.73	18.18		3	0	.0
135.451	.0040					.0060	.81	5.84	1.30	7.300	.013			IR-COV
2287.495	2255.354	5.513	2260.867	2543.00	20.58	6.58	2267.45	.00	6.73	18.91		3	0	.0
127.841	.0040					.0068	.87	5.51	1.42	7.300	.013			IR-COV
2415.335	2255.865	5.215	2261.080	2543.00	21.59	7.24	2268.32	.00	6.73	19.58		3	0	.0
119.632	.0040					.0077	.93	5.22	1.55	7.300	.013			IR-COV
2534.968	2256.343	4.939	2261.282	2543.00	22.64	7.96	2269.24	.00	6.73	20.14		3	0	.0
111.554	.0040					.0088	.99	4.94	1.69	7.300	.013			IR-COV
2646.522	2256.789	4.682	2261.471	2543.00	23.75	8.76	2270.23	.00	6.73	20.52		3	0	.0
103.963	.0040					.0101	1.05	4.68	1.83	7.300	.013			IR-COV
2750.485	2257.205	4.441	2261.646	2543.00	24.91	9.63	2271.28	.00	6.73	20.89		3	0	.0
97.347	.0040					.0116	1.13	4.44	1.99	7.300	.013			IR-COV

FILE: gpsb2543.WSW

 W S P G W - CIVILDESIGN Version 14.05  
 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 4

Date: 3-25-2002 Time: 3:59: 0

 Green Park Holdings, LLC  
 Parallel System to Main Culvert  
 File: gpsb2543 By: Alex Kingston

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top	Height/ Base Wt	No Wth		
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
2847.833	2257.594	4.215	2261.809	2543.00	26.12	10.60	2272.41	.00	6.73	21.23		3	0	.0
91.538	.0040					.0133	1.21	4.22	2.15	7.300	.013			IR-COV
2939.370	2257.960	4.004	2261.964	2543.00	27.40	11.66	2273.62	.00	6.73	21.54		3	0	.0
TRANS STR	1.0250					.0128	.00	4.00	2.33		.013			IR-COV
2939.380	2257.970	4.769	2262.739	2543.00	26.66	11.04	2273.78	2.85	7.95	20.00	14.000	20.000	.00	0
25.086	.0385					.0107	.27	7.62	2.15	3.16	.013	.00	.00	RECTANG
2964.466	2258.936	4.977	2263.914	2543.00	25.55	10.13	2274.05	2.62	7.95	20.00	14.000	20.000	.00	0
23.309	.0385					.0094	.22	7.59	2.02	3.16	.013	.00	.00	RECTANG
2987.774	2259.834	5.220	2265.054	2543.00	24.36	9.21	2274.27	2.38	7.95	20.00	14.000	20.000	.00	0
19.226	.0385					.0082	.16	7.60	1.88	3.16	.013	.00	.00	RECTANG
3007.000	2260.575	5.475	2266.050	2543.00	23.22	8.37	2274.42	2.16	7.95	20.00	14.000	20.000	.00	0

T1 Green Park Holdings  
T2 Flow From I-15 to Las Vegas BLVD  
T3 gpsbalt3  
SO 945.1602248.900 1 2254.490  
1000.0002249.200 1 .028 .000  
1040.3402249.360 1 .028 .000  
1050.3402249.400 1 4 .028 10.000 2249.400 .0 .000  
WX 1050.3402249.400 5  
R 1237.8902250.150 5 .013 .000 .000 0  
TS 1316.5402250.450 6 .013 -47.074  
R 1400.0002250.800 6 .013 .000 .000 0  
R 2200.0002254.000 6 .013 .000 .000 0  
R 2600.0002255.600 6 .013 .000 .000 0  
R 2938.2902256.950 6 .013 .000 .000 0  
R 3093.3902257.570 6 .013 88.866 .000 0  
R 3545.4702259.380 6 .013 .000 .000 0  
R 3729.7002260.120 6 .013 -88.866 .000 0  
R 3880.2502260.720 6 .013 .000 .000 0  
R 4029.5902261.320 6 .013 85.566 .000 0  
R 4200.0002262.000 6 .013 .000 .000 0  
R 4600.0002263.600 6 .013 .000 .000 0  
R 4835.6902264.500 6 .013 .000 .000 0  
R 4862.0002264.200 6 .013 .000 .000 0  
JX 4872.0002265.450 6 9 .013 482.000 2264.68 12.0 10.000  
R 4887.3802268.800 6 .013 6.287 .000 0  
R 5092.0002270.840 6 .013 23.674 .000 0  
R 5168.7902272.750 6 .013 .000 .000 0  
TS 5327.9602276.770 7 .013 -60.798  
R 5361.7202277.620 7 .013 -12.895 .000 0  
R 5497.9802281.100 7 .013 .000 .000 0  
R 5773.0002282.700 7 .013 .000 .000 0  
WE 5773.0002282.700 8 .500  
R 5803.0302283.190 8 .013 .000  
SH 5803.0302283.190 8 2283.190  
CD 2 3 0 .000 7.000 10.000 .000 .000 .00  
CD 3 3 2 .500 5.000 22.000 .000 .000 .00  
CD 4 3 2 .500 5.000 22.000 .000 .000 .00  
CD 5 3 2 .500 5.000 22.000 .000 .000 .00  
CD 7 3 2 .500 4.000 37.000 .000 .000 .00  
CD 8 1 0 .000 4.000 60.000 .330 .330 .00  
CD 1 5 0 .000 .00 .00 .00 .00 .00 .00 .00 .00 .00  
CD 6 6 0 .000 .00 .00 .00 .00 .00 .00 .00 .00 .00  
CD 9 6 0 .000 .00 .00 .00 .00 .00 .00 .00 .00 .00  
PTS 1 4 .000 5.000 10.000 .000 90.000 .000 100.000 5.000  
PTS 621 .000 .000 16.000 .000 16.000 1.000 15.640 3.000 15.410 3.540  
PTS 15.160 4.000 14.770 4.550 14.380 5.000 13.090 6.000 12.000 6.480  
PTS 11.000 6.720 8.000 7.000 5.000 6.720 4.000 6.480 2.910 6.000  
PTS 1.620 5.000 1.230 4.550 .840 4.000 .590 3.540 .360 3.000  
921 .000 1.000  
.000 .000 10.000 .000 10.000 4.000 9.820 4.530 9.490 5.000  
9.000 5.390 8.500 5.590 8.000 5.730 7.520 5.830 7.000 5.930  
PTS 6.000 6.050 5.000 6.090 4.000 6.050 3.000 5.930 2.480 5.830  
PTS 2.000 5.730 1.500 5.590 1.000 5.390 .510 5.000 .180 4.530  
PTS .000 4.000  
Q 1326.000 .0

FILE: gpsbalt3.WSW

W S P G W - CIVILDESIGN Version 14.05  
 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 1

Date: 3-26-2002 Time: 4:27:14

Green Park Holdings  
 Flow From 1-15 to Las Vegas BLVD  
 gpsbalt3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top	Height/Dia.	Base Wt	No Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZL
													Type Ch
945.160	2248.900	5.590	2254.490	1818.00	3.57	.20	2254.69	.00	2.47	100.00		1	0 .0
44.460	.0055					.0006	.03	5.59	.28	2.840	.028		IR-OPEN
989.620	2249.143	5.353	2254.496	1818.00	3.75	.22	2254.71	.00	2.47	100.00		1	0 .0
10.380	.0055					.0006	.01	5.35	.30	2.840	.028		IR-OPEN
1000.000	2249.200	5.298	2254.498	1818.00	3.79	.22	2254.72	.00	2.47	100.00		1	0 .0
40.340	.0040					.0007	.03	5.30	.30	3.124	.028		IR-OPEN
1040.340	2249.360	5.152	2254.512	1818.00	3.91	.24	2254.75	.00	2.47	100.00		1	0 .0
JUNCT STR	.0040					.0007	.01	5.15	.32		.028		IR-OPEN
----- WARNING - Junction Analysis - Irregular Channel -----													
1050.340	2249.400	5.121	2254.521	1808.00	3.91	.24	2254.76	.00	2.46	100.00		1	0 .0
HYDRAULIC JUMP													
1050.340	2249.400	4.727	2254.127	1808.00	18.21	5.15	2259.28	.00	5.00	22.00	5.000	22.000	.00 2 .5
HYDRAULIC DROP													
----- WARNING - Flow depth near top of box conduit -----													
1050.340	2249.400	5.121	2254.521	1808.00	17.22	4.60	2259.12	.00	5.00	22.00	5.000	22.000	.00 2 .5
187.550	.0040					.0137	2.57	5.12	1.39	5.00	.013	.00	BOX

FILE: gpsbalt3.WSW

 W S P G W - CIVILDESIGN Version 14.05  
 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 2

Date: 3-26-2002 Time: 4:27:14

 Green Park Holdings  
 Flow From I-15 to Las Vegas BLVD  
 gpsbalt3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt I.D.	ZL	No Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
1237.890	2250.150	6.945	2257.094	1808.00	17.22	4.60	2261.70	.00	5.00	22.00	5.000	22.000	.00	2 .5
TRANS STR	.0038							.00	1.39		.013	.00	.00	BOX
1316.540	2250.450	6.523	2256.973	1808.00	19.68	6.02	2262.99	.00	6.52	7.64			6	0 .0
.114	.0042					.0075	.00	6.52	1.00	6.500	.013			IR-COV
1316.654	2250.450	6.500	2256.950	1808.00	19.72	6.04	2262.99	.00	6.52	7.83			6	0 .0
83.346	.0042					.0075	.62	6.50	1.02	6.500	.013			IR-COV
1400.000	2250.800	6.500	2257.300	1808.00	19.72	6.04	2263.34	.00	6.52	7.83			6	0 .0
1400.000	2250.800	6.523	2257.323	1808.00	19.68	6.02	2263.34	.00	6.52	7.64			6	0 .0
.108	.0040					.0075	.00	6.52	1.00	6.500	.013			IR-COV
1400.108	2250.801	6.500	2257.301	1808.00	19.72	6.04	2263.34	.00	6.52	7.83			6	0 .0
799.892	.0040					.0075	5.99	6.50	1.02	6.500	.013			IR-COV
2200.000	2254.000	6.500	2260.500	1808.00	19.72	6.04	2266.54	.00	6.52	7.83			6	0 .0
2200.000	2254.000	6.523	2260.523	1808.00	19.68	6.02	2266.54	.00	6.52	7.64			6	0 .0
.108	.0040					.0075	.00	6.52	1.00	6.500	.013			IR-COV
2200.108	2254.000	6.500	2260.500	1808.00	19.72	6.04	2266.54	.00	6.52	7.83			6	0 .0
399.892	.0040					.0075	2.99	6.50	1.02	6.500	.013			IR-COV
2600.000	2255.600	6.500	2262.100	1808.00	19.72	6.04	2268.14	.00	6.52	7.83			6	0 .0
500.000	2255.600	6.523	2262.123	1808.00	19.68	6.02	2268.14	.00	6.52	7.64			6	0 .0
.107	.0040					.0075	.00	6.52	1.00	6.500	.013			IR-COV
2600.107	2255.601	6.500	2262.101	1808.00	19.72	6.04	2268.14	.00	6.52	7.83			6	0 .0
338.183	.0040					.0075	2.53	6.50	1.02	6.500	.013			IR-COV

FILE: gpsbalt3.WSW

 W S P G W - CIVILDESIGN Version 14.05  
 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 3

Date: 3-26-2002 Time: 4:27:14

 Green Park Holdings  
 Flow From I-15 to Las Vegas BLVD  
 gpsbalt3

gpsbalt3														
Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top	Height/Dia.-FT	Base Wt I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
2938.290	2256.950	6.500	2263.450	1808.00	19.72	6.04	2269.49	.00	6.52	7.83			6	0 .0
2938.290	2256.950	6.523	2263.473	1808.00	19.68	6.02	2269.49	7.00	6.52	7.64			6	0 .0
.108	.0040					.0075	.00	7.00	1.00	6.500	.013			IR-COV
2938.398	2256.950	6.500	2263.450	1808.00	19.72	6.04	2269.49	7.00	6.52	7.83			6	0 .0
154.992	.0040					.0075	1.16	7.00	1.02	6.500	.013			IR-COV
3093.390	2257.570	6.500	2264.070	1808.00	19.72	6.04	2270.11	7.00	6.52	7.83			6	0 .0
3093.390	2257.570	6.523	2264.093	1808.00	19.68	6.02	2270.11	.00	6.52	7.64			6	0 .0
.108	.0040					.0075	.00	6.52	1.00	6.500	.013			IR-COV
3093.498	2257.571	6.500	2264.071	1808.00	19.72	6.04	2270.11	.00	6.52	7.83			6	0 .0
451.972	.0040					.0075	3.38	6.50	1.02	6.500	.013			IR-COV
3545.470	2259.380	6.500	2265.880	1808.00	19.72	6.04	2271.92	.00	6.52	7.83			6	0 .0
3545.470	2259.380	6.523	2265.903	1808.00	19.68	6.02	2271.92	7.00	6.52	7.64			6	0 .0
.108	.0040					.0075	.00	7.00	1.00	6.500	.013			IR-COV
3545.578	2259.380	6.500	2265.880	1808.00	19.72	6.04	2271.92	7.00	6.52	7.83			6	0 .0
184.122	.0040					.0075	1.38	7.00	1.02	6.500	.013			IR-COV
3729.700	2260.120	6.500	2266.620	1808.00	19.72	6.04	2272.66	7.00	6.52	7.83			6	0 .0
3729.700	2260.120	6.523	2266.643	1808.00	19.68	6.02	2272.66	.00	6.52	7.64			6	0 .0
.107	.0040					.0075	.00	6.52	1.00	6.500	.013			IR-COV
3729.807	2260.121	6.500	2266.621	1808.00	19.72	6.04	2272.66	.00	6.52	7.83			6	0 .0
150.443	.0040					.0075	1.13	6.50	1.02	6.500	.013			IR-COV
3880.250	2260.720	6.500	2267.220	1808.00	19.72	6.04	2273.26	.00	6.52	7.83			6	0 .0
3880.250	2260.720	6.523	2267.243	1808.00	19.68	6.02	2273.26	7.00	6.52	7.64			6	0 .0
.108	.0040					.0075	.00	7.00	1.00	6.500	.013			IR-COV



FILE: gpsbalt3.WSW

W S P G W - CIVILDESIGN Version 14.05

PAGE 4

Program Package Serial Number: 1439

WATER SURFACE PROFILE LISTING

Date: 3-26-2002 Time: 4:27:14

Green Park Holdings

Flow From I-15 to Las Vegas BLVD

gpsbalt3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt I.D.	ZL	No Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
3880.358	2260.720	6.500	2267.220	1808.00	19.72	6.04	2273.26	7.00	6.52	7.83			6	0 .0
149.232	.0040					.0075	1.12	7.00	1.02	6.500	.013			IR-COV
4029.590	2261.320	6.500	2267.820	1808.00	19.72	6.04	2273.86	7.00	6.52	7.83			6	0 .0
4029.590	2261.320	6.523	2267.843	1808.00	19.68	6.02	2273.86	.00	6.52	7.64			6	0 .0
.107	.0040					.0075	.00	6.52	1.00	6.500	.013			IR-COV
4029.698	2261.321	6.500	2267.821	1808.00	19.72	6.04	2273.86	.00	6.52	7.83			6	0 .0
170.302	.0040					.0075	1.27	6.50	1.02	6.500	.013			IR-COV
4200.000	2262.000	6.500	2268.500	1808.00	19.72	6.04	2274.54	.00	6.52	7.83			6	0 .0
4200.000	2262.000	6.523	2268.523	1808.00	19.68	6.02	2274.54	.00	6.52	7.64			6	0 .0
.108	.0040					.0075	.00	6.52	1.00	6.500	.013			IR-COV
4200.108	2262.000	6.500	2268.500	1808.00	19.72	6.04	2274.54	.00	6.52	7.83			6	0 .0
399.892	.0040					.0075	2.99	6.50	1.02	6.500	.013			IR-COV
4600.000	2263.600	6.500	2270.100	1808.00	19.72	6.04	2276.14	.00	6.52	7.83			6	0 .0
4600.000	2263.600	6.523	2270.123	1808.00	19.68	6.02	2276.14	.00	6.52	7.64			6	0 .0
.103	.0038					.0075	.00	6.52	1.00	6.500	.013			IR-COV
4600.103	2263.601	6.500	2270.101	1808.00	19.72	6.04	2276.14	.00	6.52	7.83			6	0 .0
235.587	.0038					.0075	1.76	6.50	1.02	6.500	.013			IR-COV
4835.690	2264.500	6.500	2271.000	1808.00	19.72	6.04	2277.04	.00	6.52	7.83			6	0 .0
4835.690	2264.500	6.523	2271.023	1808.00	19.68	6.02	2277.04	.00	6.52	7.64			6	0 .0
4.145	-.0114					.0083	.03	6.52	1.00	.000	.013			IR-COV
4839.834	2264.453	6.849	2271.302	1808.00	19.28	5.77	2277.07	.00	6.52	3.23			6	0 .0
5.988	-.0114					.0092	.06	6.85	.63	.000	.013			IR-COV

FILE: gpsbalt3.WSW

W S P G W - CIVILDESIGN Version 14.05  
 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 5

Date: 3-26-2002 Time: 4:27:14

Green Park Holdings  
 Flow From I-15 to Las Vegas BLVD  
 gpsbalt3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.-FT	Base Wt I.D.	ZL	No Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
4845.823	2264.385	7.000	2271.385	1808.00	19.23	5.74	2277.12	.00	6.52	.00		6	0	.0
16.177	-.0114					.0092	.15	7.00	.00	.000	.013			IR-COV
4862.000	2264.200	7.334	2271.534	1808.00	19.23	5.74	2277.27	.00	6.52	.00		6	0	.0
JUNCT STR	.1250					.0050	.05	.00	.00		.013			IR-COV
----- WARNING - Junction Analysis - Irregular Channel -----														
4872.000	2265.450	10.136	2275.586	1326.00	14.10	3.09	2278.67	.00	5.72	.00		6	0	.0
8.446	.2178					.0050	.04	.00	.00	1.391	.013			IR-COV
4880.446	2267.290	8.425	2275.715	1326.00	14.10	3.09	2278.80	.00	5.72	.00		6	0	.0
HYDRAULIC JUMP														
4880.446	2267.290	3.546	2270.836	1326.00	23.89	8.86	2279.70	2.44	5.72	14.81		6	0	.0
1.792	.2178					.0132	.02	5.98	2.18	1.391	.013			IR-COV
4882.238	2267.680	3.650	2271.330	1326.00	23.25	8.39	2279.72	2.29	5.72	14.70		6	0	.0
2.777	.2178					.0119	.03	5.94	2.08	1.391	.013			IR-COV
4885.015	2268.285	3.841	2272.126	1326.00	22.17	7.63	2279.76	2.05	5.72	14.49		6	0	.0
2.365	.2178					.0104	.02	5.89	1.92	1.391	.013			IR-COV
4887.380	2268.800	4.044	2272.844	1326.00	21.13	6.94	2279.78	.52	5.72	14.26		6	0	.0
204.620	.0100					.0096	1.96	4.56	1.78	4.001	.013			IR-COV

FILE: gpsbalt3.WSW

W S P G W - CIVILDESIGN Version 14.05

PAGE 6

Program Package Serial Number: 1439

WATER SURFACE PROFILE LISTING

Date: 3-26-2002 Time: 4:27:14

Green Park Holdings  
Flow From I-15 to Las Vegas BLVD  
gpsbalt3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top/Height	Base Wt/I.D.	No Wth ZL	Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR
													Type Ch
5092.000	2270.840	4.080	2274.920	1326.00	20.97	6.83	2281.75	.00	5.72	14.21		6	0 .0
24.878	.0249					.0089	.22	4.08	1.75	2.865	.013		IR-COV
5116.878	2271.459	4.297	2275.756	1326.00	20.00	6.21	2281.97	.00	5.72	13.90		6	0 .0
19.283	.0249					.0078	.15	4.30	1.61	2.865	.013		IR-COV
5136.161	2271.938	4.533	2276.472	1326.00	19.07	5.64	2282.12	.00	5.72	13.56		6	0 .0
14.425	.0249					.0069	.10	4.53	1.48	2.865	.013		IR-COV
5150.585	2272.297	4.787	2277.084	1326.00	18.18	5.13	2282.22	.00	5.72	13.13		6	0 .0
10.100	.0249					.0061	.06	4.79	1.36	2.865	.013		IR-COV
5160.686	2272.549	5.064	2277.613	1326.00	17.33	4.66	2282.28	.00	5.72	12.59		6	0 .0
6.078	.0249					.0054	.03	5.06	1.24	2.865	.013		IR-COV
5166.763	2272.700	5.370	2278.070	1326.00	16.53	4.24	2282.31	.00	5.72	11.81		6	0 .0
2.027	.0249					.0049	.01	5.37	1.12	2.865	.013		IR-COV
5168.790	2272.750	5.716	2278.466	1326.00	15.75	3.85	2282.32	.73	5.72	10.91		6	0 .0
TRANS STR	.0253							6.445	1.000		.013		IR-COV
5327.960	2276.770	1.908	2278.678	1326.00	19.30	5.79	2284.46	4.00	3.48	37.00	4.000	37.000	.00 2 .5
33.760	.0252					.0166	.56	4.00	2.50	1.69	.013	.00	.00 BOX
5361.720	2277.620	1.968	2279.588	1326.00	18.72	5.44	2285.03	.00	3.48	37.00	4.000	37.000	.00 2 .5
19.164	.0255					.0154	.29	1.97	2.38	1.68	.013	.00	.00 BOX

FILE: gpsbalt3.WSW

W S P G W - CIVILDESIGN Version 14.05  
 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 7

Date: 3-26-2002 Time: 4:27:14

Green Park Holdings  
 Flow From I-15 to Las Vegas BLVD  
 gpsbalt3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height/Dia.	Base Wt	Ino Wth
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR
													Type Ch
5380.884	2278.109	2.013	2280.122	1326.00	18.30	5.20	2285.32	.00	3.48	37.00	4.000	37.000	.00 2 .5
-	-	-	-	-	-	-	-	-	-	-	-	-	-
32.040	.0255					.0138	.44	2.01	2.30	1.68	.013	.00	BOX
5412.924	2278.928	2.111	2281.039	1326.00	17.45	4.73	2285.77	.00	3.48	37.00	4.000	37.000	.00 2 .5
-	-	-	-	-	-	-	-	-	-	-	-	-	-
24.157	.0255					.0120	.29	2.11	2.15	1.68	.013	.00	BOX
5437.081	2279.545	2.214	2281.759	1326.00	16.64	4.30	2286.06	.00	3.48	37.00	4.000	37.000	.00 2 .5
-	-	-	-	-	-	-	-	-	-	-	-	-	-
18.702	.0255					.0104	.19	2.21	2.00	1.68	.013	.00	BOX
5455.782	2280.022	2.322	2282.344	1326.00	15.86	3.91	2286.25	.00	3.48	37.00	4.000	37.000	.00 2 .5
-	-	-	-	-	-	-	-	-	-	-	-	-	-
14.671	.0255					.0091	.13	2.32	1.86	1.68	.013	.00	BOX
5470.453	2280.397	2.435	2282.832	1326.00	15.12	3.55	2286.38	.00	3.48	37.00	4.000	37.000	.00 2 .5
-	-	-	-	-	-	-	-	-	-	-	-	-	-
11.546	.0255					.0079	.09	2.44	1.73	1.68	.013	.00	BOX
5481.999	2280.692	2.554	2283.246	1326.00	14.42	3.23	2286.48	.00	3.48	37.00	4.000	37.000	.00 2 .5
-	-	-	-	-	-	-	-	-	-	-	-	-	-
9.032	.0255					.0068	.06	2.55	1.61	1.68	.013	.00	BOX
5491.031	2280.923	2.679	2283.601	1326.00	13.75	2.94	2286.54	.00	3.48	37.00	4.000	37.000	.00 2 .5
-	-	-	-	-	-	-	-	-	-	-	-	-	-
6.949	.0255					.0059	.04	2.68	1.50	1.68	.013	.00	BOX
5497.980	2281.100	2.810	2283.910	1326.00	13.11	2.67	2286.58	.00	3.48	37.00	4.000	37.000	.00 2 .5
-	-	-	-	-	-	-	-	-	-	-	-	-	-
119.408	.0058					.0054	.64	2.81	1.40	2.76	.013	.00	BOX
5617.388	2281.795	2.875	2284.670	1326.00	12.81	2.55	2287.22	.00	3.48	37.00	4.000	37.000	.00 2 .5
-	-	-	-	-	-	-	-	-	-	-	-	-	-
93.849	.0058					.0048	.45	2.88	1.35	2.76	.013	.00	BOX

FILE: gpsbalt3.WSW

W S P G W - CIVILDESIGN Version 14.05  
 Program Package Serial Number: 1439  
 WATER SURFACE PROFILE LISTING

PAGE 8

Date: 3-26-2002 Time: 4:27:14

Green Park Holdings  
 Flow From I-15 to Las Vegas BLVD  
 gpsbalt3

Station	Invert Elev	Depth (FT)	Water Elev	Q (CFS)	Vel (FPS)	Vel Head	Energy Grd.El.	Super Elev	Critical Depth	Flow Top Width	Height Dia.-FT	Base Wt I.D.	No ZL	Wth Prs/Pip
L/Elem	Ch Slope					SF Ave	HF	SE Dpth	Froude N	Norm Dp	"N"	X-Fall	ZR	Type Ch
5711.237	2282.341	3.016	2285.356	1326.00	12.21	2.32	2287.67	.00	3.48	37.00	4.000	37.000	.00	2 .5
39.737	.0058					.0042	.17	3.02	1.26	2.76	.013	.00	.00	BOX
5750.974	2282.572	3.163	2285.735	1326.00	11.65	2.11	2287.84	.00	3.48	37.00	4.000	37.000	.00	2 .5
17.362	.0058					.0037	.06	3.16	1.17	2.76	.013	.00	.00	BOX
5768.336	2282.673	3.317	2285.990	1326.00	11.10	1.91	2287.90	.00	3.48	37.00	4.000	37.000	.00	2 .5
4.664	.0058					.0032	.01	3.32	1.09	2.76	.013	.00	.00	BOX
5773.000	2282.700	3.480	2286.180	1326.00	10.58	1.74	2287.92	.00	3.48	37.00	4.000	37.000	.00	2 .5
WALL ENTRANCE														
5773.000	2282.700	6.168	2288.868	1326.00	3.47	.19	2289.05	.00	2.46	64.07	4.000	60.000	.33	0 .0
16.021	.0163					.0001	.00	6.17	.25	1.30	.013	.00	.33	TRAP
5789.021	2282.961	5.890	2288.851	1326.00	3.63	.21	2289.06	.00	2.46	63.89	4.000	60.000	.33	0 .0
14.008	.0163					.0001	.00	5.89	.27	1.30	.013	.00	.33	TRAP
5803.030	2283.190	5.644	2288.834	1326.00	3.80	.22	2289.06	.00	2.46	63.73	4.000	60.000	.33	0 .0

*SOUTH BOX OUTLET  
HEC-RAS MODEL*

HEC-RAS Plan: Plan 01 River: Outlet Reach: 1

Reach	River Sta	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
		(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
1	242	2973.00	50.40	54.93	54.89	57.14	0.001786	11.94	248.96	55.00	0.99
1	241.5	2973.00	50.00	54.93	54.86	56.83	0.005517	11.42	303.58	99.25	0.91
1	241	2973.00	50.37	53.64	53.56	54.14	0.010973	6.99	584.26	423.00	0.73
1	240	2973.00	50.37	52.70		53.11	0.013695	6.78	607.87	507.28	0.78
1	239	6470.00	44.37	48.62		48.77	0.008506	6.06	2276.58	1214.19	0.69
1	238	6470.00	42.37	44.87	44.31	45.19	0.007540	5.94	1559.52	1048.73	0.66

HEC-RAS September 1998 Version 2.2  
 U.S. Army Corp of Engineers  
 Hydrologic Engineering Center  
 609 Second Street, Suite D  
 Davis, California 95616-4687  
 (916) 756-1104

```

X   X   XXXXXX   XXXX   XXXX   XX   XXXX
X   X   X       X   X   X   X   X   X   X
X   X   X       X   X   X   X   X   X   X
XXXXXXX XXXX   X   XXX XXXX   XXXXXX   XXXX
X   X   X       X   X   X   X   X   X   X
X   X   X       X   X   X   X   X   X   X
X   X   XXXXXX   XXXX   X   X   X   X   XXXXX

```

## PROJECT DATA

Project Title: Southbox Outlet  
 Project File : sbox.prj  
 Run Date and Time: 3/25/2002 1:37:42 PM

Project in English units

## PLAN DATA

Plan Title: Plan 01  
 Plan File : C:\HEC\RAS\sbox.p01

Geometry Title: Geom 01  
 Geometry File : C:\HEC\RAS\sbox.g01

Flow Title : Flow 01  
 Flow File : C:\HEC\RAS\sbox.f01

## Plan Summary Information:

Number of: Cross Sections =	17	Multiple Openings =	0
Culverts =	0	Inline Weirs =	0
Bridges =	0		

## Computational Information

Water surface calculation tolerance = 0.01  
 Critical depth calculation tolerance = 0.01  
 Maximum number of iterations = 20  
 Maximum difference tolerance = 0.3  
 Flow tolerance factor = 0.001

## Computation Options

Critical depth computed only where necessary  
 Conveyance Calculation Method: At breaks in n values only  
 Friction Slope Method: Average Conveyance  
 Computational Flow Regime: Mixed Flow

## FLOW DATA

Flow Title: Flow 01  
 Flow File : C:\HEC\RAS\sbox.f01

## Flow Data (cfs)

River	Reach	RS	PF 1
Outlet	1	242	2973
Outlet	1	239	6470

## Boundary Conditions

River	Reach	Profile	Upstream	Downstream
Outlet	1	PF 1	Known WS = 55.2	Known WS = 44.87

## GEOMETRY DATA

Geometry Title: Geom 01  
 Geometry File : C:\HEC\RAS\sbox.g01

CROSS SECTION RIVER: Outlet  
 REACH: 1 RS: 242



**Description:**

Station Elevation Data				num=	4				
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
1000	55.4	1000	50.4	1055	50.4	1055	55.4		

Manning's n Values		num=		3	
Sta	n Val	Sta	n Val	Sta	n Val
1000	.013	1000	.013	1055	.013

Bank Sta:	Left	Right	Lengths:	Left Channel	Right	Coeff Contr.	Expan.
	1000	1055		77	77	.1	.3

## CROSS SECTION OUTPUT      Profile #PF 1

E.G. Elev (ft)	57.14	Element	Left OB	Channel	Right OB
Vel Head (ft)	2.21	Wt. n-Val.		0.013	
W.S. Elev (ft)	54.93	Reach Len. (ft)	77.00	77.00	77.00
Crit W.S. (ft)	54.89	Flow Area (sq ft)		248.96	
E.G. Slope (ft/ft)	0.001786	Area (sq ft)		248.96	
Q Total (cfs)	2973.00	Flow (cfs)		2973.00	
Top Width (ft)	55.00	Top Width (ft)		55.00	
Vel Total (ft/s)	11.94	Avg. Vel. (ft/s)		11.94	
Max Chl Dpth (ft)	4.53	Hydr. Depth (ft)		4.53	
Conv. Total (cfs)	70346.8	Conv. (cfs)		70346.8	
Length Wtd. (ft)	77.00	Wetted Per. (ft)		64.05	
Min Ch El (ft)	50.40	Shear (lb/sq ft)		0.43	
Alpha	1.00	Stream Power (lb/ft s)		5.18	
Frctn Loss (ft)	0.22	Cum Volume (acre-ft)	19.62	6.10	7.21
C & E Loss (ft)	0.10	Cum SA (acres)	12.68	1.92	7.37

Warning: The conveyance ratio (upstream conveyance divided by downstream conveyance) is less than 0.7 or greater than 1.4. This may indicate the need for additional cross sections.

CROSS SECTION RIVER: Outlet  
REACH: 1 RS: 241.5

## INPUT

**Description:**

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
1007	56	1020	50	1069	50	1076	54	1111	55
1158	56	1238	57						

Manning's n Values		num= 3	
Sta	n Val	Sta	n Val
1007	.04	1020	.028
		1069	.04

Bank Sta:	Left	Right	Lengths:		Left Channel	Right	Coeff Contr.	Expan.
	1020	1069	58.85	87.57	78.26	.1	.3	

## CROSS SECTION OUTPUT      Profile #PF 1

E.G. Elev (ft)	56.83	Element	Left OB	Channel	Right OB
Vel Head (ft)	1.90	Wt. n-Val.	0.040	0.028	0.040
W.S. Elev (ft)	54.93	Reach Len. (ft)	58.85	87.57	78.26
Crit W.S. (ft)	54.86	Flow Area (sq ft)	26.33	241.59	35.66
E.G. Slope (ft/ft)	0.005517	Area (sq ft)	26.33	241.59	35.66
Q Total (cfs)	2973.00	Flow (cfs)	124.33	2758.49	90.18
Top Width (ft)	99.25	Top Width (ft)	10.68	49.00	39.56
Vel Total (ft/s)	9.79	Avg. Vel. (ft/s)	4.72	11.42	2.53
Max Chl Dpth (ft)	4.93	Hydr. Depth (ft)	2.47	4.93	0.90
Conv. Total (cfs)	40027.4	Conv. (cfs)	1673.9	37139.2	1214.2
Length Wtd. (ft)	85.47	Wetted Per. (ft)	11.77	49.00	40.64
Min Ch El (ft)	50.00	Shear (lb/sq ft)	0.77	1.70	0.30
Alpha	1.27	Stream Power (lb/ft s)	3.64	19.39	0.76
Frcn Loss (ft)	0.47	Cum Volume (acre-ft)	19.60	5.67	7.18
C & E Loss (ft)	0.18	Cum SA (acres)	12.67	1.82	7.33

Warning: The velocity head has changed by more than 0.5 ft (0.15 m). This may indicate the need for additional cross sections.

CROSS SECTION RIVER: Outlet  
REACH: 1 RS: 241.375<sup>A</sup>

## INPUT

**Description:**

Station Elevation Data			num= 10								
Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
990.75	55.59	1015	50.33	1022.5	50.09	1024.22	50.09	1065.5	50.09		
1070.5	50.47	1078.9	53.48	1120.89	54.31	1177.27	55.16	1273.25	56.08		

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
990.75	.041	1015	.032	1070.5	.04	1273.25	.04

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 1015 1070.5 58.85 87.57 78.26 .1 .3

CROSS SECTION RIVER: Outlet  
 CH: 1 RS: 241.25\*

## INPUT

## Description:

Station Elevation Data		num=	10
Sta	Elev	Sta	Elev
974.5	55.18	1010	50.67
1072	50.93	1081.8	52.96

Manning's n Values		num=	4
Sta	n Val	Sta	n Val
974.5	.043	1010	.036
1072	.043	1081.8	.036

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 1010 1072 58.85 87.57 78.26 .1 .3

CROSS SECTION RIVER: Outlet  
 REACH: 1 RS: 241.125\*

## INPUT

## Description:

Station Elevation Data		num=	10
Sta	Elev	Sta	Elev
958.25	54.78	1005	51
1073.5	51.4	1084.69	52.45

Manning's n Values		num=	4
Sta	n Val	Sta	n Val
958.25	.044	1005	.041
1073.5	.044	1084.69	.041

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 1005 1073.5 58.85 87.57 78.26 .1 .3

CROSS SECTION RIVER: Outlet  
 REACH: 1 RS: 241

## INPUT

## Description:

Station Elevation Data		num=	7
Sta	Elev	Sta	Elev
942	54.37	1000	51.33
1075	51.87	1379	53.31

Manning's n Values		num=	3
Sta	n Val	Sta	n Val
942	.045	1000	.045
1075	.045	1379	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 1000 1075 51.4 90.06 83.44 .1 .3

CROSS SECTION OUTPUT Profile #PF 1

Element	Left OB	Channel	Right OB
E.G. Elev (ft)	54.14	54.14	54.14
Vel Head (ft)	0.51	0.045	0.040
W.S. Elev (ft)	53.64	53.64	53.64
Crit W.S. (ft)	53.56	53.56	53.56
E.G. Slope (ft/ft)	0.010973	0.010973	0.010973
Q Total (cfs)	2973.00	192.75	1274.09
Top Width (ft)	423.00	44.00	304.00
Vel Total (ft/s)	5.09	3.80	4.01
Max Chl Dpth (ft)	3.27	1.15	1.05
Conv. Total (cfs)	28381.5	1840.1	12163.0
Length Wtd. (ft)	82.49	44.06	304.33
Min Ch El (ft)	50.37	0.79	0.72
Alpha	1.26	3.00	2.87
Frctn Loss (ft)	1.00	19.31	6.02
C & E Loss (ft)	0.03	12.51	6.08

Warning: The cross-section end points had to be extended vertically for the computed water surface.  
 Warning: The energy loss was greater than 1.0 ft (0.3 m) between the current and previous cross section. This may indicate the need for additional cross sections.

CROSS SECTION RIVER: Outlet  
 REACH: 1 RS: 240

## INPUT

## Description:

Station Elevation Data		num=	5
Sta	Elev	Sta	Elev
853	54.17	1000	50.37
1047	50.37	1334	52.37

Manning's n Values		num=	3
Sta	n Val	Sta	n Val
853	.045	1000	.045
1047	.045	1334	.045

Bank Sta: Left	Right	Lengths: Left Channel	Right	Coeff Contr.	Expan.
1000	1047	101.5 87.25	84	.1	.3

## CROSS SECTION OUTPUT Profile #PF 1

E.G. Elev (ft)	53.11	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.41	Wt. n-Val.	0.045	0.045	0.040
W.S. Elev (ft)	52.70	Reach Len. (ft)	101.50	87.25	84.00
Crit W.S. (ft)		Flow Area (sq ft)	104.60	109.30	393.97
E.G. Slope (ft/ft)	0.013695	Area (sq ft)	104.60	109.30	393.97
Q Total (cfs)	2973.00	Flow (cfs)	446.84	741.33	1784.83
Top Width (ft)	507.28	Top Width (ft)	89.96	47.00	370.32
Vel Total (ft/s)	4.89	Avg. Vel. (ft/s)	4.27	6.78	4.53
Max Chl Dpth (ft)	2.33	Hydr. Depth (ft)	1.16	2.33	1.06
Conv. Total (cfs)	25404.7	Conv. (cfs)	3818.3	6334.8	15251.6
Length Wtd. (ft)	88.80	Wetted Per. (ft)	89.99	47.00	370.33
Min Ch El (ft)	50.37	Shear (lb/sq ft)	0.99	1.99	0.91
Alpha	1.11	Stream Power (lb/ft s)	4.25	13.49	4.12
Frctn Loss (ft)	1.13	Cum Volume (acre-ft)	19.22	3.29	5.34
C & E Loss (ft)	0.02	Cum SA (acres)	12.43	1.20	5.43

Warning: The energy loss was greater than 1.0 ft (0.3 m). between the current and previous cross section. This may indicate the need for additional cross sections.

CROSS SECTION RIVER: Outlet  
REACH: 1 RS: 239.75\*

## INPUT

## Description:

Station Elevation Data	num=	16
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev		
860.22 53.22 900.44 51.94 920.19 51.81 1109.85 50.5 1118.03 50.45		
1152.08 50.21 1162.97 50.14 1165.36 50.12 1275 50.37 1275.25 50.37		
1279.75 49.37 1281 48.87 1316.25 48.87 1319 49.37 1581.62 51.53		
1698.75 52.19		

Manning's n Values	num=	4
Sta n Val Sta n Val Sta n Val Sta n Val		
860.22 .054 1275 .044 1319 .041 1698.75 .041		

Bank Sta: Left	Right	Lengths: Left Channel	Right	Coeff Contr.	Expan.
1275	1319	101.5 87.25	84	.1	.3

CROSS SECTION RIVER: Outlet  
REACH: 1 RS: 239.5\*

## INPUT

## Description:

Station Elevation Data	num=	16
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev		
867.43 52.27 933.63 50.09 966.13 50 1278.24 49.13 1291.68 49.09		
1347.72 48.93 1365.65 48.88 1369.57 48.87 1550 50.37 1550.5 50.37		
1559.5 48.37 1562 47.37 1585.5 47.37 1591 48.37 1829.24 50.68		
1935.5 51.52		

Manning's n Values	num=	4
Sta n Val Sta n Val Sta n Val Sta n Val		
867.43 .062 1550 .043 1591 .043 1935.5 .043		

Bank Sta: Left	Right	Lengths: Left Channel	Right	Coeff Contr.	Expan.
1550	1591	101.5 87.25	84	.1	.3

CROSS SECTION RIVER: Outlet  
REACH: 1 RS: 239.25\*

## INPUT

## Description:

Station Elevation Data	num=	16
Sta Elev Sta Elev Sta Elev Sta Elev Sta Elev		
874.65 51.32 966.81 48.23 1012.06 48.18 1446.62 47.75 1465.34 47.73		
1543.36 47.65 1568.32 47.63 1573.79 47.62 1825 50.37 1825.75 50.37		
1839.25 47.37 1843 45.87 1854.75 45.87 1863 47.37 2076.87 49.84		
2172.25 50.84		

Manning's n Values	num=	4
Sta n Val Sta n Val Sta n Val Sta n Val		
874.65 .071 1825 .041 1863 .044 2172.25 .044		

Bank Sta: Left	Right	Lengths: Left Channel	Right	Coeff Contr.	Expan.
1825	1863	101.5 87.26	84	.1	.3

CROSS SECTION RIVER: Outlet  
REACH: 1 RS: 239

## INPUT

## Description:

Station Elevation Data	num=	14
------------------------	------	----

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
881.87	50.37	1000	46.37	1058	46.37	1615	46.37	1639	46.37
1739	46.37	1771	46.37	1778	46.37	2100	50.37	2101	50.37
2119	46.37	2124	44.37	2135	46.37	2409	50.17		

Manning's n Values num= 3

Sta	n Val	Sta	n Val	Sta	n Val
881.87	.08	2100	.04	2135	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Left	Channel	Right	Coeff	Contr.	Expan.
2100	2135	63.5	92.46	92.14	.1	.3	

## CROSS SECTION OUTPUT Profile #PF 1

E.G. Elev (ft)	48.77	Element	Left OB	Channel	Right OB
Vel Head (ft)	0.15	Wt. n-Val.	0.080	0.040	0.045
W.S. Elev (ft)	48.62	Reach Len. (ft)	63.50	92.46	92.14
Crit W.S. (ft)		Flow Area (sq ft)	2030.42	63.43	182.74
E.G. Slope (ft/ft)	0.008506	Area (sq ft)	2030.42	63.43	182.74
Q Total (cfs)	6470.00	Flow (cfs)	5483.29	384.54	602.17
Top Width (ft)	1214.19	Top Width (ft)	1025.72	26.13	162.34
Vel Total (ft/s)	2.84	Avg. Vel. (ft/s)	2.70	6.06	3.30
Max Chl Dpth (ft)	4.25	Hydr. Depth (ft)	1.98	2.43	1.13
Conv. Total (cfs)	70152.6	Conv. (cfs)	59453.9	4169.5	6529.2
Length Wtd. (ft)	69.16	Wetted Per. (ft)	1025.78	26.94	162.35
Min Ch El (ft)	44.37	Shear (lb/sq ft)	1.05	1.25	0.60
Alpha	1.16	Stream Power (lb/ft s)	2.84	7.58	1.97
Frctn Loss (ft)	0.58	Cum Volume (acre-ft)	12.36	2.52	3.46
C & E Loss (ft)	0.01	Cum SA (acres)	7.05	0.89	3.66

Warning: Divided flow computed for this cross-section.

CROSS SECTION RIVER: Outlet  
REACH: 1 RS: 238.833<sup>4</sup>

## INPUT

## Description:

Station Elevation Data num= 21

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
901.56	49.7	988.23	46.68	1008.87	46.03	1061.56	46.01	1183.75	45.95
1567.57	45.74	1589.37	45.73	1643.33	45.7	1680.22	45.7	1709.29	45.7
1715.65	45.7	2008.17	49.04	2009	49.04	2024	45.7	2028.17	44.04
2038.33	44.04	2040	44.04	2046.33	44.04	2055.5	45.7	2167.21	47.07
210.37	49.38								

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
901.56	.074	2008.17	.04	2055.5	.045	2410.37	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Left	Channel	Right	Coeff	Contr.	Expan.
2008.17	2055.5	63.5	92.46	92.14	.1	.3	

CROSS SECTION RIVER: Outlet  
REACH: 1 RS: 238.666<sup>4</sup>

## INPUT

## Description:

Station Elevation Data num= 21

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
921.25	49.04	999.19	46.22	1017.75	45.69	1065.13	45.64	1175	45.54
1520.14	45.12	1539.74	45.1	1588.26	45.04	1621.43	45.04	1647.57	45.04
1653.29	45.04	1916.33	47.7	1917	47.7	1929	45.04	1932.33	43.7
1952.67	43.7	1956	43.7	1968.67	43.7	1976	45.04	2113.17	46.57
2411.74	48.58								

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
921.25	.068	1916.33	.04	1976	.045	2411.74	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.

Left	Right	Left	Channel	Right	Coeff	Contr.	Expan.
1916.33	1976	63.5	92.46	92.14	.1	.3	

CROSS SECTION RIVER: Outlet  
REACH: 1 RS: 238.5<sup>4</sup>

## INPUT

## Description:

Station Elevation Data num= 21

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
940.93	48.37	1010.14	45.75	1026.62	45.34	1068.69	45.28	1166.25	45.12
1472.71	44.49	1490.12	44.46	1533.2	44.37	1562.65	44.37	1585.86	44.37
1590.94	44.37	1824.5	46.37	1825	46.37	1834	44.37	1836.5	43.37
1867	43.37	1872	43.37	1891	43.37	1896.5	44.37	2059.13	46.07
213.11	47.78								

Manning's n Values num= 4

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
940.93	.062	1824.5	.04	1896.5	.045	2413.11	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 1824.5 1896.5 63.5 92.46 92.14 .1 .3

CROSS SECTION RIVER: Outlet  
 REACH: 1 RS: 238.333\*

## INPUT

Description:  
 Station Elevation Data num= 21  

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
960.62	47.7	1021.09	45.29	1035.49	45	1072.25	44.91	1157.5	44.7
1425.28	43.87	1440.49	43.82	1478.13	43.7	1503.87	43.7	1524.15	43.7
1528.59	43.7	1732.67	45.04	1733	45.04	1739	43.7	1740.67	43.04
1781.33	43.04	1788	43.04	1813.33	43.04	1817	43.7	2005.08	45.58
2414.48	46.99								

Manning's n Values num= 4  

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
960.62	.057	1732.67	.04	1817	.045	2414.48	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 1732.67 1817 63.5 92.46 92.14 .1 .3

CROSS SECTION RIVER: Outlet  
 REACH: 1 RS: 238.166\*

## INPUT

Description:  
 Station Elevation Data num= 21  

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
980.31	47.04	1032.05	44.83	1044.37	44.66	1075.82	44.55	1148.75	44.29
1377.85	43.24	1390.86	43.18	1423.07	43.04	1445.08	43.04	1462.44	43.04
1466.23	43.04	1640.83	43.7	1641	43.7	1644	43.04	1644.83	42.7
1695.67	42.7	1704	42.7	1735.67	42.7	1737.5	43.04	1951.04	45.08
2415.85	46.19								

Manning's n Values num= 4  

Sta	n Val	Sta	n Val	Sta	n Val	Sta	n Val
980.31	.051	1640.83	.04	1737.5	.045	2415.85	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 1640.83 1737.5 63.5 92.46 92.14 .1 .3

CROSS SECTION RIVER: Outlet  
 REACH: 1 RS: 238

## INPUT

Description:  
 Station Elevation Data num= 10  

Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev	Sta	Elev
1000	46.37	1043	44.37	1140	43.87	1368	42.37	1549	42.37
1610	42.37	1620	42.37	1658	42.37	1897	44.58	2417.22	45.4

Manning's n Values num= 3  

Sta	n Val	Sta	n Val	Sta	n Val
1000	.045	1549	.04	1658	.045

Bank Sta: Left Right Lengths: Left Channel Right Coeff Contr. Expan.  
 1549 1658 0 0 0 .1 .3

## CROSS SECTION OUTPUT Profile #PF 1

			Left OB	Channel	Right OB
E.G. Elev (ft)	45.19	Element	0.045	0.040	0.045
Vel Head (ft)	0.32	Wt. n-Val.			
W.S. Elev (ft)	44.87	Reach Len. (ft)	926.94	272.50	360.08
Crit W.S. (ft)	44.31	Flow Area (sq ft)	926.94	272.50	360.08
E.G. Slope (ft/ft)	0.007540	Area (sq ft)	3923.56	1619.09	927.35
Q Total (cfs)	6470.00	Flow (cfs)	516.75	109.00	422.98
Top Width (ft)	1048.73	Top Width (ft)	4.23	5.94	2.58
Vel Total (ft/s)	4.15	Avg. Vel. (ft/s)	1.79	2.50	0.85
Max Chl Dpth (ft)	2.50	Hydr. Depth (ft)	45185.9	18646.4	10679.8
Conv. Total (cfs)	74512.1	Conv. (cfs)	516.77	109.00	422.99
Length Wtd. (ft)		Wetted Per. (ft)	0.84	1.18	0.40
Min Ch El (ft)	42.37	Shear (lb/sq ft)	3.57	6.99	1.03
Alpha	1.20	Stream Power (lb/ft s)			
Frcn Loss (ft)		Cum Volume (acre-ft)			
C & E Loss (ft)		Cum SA (acres)			

## SUMMARY OF MANNING'S N VALUES

Reach: 1 River Sta. n1 n2 n3 n4  
 1 242 .013 .013 .013

1	241.5	.04	.028	.04	
1	241.375 <sup>A</sup>	.041	.032	.04	.04
1	241.25 <sup>A</sup>	.043	.036	.04	.04
1	241.125 <sup>A</sup>	.044	.041	.04	.04
1	241	.045	.045	.04	
1	240	.045	.045	.04	
1	239.75 <sup>A</sup>	.054	.044	.041	.041
1	239.5 <sup>A</sup>	.062	.043	.043	.043
1	239.25 <sup>A</sup>	.071	.041	.044	.044
1	239	.08	.04	.045	
1	238.833 <sup>A</sup>	.074	.04	.045	.045
1	238.666 <sup>A</sup>	.068	.04	.045	.045
1	238.5 <sup>A</sup>	.062	.04	.045	.045
1	238.333 <sup>A</sup>	.057	.04	.045	.045
1	238.166 <sup>A</sup>	.051	.04	.045	.045
1	238	.045	.04	.045	

## SUMMARY OF REACH LENGTHS

River: Outlet

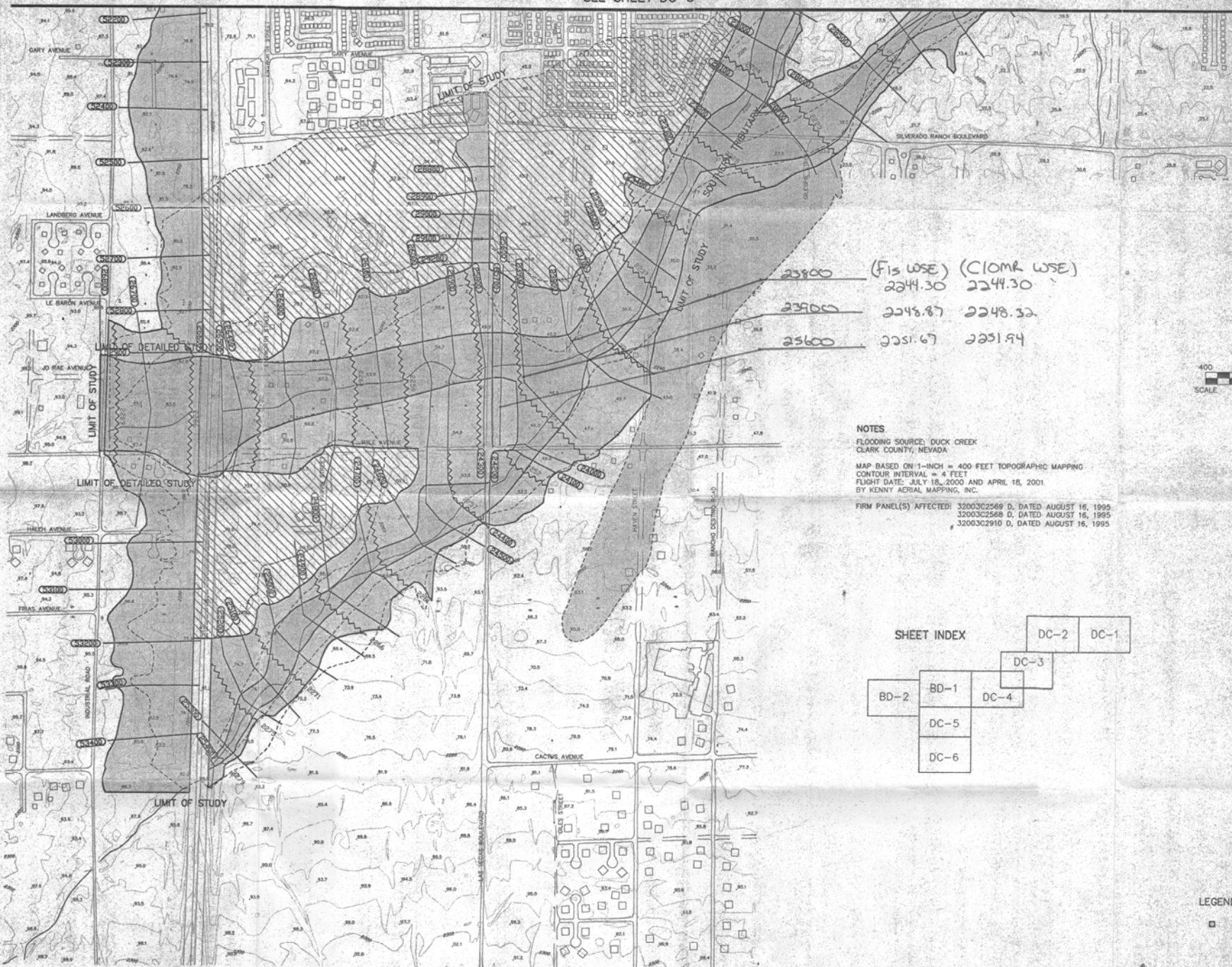
Reach	River Sta.	Left	Channel	Right
1	242	77	77	77
1	241.5	58.85	87.57	78.26
1	241.375 <sup>A</sup>	58.85	87.57	78.26
1	241.25 <sup>A</sup>	58.85	87.57	78.26
1	241.125 <sup>A</sup>	58.85	87.57	78.26
1	241	51.4	90.06	83.44
1	240	101.5	87.25	84
1	239.75 <sup>A</sup>	101.5	87.25	84
1	239.5 <sup>A</sup>	101.5	87.25	84
1	239.25 <sup>A</sup>	101.5	87.26	84
1	239	63.5	92.46	92.14
1	238.833 <sup>A</sup>	63.5	92.46	92.14
1	238.666 <sup>A</sup>	63.5	92.46	92.14
1	238.5 <sup>A</sup>	63.5	92.46	92.14
1	238.333 <sup>A</sup>	63.5	92.46	92.14
1	238.166 <sup>A</sup>	63.5	92.46	92.14
1	238	0	0	0

## SUMMARY OF CONTRACTION AND EXPANSION COEFFICIENTS

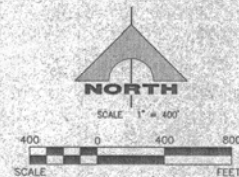
River: Outlet

Reach	River Sta.	Contr.	Expan.
1	242	.1	.3
1	241.5	.1	.3
1	241.375 <sup>A</sup>	.1	.3
1	241.25 <sup>A</sup>	.1	.3
1	241.125 <sup>A</sup>	.1	.3
1	241	.1	.3
1	240	.1	.3
1	239.75 <sup>A</sup>	.1	.3
1	239.5 <sup>A</sup>	.1	.3
1	239.25 <sup>A</sup>	.1	.3
1	239	.1	.3
1	238.833 <sup>A</sup>	.1	.3
1	238.666 <sup>A</sup>	.1	.3
1	238.5 <sup>A</sup>	.1	.3
1	238.333 <sup>A</sup>	.1	.3
1	238.166 <sup>A</sup>	.1	.3
1	238	.1	.3

SEE SHEET DC-5



23800	(FIS WSE)	(CIOMR WSE)
	2244.30	2244.30
23900	2248.87	2248.32
24000	2251.67	2251.94



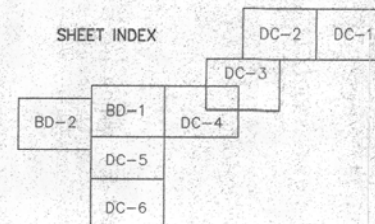
NOTES

FLOODING SOURCE: DUCK CREEK  
CLARK COUNTY, NEVADA

MAP BASED ON 1-INCH = 400 FEET TOPOGRAPHIC MAPPING  
CONTOUR INTERVAL = 4 FEET  
FLIGHT DATE: JULY 18, 2000 AND APRIL 18, 2001  
BY KENNY AERIAL MAPPING, INC.

FIRM PANEL(S) AFFECTED: 32003C2569 D, DATED AUGUST 16, 1995  
32003C2568 D, DATED AUGUST 16, 1995  
32003C2910 D, DATED AUGUST 16, 1995

SHEET INDEX



LEGEND

□ ELEVATION REFERENCE MARKER

RECEIVED  
FEB 01 2002  
STANTEC

FEB 05 2002

North Box Flows 6



EASEMENTS AND EXCEPTIONS

LAND TITLE OF NEVADA, INC., PRELIMINARY TITLE REPORT NO. 00218578 DJG, DATED: SEPTEMBER 11, 2000

INDICATES PLOTTABLE EASEMENTS

EXCEPTIONS

THE FOLLOWING ITEMS AFFECT ALL PARCELS:

THE TERMS EASEMENTS AND PROVISIONS, CONTAINED IN AN INSTRUMENT ENTITLED : DECISION RIGHT-OF-WAY GRANTED  
RECORDED : JANUARY 2, 1970  
BOOK NO. : 1  
DOCUMENT NO. : 204, OFFICIAL RECORDS

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

(AFFECTS A PORTION OF SAID LAND)

THE TERMS AND PROVISIONS, CONTAINED IN AN INSTRUMENT ENTITLED : RESOLUTION RELATIVE TO ACQUISITION OF RIGHTS-OF WAY  
RECORDED : NOVEMBER 6, 1975  
BOOK NO. : 568  
DOCUMENT NO. : 527123, OFFICIAL RECORDS

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

THE TERMS AND PROVISIONS, CONTAINED IN AN INSTRUMENT ENTITLED : RESOLUTION OF RELINQUISHMENT  
RECORDED : JANUARY 21, 1982  
BOOK NO. : 1513  
DOCUMENT NO. : 1472121, OFFICIAL RECORDS

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

ANY BOUNDARY DISCREPANCIES OR RIGHTS WHICH MAY EXIST OR ARISE BY REASON OF A RECORD OF SURVEY:  
FILE : 51  
PAGE : 46  
RECORDED : MAY 3, 1989  
BOOK NO. : 890503  
DOCUMENT NO. : 00506, OFFICIAL RECORDS

ANY BOUNDARY DISCREPANCIES OR RIGHTS WHICH MAY EXIST OR ARISE BY REASON OF A RECORD OF SURVEY:  
FILE : 92  
PAGE : 48  
RECORDED : NOVEMBER 18, 1997  
BOOK NO. : 971118  
DOCUMENT NO. : 00979, OFFICIAL RECORDS

ANY BOUNDARY DISCREPANCIES OR RIGHTS WHICH MAY EXIST OR ARISE BY REASON OF A RECORD OF SURVEY:  
FILE : 100  
PAGE : 65  
RECORDED : FEBRUARY 17, 1999  
BOOK NO. : 990217  
DOCUMENT NO. : 02313, OFFICIAL RECORDS

ANY BOUNDARY DISCREPANCIES OR RIGHTS WHICH MAY EXIST OR ARISE BY REASON OF A RECORD OF SURVEY:  
FILE : 102  
PAGE : 16  
RECORDED : APRIL 26, 1999  
BOOK NO. : 990426  
DOCUMENT NO. : 00439, OFFICIAL RECORDS

SUBJECT TO THE RIGHTS OF THE PUBLIC TO USE THAT PORTION OF SAID LAND LYING WITHIN LAS VEGAS BOULEVARD SOUTH, STATE HIGHWAY 91-466

THE FOLLOWING ITEMS AFFECT PARCEL 1:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : AUGUST 22, 1997  
BOOK NO. : 970822  
DOCUMENT NO. : 00851, OFFICIAL RECORDS  
A RIGHT-OF-WAY FOR A FEDERAL AID HIGHWAY, EXACT LOCATION UNDISCLOSED, AS REFERRED TO IN PATENT FROM THE UNITED STATES OF AMERICA, RECORDED AUGUST 22, 1997 IN BOOK 970822 OF OFFICIAL RECORDS AS DOCUMENT NO. 00851.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

ANY EASEMENT FOR THE CONSTRUCTION AND MAINTENANCE OF WATER PIPELINES AND INCIDENTAL PURPOSES, SAID EASEMENT BEING DISCLOSED BY THAT RELINQUISHMENT OF TEMPORARY CONSTRUCTION EASEMENT RECORDED JANUARY 20, 1996, IN BOOK 980120, OF OFFICIAL RECORDS, AS INSTRUMENT NO. 00518.

THE FOLLOWING ITEMS AFFECT PARCEL 2:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : OCTOBER 2, 1961  
BOOK NO. : 321  
DOCUMENT NO. : 259307, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

ANY BOUNDARY DISCREPANCIES OR RIGHTS WHICH MAY EXIST OR ARISE BY REASON OF A RECORD OF SURVEY:  
FILE : 51  
PAGE : 46  
RECORDED : MAY 3, 1989  
BOOK NO. : 890503  
DOCUMENT NO. : 00056, OFFICIAL RECORDS

AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR : PIPELINES  
IN FAVOR OF : SOUTHERN NEVADA WATER AUTHORITY  
RECORDED : JUNE 13, 1996  
BOOK NO. : 960613  
DOCUMENT NO. : 01347, OFFICIAL RECORDS  
REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

THE FOLLOWING ITEMS AFFECT PARCEL 3:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : FEBRUARY 27, 1961  
DOCUMENT NO. : 230170, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

EASEMENTS AND EXCEPTIONS (CONTINUED)

ANY EASEMENT OR LESSER RIGHT FOR THE PURPOSES STATED HEREIN INCLUDING INCIDENTAL PURPOSES DISCLOSED BY A SURVEY OR INSPECTION OF SAID LAND:  
FOR : ROADWAY  
AFFECTS : THE NORTHWEST CORNER OF SAID LAND

AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR : PIPELINES  
IN FAVOR OF : SOUTHERN NEVADA WATER AUTHORITY  
RECORDED : NOVEMBER 22, 1996  
BOOK NO. : 961122  
DOCUMENT NO. : 00799, OFFICIAL RECORDS  
REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

THE FOLLOWING ITEMS AFFECT PARCEL 4:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : JULY 12, 1984  
BOOK NO. : 1954  
DOCUMENT NO. : 1913797, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

ANY BOUNDARY DISCREPANCIES OR RIGHTS WHICH MAY EXIST OR ARISE BY REASON OF A RECORD OF SURVEY:  
FILE : 51  
PAGE : 46  
RECORDED : MAY 3, 1989  
BOOK NO. : 890503  
DOCUMENT NO. : 00506, OFFICIAL RECORDS

AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR : PIPELINES  
IN FAVOR OF : SOUTHERN NEVADA WATER AUTHORITY  
RECORDED : AUGUST 21, 1996  
BOOK NO. : 960821  
DOCUMENT NO. : 01300, OFFICIAL RECORDS  
REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

THE FOLLOWING ITEMS AFFECT PARCEL 5:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : DECEMBER 5, 1960  
BOOK NO. : 271  
DOCUMENT NO. : 219946, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR : ELECTRIC TRANSMISSION AND/OR DISTRIBUTION LINE OR SYSTEM  
IN FAVOR OF : AMARGOSA VALLEY COOPERATIVE, INC., A COOPERATIVE CORPORATION  
RECORDED : JUNE 3, 1985  
BOOK NO. : 2119  
DOCUMENT NO. : 2078853, OFFICIAL RECORDS  
REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.  
(BLANKET EASEMENT)

AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR : PIPELINES  
IN FAVOR OF : SOUTHERN NEVADA WATER AUTHORITY  
RECORDED : AUGUST 7, 1996  
BOOK NO. : 960807  
DOCUMENT NO. : 00433, OFFICIAL RECORDS  
REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR : TEMPORARY CONSTRUCTION PURPOSES  
IN FAVOR OF : SOUTHERN NEVADA WATER AUTHORITY  
RECORDED : AUGUST 7, 1996  
BOOK NO. : 960807  
DOCUMENT NO. : 00434, OFFICIAL RECORDS  
REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

A DEED OF TRUST TO SECURE INDEBTEDNESS OF THE AMOUNT STATED HEREIN AND ANY OTHER AMOUNTS PAYABLE UNDER THE TERMS THEREOF:  
DATED : DECEMBER 29, 1997  
TRUSTOR : PERMA-BILT, A NEVADA CORPORATION  
TRUSTEE : LAND TITLE OF NEVADA, INC., A NEVADA CORPORATION  
BENEFICIARY : JOE GUSKI, A WIDOWER  
AMOUNT : \$150,000.00  
RECORDED : JANUARY 9, 1998  
BOOK NO. : 860109  
DOCUMENT NO. : 00231, OFFICIAL RECORDS

THE FOLLOWING ITEM AFFECT PARCEL 6:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : JUNE 13, 1961  
BOOK NO. : 302  
DOCUMENT NO. : 244662, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
A RIGHT-OF-WAY FOR A FEDERAL AID HIGHWAY, EXACT LOCATION UNDISCLOSED, AS REFERRED TO IN PATENT FROM THE UNITED STATES OF AMERICA, RECORDED JUNE 13, 1961 IN BOOK 302 OF OFFICIAL RECORDS AS DOCUMENT NO. 244662.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

THE FOLLOWING ITEMS AFFECT PARCELS 7 THROUGH 14:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : AUGUST 22, 1997  
BOOK NO. : 970822  
DOCUMENT NO. : 00851, OFFICIAL RECORDS  
A RIGHT-OF-WAY FOR A FEDERAL AID HIGHWAY, EXACT LOCATION UNDISCLOSED, AS REFERRED TO IN PATENT FROM THE UNITED STATES OF AMERICA, RECORDED AUGUST 22, 1997 IN BOOK 970822 OF OFFICIAL RECORDS AS DOCUMENT NO. 00851.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

EASEMENTS AND EXCEPTIONS (CONTINUED)

THE FOLLOWING ITEMS AFFECT PARCEL 15:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : JUNE 18, 1976  
BOOK NO. : 632  
DOCUMENT NO. : 591272, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

A DEED OF TRUST TO SECURE INDEBTEDNESS OF THE AMOUNT STATED HEREIN AND ANY OTHER AMOUNTS PAYABLE UNDER THE TERMS THEREOF:  
DATED : JULY 23, 1996  
TRUSTOR : PERMA-BILT, A NEVADA CORPORATION  
TRUSTEE : LAND TITLE OF NEVADA, INC., A NEVADA CORPORATION  
BENEFICIARY : SUSAN L. YUSI MANION  
AMOUNT : \$90,000.00  
RECORDED : JULY 31, 1996  
BOOK NO. : 9607231  
DOCUMENT NO. : 02206, OFFICIAL RECORDS

THE FOLLOWING ITEMS AFFECT PARCEL 16:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : AUGUST 31, 1976  
BOOK NO. : 655  
DOCUMENT NO. : 614938, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

A DEED OF TRUST TO SECURE INDEBTEDNESS OF THE AMOUNT STATED HEREIN AND ANY OTHER AMOUNTS PAYABLE UNDER THE TERMS THEREOF:  
DATED : JULY 1, 1996  
TRUSTOR : PERMA-BILT, A NEVADA CORPORATION  
TRUSTEE : LAND TITLE OF NEVADA, INC., A NEVADA CORPORATION  
BENEFICIARY : NORMA VANGELOS, SUCCESSOR TRUSTEE OF THE ROBERT FREDERICK PICARDO TRUST U/A DATED APRIL 23, 1991  
AMOUNT : \$88,000.00  
RECORDED : AUGUST 9, 1996  
BOOK NO. : 960809  
DOCUMENT NO. : 00284, OFFICIAL RECORDS

THE FOLLOWING ITEMS AFFECT PARCEL 17:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : JANUARY 15, 1997  
BOOK NO. : 970115  
DOCUMENT NO. : 00357, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

THE FOLLOWING ITEMS AFFECT PARCEL 18:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : JULY 5, 1961  
BOOK NO. : 306  
DOCUMENT NO. : 247681, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
A RIGHT-OF-WAY FOR A FEDERAL AID HIGHWAY, EXACT LOCATION UNDISCLOSED, AS REFERRED TO IN PATENT FROM THE UNITED STATES OF AMERICA, RECORDED JULY 5, 1961 IN BOOK 306 OF OFFICIAL RECORDS AS DOCUMENT NO. 247681.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

THE FOLLOWING ITEMS AFFECT PARCEL 19:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : JULY 20, 1983  
BOOK NO. : 1773  
DOCUMENT NO. : 1732959, OFFICIAL RECORDS  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

THE FOLLOWING ITEMS AFFECT PARCEL 20:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : DECEMBER 29, 1960  
BOOK NO. : 275  
DOCUMENT NO. : 222721, OFFICIAL RECORDS  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

THE FOLLOWING ITEMS AFFECT PARCEL 21:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : JANUARY 13, 1961  
BOOK NO. : 278  
DOCUMENT NO. : 225320, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
A RIGHT-OF-WAY FOR A FEDERAL AID HIGHWAY, EXACT LOCATION UNDISCLOSED, AS REFERRED TO IN PATENT FROM THE UNITED STATES OF AMERICA, RECORDED JANUARY 13, 1961 IN BOOK 278 OF OFFICIAL RECORDS AS DOCUMENT NO. 225320.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

THE FOLLOWING ITEMS AFFECT PARCEL 22:

RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : OCTOBER 2, 1961  
DOCUMENT NO. : 259308, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.  
REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.  
AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR : POWER AND COMMUNICATION  
IN FAVOR OF : NEVADA POWER COMPANY AND CENTRAL TELEPHONE COMPANY  
RECORDED : JULY 11, 1973  
BOOK NO. : 345  
DOCUMENT NO. : 304094, OFFICIAL RECORDS  
REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.



LEGAL DESCRIPTION

**PARCEL ONE (1):**  
THE NORTH HALF (N 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL TWO (2):**  
THE WEST HALF (W 1/2) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M. NEVADA.

**PARCEL THREE (3):**  
THE EAST HALF (E 1/2) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL FOUR (4):**  
THE WEST HALF (W 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL FIVE (5):**  
THE EAST HALF (E 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL SIX (6):**  
THE NORTH ONE-HALF (N 1/2) OF THE NORTH ONE-HALF (N 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M. NEVADA.

EXCEPTING THEREFROM THE NORTH FIFTY (50) FEET AS CONVEYED TO CLARK COUNTY, BY DEED RECORDED APRIL 9, 1973 IN BOOK 316 OF OFFICIAL RECORDS, CLARK COUNTY, NEVADA RECORDS, AS DOCUMENT NO. 275855.

EXCEPTING THEREFROM ANY PORTION OF SAID LAND LYING WITHIN STATE HIGHWAY 91-466.

**PARCEL SEVEN (7):**  
THE SOUTHWEST QUARTER (SW 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL EIGHT (8):**  
THE EAST HALF (E 1/2) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL NINE (9):**  
THE SOUTH HALF (S 1/2) OF THE NORTH HALF (N 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL TEN (10):**  
THE NORTH HALF (N 1/2) OF THE SOUTH HALF (S 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL ELEVEN (11):**  
THE NORTHWEST QUARTER (NW 1/4) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL TWELVE (12):**  
THE WEST HALF (W 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL THIRTEEN (13):**  
THE EAST HALF (E 1/2) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL FOURTEEN (14):**  
THE SOUTHEAST QUARTER (SE 1/4) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL FIFTEEN (15):**  
THE SOUTHWEST QUARTER (SW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

EXCEPTING THEREFROM THOSE PORTIONS DESIGNATED AS COUNTY ROADS BY CLARK COUNTY IN DOCUMENT RECORDED NOVEMBER 6, 1975 IN BOOK 568 AS DOCUMENT NO. 527123, OFFICIAL RECORDS, CLARK COUNTY, NEVADA.

**PARCEL SIXTEEN (16):**  
THE SOUTHWEST QUARTER (SW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL SEVENTEEN (17):**  
THE WEST HALF (W 1/2) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL EIGHTEEN (18):**  
THE SOUTH HALF (S 1/2) OF THE SOUTH HALF (S 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

EXCEPTING THEREFROM THOSE PORTIONS CONVEYED TO CLARK COUNTY FOR ROAD PURPOSES BY GRANT, BARGAIN, SALE DEED, RECORDED APRIL 9, 1973 IN BOOK 316 AS INSTRUMENT NO. 275855 OF OFFICIAL RECORDS.

**PARCEL NINETEEN (19):**  
THE NORTHEAST QUARTER (NE 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL TWENTY (20):**  
THE EAST HALF (E 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

**PARCEL TWENTY-ONE (21):**  
THE NORTH HALF (N 1/2) OF THE NORTH HALF (N 1/2) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

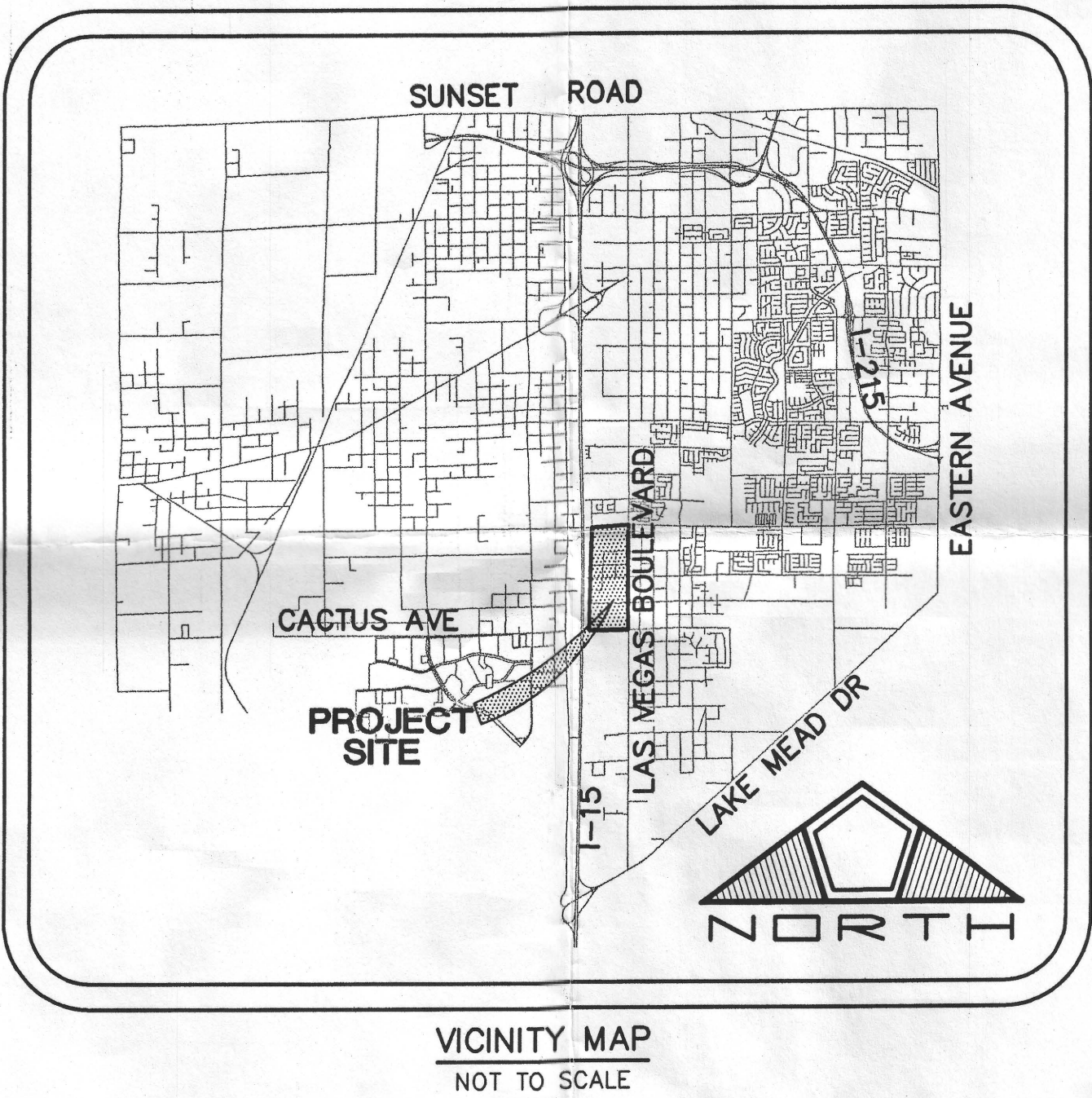
EXCEPTING THEREFROM THE NORTH THIRTY (30) FEET AS CONVEYED TO CLARK COUNTY BY DEED RECORDED APRIL 25, 1973 AS DOCUMENT NO. 280899 OF OFFICIAL RECORDS.

FURTHER EXCEPTING THEREFROM ANY PORTION LYING WITHIN STATE HIGHWAY NO. 91-466.

**PARCEL TWENTY-TWO (22):**  
THE SOUTH HALF (S 1/2) OF THE NORTH HALF (N 1/2) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

EXCEPTING THEREFROM ANY PORTION OF SAID LAND LYING WITHIN LAS VEGAS BOULEVARD (STATE HIGHWAY 91-466)

ALTA/ACSM LAND TITLE SURVEY  
A PORTION OF THE WEST HALF (W 1/2) OF SECTION 29,  
TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.M., CLARK COUNTY, NEVADA



LEGAL DESCRIPTION (CONTINUED)

**PARCEL TWENTY-THREE (23):**  
THE NORTH HALF (N 1/2) OF THE SOUTH HALF (S 1/2) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M., CLARK COUNTY, NEVADA.

**PARCEL TWENTY-FOUR (24):**  
THE SOUTH HALF (S 1/2) OF THE SOUTH HALF (S 1/2) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

EXCEPTING THEREFROM THAT PORTION LYING WITHIN HIGHWAY 91.

ALSO EXCEPTING THEREFROM THAT PORTION RELINQUISHED TO CLARK COUNTY IN RESOLUTION OF RELINQUISHMENT RECORDED JANUARY 21, 1982 IN BOOK 1618 AS DOCUMENT NO. 1472121, OFFICIAL RECORDS, CLARK COUNTY, NEVADA.

**PARCEL TWENTY-FIVE A (25 A) THROUGH TWENTY-FIVE I (25 I):**  
**PARCEL 25-A:**  
THE SOUTH HALF (S 1/2) OF THE SOUTH HALF (S 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

SUBJECT TO AN EASEMENT OVER THE WEST 30 FEET AND THE SOUTH 30 FEET TOGETHER WITH A 15 FOOT SPANDREL AREA IN THE NORTHWEST CORNER THEREOF CONCAVE SOUTHEASTERLY AND BEING TANGENT TO THE SOUTH LINE OF SAID NORTH 30 FEET AND TANGENT TO THE EAST LINE OF SAID WEST 30 FEET, ALSO TOGETHER WITH A 15 FOOT SPANDREL AREA IN THE SOUTHWEST CORNER THEREOF CONCAVE NORTHEASTERLY AND BEING TANGENT TO THE EAST LINE OF SAID SOUTH 30 FEET AND TANGENT TO THE NORTH LINE OF SAID WEST 30 FEET FOR ROAD, PUBLIC UTILITIES, AND FLOOD CONTROL PURPOSES TO INSURE CONTINUED INGRESS AND EGRESS TO ADJACENT LANDS IN FAVOR OF THE COUNTY OF CLARK, STATE OF NEVADA.

**PARCEL 25-B:**  
THE WEST HALF (W 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

SUBJECT TO AN EASEMENT OVER THE NORTH 30 FEET FOR ROAD, PUBLIC UTILITIES, AND FLOOD CONTROL PURPOSES TO INSURE CONTINUED INGRESS AND EGRESS TO ADJACENT LANDS IN FAVOR OF THE COUNTY OF CLARK, STATE OF NEVADA.

**PARCEL 25-C:**  
THE WEST HALF (W 1/2) OF THE NORTHWEST QUARTER (NW 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

SUBJECT TO AN EASEMENT OVER THE NORTH 30 FEET, THE WEST 30 FEET AND THE SOUTH 30 FEET TOGETHER WITH A 15 FOOT SPANDREL AREA IN THE NORTHWEST CORNER THEREOF CONCAVE SOUTHEASTERLY AND BEING TANGENT TO THE SOUTH LINE OF SAID NORTH 30 FEET AND TANGENT TO THE EAST LINE OF SAID WEST 30 FEET, ALSO TOGETHER WITH A 15 FOOT SPANDREL AREA IN THE SOUTHWEST CORNER THEREOF CONCAVE NORTHEASTERLY AND BEING TANGENT TO THE EAST LINE OF SAID SOUTH 30 FEET AND TANGENT TO THE NORTH LINE OF SAID WEST 30 FEET FOR ROAD, PUBLIC UTILITIES, AND FLOOD CONTROL PURPOSES TO INSURE CONTINUED INGRESS AND EGRESS TO ADJACENT LANDS IN FAVOR OF THE COUNTY OF CLARK, STATE OF NEVADA.

**PARCEL 25-D:**  
THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHWEST QUARTER (NW 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

SUBJECT TO AN EASEMENT OVER THE SOUTH 30 FEET AND THE EAST 30 FEET, TOGETHER WITH A 15 FOOT SPANDREL AREA IN THE SOUTHEAST CORNER THEREOF CONCAVE NORTHEASTERLY AND BEING TANGENT TO THE NORTH LINE OF SAID SOUTH 30 FEET AND TANGENT TO THE WEST LINE OF SAID EAST 30 FEET FOR ROAD, PUBLIC UTILITIES, AND FLOOD CONTROL PURPOSES TO INSURE CONTINUED INGRESS AND EGRESS TO ADJACENT LANDS IN FAVOR OF THE COUNTY OF CLARK, STATE OF NEVADA.

**PARCEL 25-E:**  
THE WEST HALF (W 1/2) OF THE NORTHEAST QUARTER (NE 1/4) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

SUBJECT TO AN EASEMENT OVER THE NORTH 30 FEET FOR ROAD, PUBLIC UTILITIES, AND FLOOD CONTROL PURPOSES TO INSURE CONTINUED INGRESS AND EGRESS TO ADJACENT LANDS IN FAVOR OF THE COUNTY OF CLARK, STATE OF NEVADA.

**PARCEL 25-F:**  
THE WEST HALF (W 1/2) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

SUBJECT TO AN EASEMENT OVER THE SOUTH 40 FEET FOR ROAD, PUBLIC UTILITIES, AND FLOOD CONTROL PURPOSES TO INSURE CONTINUED INGRESS AND EGRESS TO ADJACENT LANDS IN FAVOR OF THE COUNTY OF CLARK, STATE OF NEVADA.

**PARCEL 25-G:**  
THE NORTHWEST QUARTER (NW 1/4) OF THE SOUTHWEST QUARTER (SW 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF THE NORTHEAST QUARTER (NE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.B. & M.

SUBJECT TO AN EASEMENT OVER THE NORTH 30 FEET AND THE WEST 30 FEET TOGETHER WITH A 15 FOOT SPANDREL AREA IN THE NORTHWEST CORNER THEREOF CONCAVE SOUTHEASTERLY AND BEING TANGENT TO THE SOUTH LINE OF SAID NORTH 30 FEET AND TANGENT TO THE EAST LINE OF SAID WEST 30 FEET FOR ROAD, PUBLIC UTILITIES, AND FLOOD CONTROL PURPOSES TO INSURE CONTINUED INGRESS AND EGRESS TO ADJACENT LANDS IN FAVOR OF THE COUNTY OF CLARK, STATE OF NEVADA.

SURVEYOR'S COMMENTS

1. THE PROPERTY DESCRIBED HEREON IS LOCATED IN THE FOLLOWING ZONES: ZONE X AREAS OF 500-YEAR FLOOD; AREAS OF 100-YEAR FLOOD WITH AVERAGE DEPTHS OF LESS THAN 1 FOOT OR WITH DRAINAGE AREAS LESS THAN 1 SQUARE MILE; AND AREAS PROTECTED BY LEVEES FROM 100-YEAR FLOOD. ZONE A NO BASE FLOOD ELEVATIONS DETERMINED AND IN ZONE AE BASE FLOOD ELEVATIONS DETERMINED AS DEFINED BY THE FEDERAL EMERGENCY MANAGEMENT AGENCY FLOOD INSURANCE RATE MAP FOR CLARK COUNTY, NEVADA AND INCORPORATED AREAS, COMMUNITY PANEL NO. 2568 OF 4090, MAP NUMBER 32003C2568 D, EFFECTIVE DATE: AUGUST 16, 1995.
2. NET AREA = 179.46 ACRES  
PARCEL A = 0.14 ACRES  
PARCEL C = 0.83 ACRES  
PARCEL D = 0.47 ACRES  
PARCEL E = 0.18 ACRES  
PARCEL F = 0.35 ACRES  
PARCEL G = 0.14 ACRES  
PARCEL H = 0.32 ACRES  
PARCEL I = 0.14 ACRES  
PARCEL J = 0.70 ACRES  
PARCEL K = 0.74 ACRES  
GROSS AREA = 183.47 ACRES
3. NO VISIBLE EVIDENCE OF CEMETERIES OR BURIAL GROUNDS.
4. ANY BURIED UTILITIES OR PIPELINES ARE AS SHOWN PER SURFACE EVIDENCE. PENTACORE WAS NOT SUPPLIED WITH "AS-BUILT" DRAWINGS OF CONSTRUCTED UTILITY LINES. PENTACORE AND THE SURVEYOR OF RECORD WILL NOT BE LIABLE FOR THE LOCATION OR THE FAILURE TO NOTE THE LOCATION OF NONVISIBLE UTILITIES.
5. NO ATTEMPT HAS BEEN MADE TO RESEARCH UTILITY SERVICE CONNECTIONS. FOR FURTHER INFORMATION REGARDING LOCATION, DEPTH OR SIZE OF UTILITIES, CONTACT THE APPROPRIATE UTILITY COMPANY:  
CALL BEFORE YOU DIG / 1-800-227-2600  
ELECTRIC NEVADA POWER COMPANY (702) 367-5000  
6226 W. SAHARA AVE., LAS VEGAS, NEVADA  
TELEPHONE SPRINT (702) 877-7963  
330 S. VALLEY VIEW, LAS VEGAS, NEVADA  
GAS SOUTHWEST GAS (702) 365-1555  
4300 W. TROPICANA, LAS VEGAS, NEVADA  
WATER LAS VEGAS VALLEY WATER DISTRICT (702) 870-2011  
3700 W. CHARLESTON, LAS VEGAS, NEVADA  
SEWER CLARK COUNTY SANITATION DISTRICT (702) 458-1180  
5857 E. FLAMINGO, LAS VEGAS, NEVADA  
CABLE TV COX COMMUNICATIONS LAS VEGAS, INC. (702) 384-8084  
121 S. MARTIN LUTHER KING, LAS VEGAS, NEVADA
6. THE FOLLOWING PARCELS ARE ZONED (R-E) RURAL ESTATES:  
1, 2, 3, 7, 11, 12, 13, 14, 15, 16, 19, 26, 30, 33, 36, 40, 44A-44E, 48, 51, 54, 57, 58, 59, 61, 62, 63, 64, 65  
MINIMUM SETBACKS (R-E) ZONING  
FROM: 10'  
SIDE: 10'  
CORNER: 15'  
REAR: 30'  
THE FOLLOWING PARCELS ARE ZONED (R-1) LIMITED RESORT AND APARTMENT:  
4, 5, 6, 8, 9, 10, 17, 18, 20, 21, 22, 23, 24, 25A-25I, 27, 28, 29, 31, 32, 34, 35, 37, 38, 39, 41, 42, 43, 45, 46, 47, 49, 50, 52A-52D, 53, 55, 56, 60  
MINIMUM SETBACKS (R-1) ZONING  
FROM: 10'  
SIDE: 10X WIDTH OF LOT, 5' MIN-50' MAX  
CORNER: 10X WIDTH OF LOT, 20' MIN-50' MAX  
REAR: 20' ADJACENT TO RESIDENTIAL

BASIS OF BEARINGS

SOUTH 89°08'18" WEST, BEING THE BEARING OF THE SOUTH LINE OF THE SOUTHEAST QUARTER (SE 1/4) OF THE SOUTHEAST QUARTER (SE 1/4) OF SECTION 29, TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.M., CLARK COUNTY, NEVADA, AS SHOWN ON THAT CERTAIN MAP ON FILE IN THE CLARK COUNTY RECORDER'S OFFICE IN FILE 100 OF SURVEYS, AT PAGE 65.

REFERENCE MAPS

FILE 41, PAGE 28 OF SURVEYS  
FILE 51, PAGE 46 OF SURVEYS  
FILE 56, PAGE 64 OF SURVEYS  
FILE 92, PAGE 48 OF SURVEYS  
FILE 32, PAGE 45 OF PARCEL MAPS  
FILE 64, PAGE 46 OF PARCEL MAPS  
FILE 67, PAGE 39 OF PARCEL MAPS  
FILE 70, PAGE 89 OF PARCEL MAPS  
FILE 79, PAGE 36 OF PARCEL MAPS

SURVEYOR'S CERTIFICATE

TO: GREEN PARK HOLDINGS, LLC  
LAND TITLE OF NEVADA, INC

THIS IS TO CERTIFY THAT THIS PLAT AND THE SURVEY ON WHICH IT IS BASED WERE MADE IN ACCORDANCE WITH "MINIMUM STANDARD DETAIL REQUIREMENTS FOR ALTA/ACSM LAND TITLE SURVEYS," JOINTLY ESTABLISHED AND ADOPTED BY THE AMERICAN LAND TITLE ASSOCIATION, THE AMERICAN CONGRESS ON SURVEYING AND MAPPING AND THE NATIONAL SOCIETY OF PROFESSIONAL SURVEYORS IN 1999, AND INCLUDES OPTIONAL ITEMS 1, 2, 3, 4, 5, 6, 8, 10, 11, 13, 14, AND 16 OF TABLE A THEREOF, PURSUANT TO THE ACCURACY STANDARDS AS ADOPTED BY THE AMERICAN LAND TITLE ASSOCIATION, THE NATIONAL SOCIETY OF PROFESSIONAL SURVEYORS AND THE AMERICAN CONGRESS ON SURVEYING AND MAPPING AND IN EFFECT ON THE DATE THIS CERTIFICATION, UNDERSIGNED FURTHER CERTIFIES THAT THE POSITIONAL UNCERTAINTIES RESULTING FROM THE SURVEY MEASUREMENTS MADE ON THE SURVEY DO NOT EXCEED THE ALLOWABLE POSITIONAL TOLERANCE.

THE EASEMENTS AND OTHER RECORDED DATA SHOWN HEREON WERE OBTAINED FROM TITLE REPORT BY LAND TITLE OF NEVADA, INC, NO. 00218578 DUG, DATED SEPTEMBER 11, 2000. THE CERTIFYING SURVEYOR HAS NOT MADE AN INDEPENDENT TITLE SEARCH AND HAS RELIED ON THE SUPPLIED DOCUMENTS AND SPECIFICALLY DISCLAIMS ANY ITEMS NOT SHOWN WHICH MAY BE OF RECORD OR NOT OF RECORD THAT MAY AFFECT THE PROPERTY SHOWN ON THE SURVEY. THIS SURVEY WAS COMPLETED ON NOVEMBER 6, 2000.

FREDERICK W. ORBAN  
PROFESSIONAL LAND SURVEYOR  
NEVADA LICENSE NO. 4541

DATE : 11/9/00  
P.M. :  
DRAWN BY : PD  
DESIGNED BY :  
CHECKED BY : FWO  
FOR THESE PLANS: 09/11/00  
11/29/00  
11/27/00

3 RELEASED LOCATION OF EASEMENT NO. 65  
2 REISED RIGHTS-OF-WAY  
1 REVISED AREAS

PENTACORE  
CIVIL ENGINEERING • LAND SURVEYING • PLANNING  
CONSTRUCTION MANAGEMENT • ASIA CONSULTING  
10000 W. LAS VEGAS BLVD., SUITE 1000, LAS VEGAS, NEVADA 89148  
(702) 258-0015  
(702) 258-0016  
(702) 258-0017

GREEN PARK HOLDINGS, LLC  
ALTA/ACSM LAND TITLE SURVEY

SHEET  
1  
OF 10

MAR 26 2002







66 AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR : ELECTRICAL LINES  
IN FAVOR OF : NEVADA POWER COMPANY  
RECORDED : MAY 14, 1980  
BOOK NO. : 1227  
DOCUMENT NO. : 1186290, OFFICIAL RECORDS

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

167. EASEMENTS AND DEDICATIONS AND RESTRICTIONS AS PROVIDED FOR OR DELINEATED ON THE PLAT OF SAID PARCEL MAP ON FILE IN FILE 32 OF PARCEL MAPS, PAGE 45 IN THE OFFICE OF THE COUNTY RECORDER, CLARK COUNTY, NEVADA.

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

169. RESERVATIONS AS CONTAINED IN THE PATENT CONVEYING SAID LAND.  
NOTE: IT IS REQUIRED THAT THE ORIGINAL PATENT, OR CERTIFIED COPY THEREOF, BE RECORDED PRIOR TO  
THE CLOSE OF THIS TRANSACTION

173. RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : OCTOBER 2, 1962  
BOOK NO. : 390  
DOCUMENT NO. : 314830, OFFICIAL RECORDS

REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.

174. AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR : ELECTRICAL LINES

IN FAVOR OF : NEVADA POWER COMPANY  
RECORDED : NOVEMBER 30, 1983  
BOOK NO. : 1840  
DOCUMENT NO. : 1799264, OFFICIAL RECORDS

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.  
(DOES NOT AFFECTS PROPERTY)

175. ANY BOUNDARY DISCREPANCIES OR RIGHTS WHICH MAY EXIST OR ARISE BY REASON OF A RECORD OF SURVEY:

FILE : 41  
PAGE : 28  
RECORDED : FEBRUARY 27, 1984  
BOOK NO. : 1881

DOCUMENT NO. : 1840335, OFFICIAL RECORDS

176. COVENANTS, CONDITIONS AND RESTRICTIONS AND EASEMENTS, IF ANY, (BUT DELETING ANY COVENANT, CONDITION OR RESTRICTIONS INDICATING A PREFERENCE, LIMITATION OR DISCRIMINATION BASED UPON RACE, COLOR, RELIGION, SEX, HANDICAP, FAMILIAL STATUS OR NATIONAL ORIGIN TO THE EXTENT SUCH COVENANTS, CONDITIONS OR RESTRICTIONS VIOLATE 42 USC 3604 ( C )), IN AN INSTRUMENT:  
RECORDED : MAY 24, 1984  
BOOK NO. : 1926  
DOCUMENT NO. : 1885327, OFFICIAL RECORDS

DOCUMENT NO. : 1885327, OFFICIAL RECORDS

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

177. EASEMENTS AND DEDICATIONS AND RESTRICTIONS AS PROVIDED FOR OR DELINEATED ON THE PLAT OF SAID PARCEL MAP ON FILE IN FILE 70 OF PARCEL MAPS, PAGE 89 IN THE OFFICE OF THE COUNTY RECORDER, CLARK COUNTY, NEVADA.

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

178. AN EASEMENT SHOWN BELOW AND INCIDENTAL PURPOSES RELATED THERETO:  
FOR ; ELECTRICAL LINES

IN FAVOR OF : NEVADA POWER COMPANY  
RECORDED : DECEMBER 31, 1992  
BOOK NO. : 921231  
DOCUMENT NO. : 01957, OFFICIAL RECORDS

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

179. ANY EASEMENT NOT VACATED BY THAT CERTAIN ORDER OF VACATION:

RECORDED : MARCH 4, 1993  
BOOK NO. : 930304  
DOCUMENT NO. : 00550, OFFICIAL RECORDS

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

180. A DEED OF TRUST TO SECURE INDEBTEDNESS OF THE AMOUNT STATED HEREIN AND ANY OTHER AMOUNTS PAYABLE UNDER THE TERMS THEREOF:

DATED : APRIL 1, 1996  
TRUSTOR : PHIL DAVIS, A MARRIED MAN AS HIS SOLE AND SEPARATE PROPERTY  
TRUSTEE : NEVADA TITLE COMPANY, A NEVADA CORPORATION  
BENEFICIARY : RUTH D. HAMPE, TRUSTEE OF THE STEPHEN J. HAMPE AND RUTH  
DATED DECEMBER 23, 1992

AMOUNT : \$50,000.00  
RECORDED : APRIL 2, 1996  
BOOK NO. : 960402  
DOCUMENT NO. : 00217, OFFICIAL RECORDS

182. RESERVATIONS AS CONTAINED IN THE PATENT CONVEYING SAID LAND.  
NOTE: IT IS REQUIRED THAT THE ORIGINAL PATENT, OR A CERTIFIED COPY THEREOF, BE RECORDED PRIOR  
TO THE CLOSE OF THIS TRANSACTION.

- 183 EASEMENTS, TERMS AND PROVISIONS, CONTAINED IN AN INSTRUMENT  
ENTITLED : UNITED STATES, DEPARTMENT OF THE INTERIOR, BUREAU OF LAND MANAGEMENT,  
RIGHT-OF-WAY/TEMPORARY USE PERMIT  
RECORDED : AUGUST 31, 1993

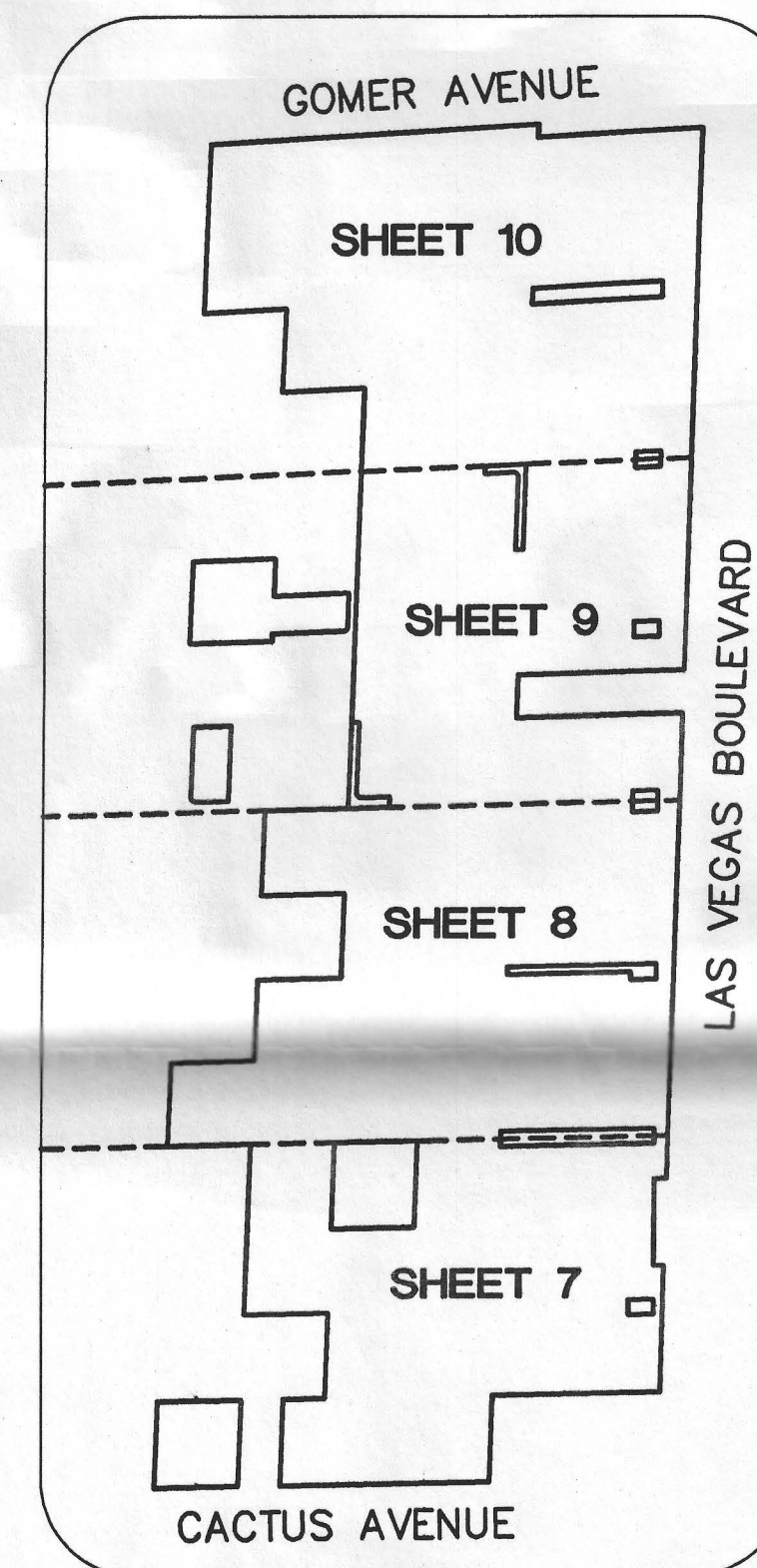
BOOK NO. : 930831  
DOCUMENT NO. : 01745, OFFICIAL RECORDS

REFERENCE TO SAID INSTRUMENT IS MADE FOR FULL PARTICULARS.

187 RESERVATIONS AND EASEMENTS IN THE PATENT FROM THE UNITED STATES OF AMERICA:  
RECORDED : APRIL 10, 1973  
BOOK NO. : 317

DOCUMENT NO. : 276264, OFFICIAL RECORDS  
NOTE: SAID PATENT FURTHER RESERVED AND IS SUBJECT TO A RIGHT-OF-WAY NOT EXCEEDING 33 FEET  
IN WIDTH FOR ROADWAY AND PUBLIC UTILITIES.  
AFFECTS : A PORTION OF SAID LAND.

REFERENCE SHOULD BE MADE TO THE RECORD FOR FULL PARTICULARS.



## KEY MAP









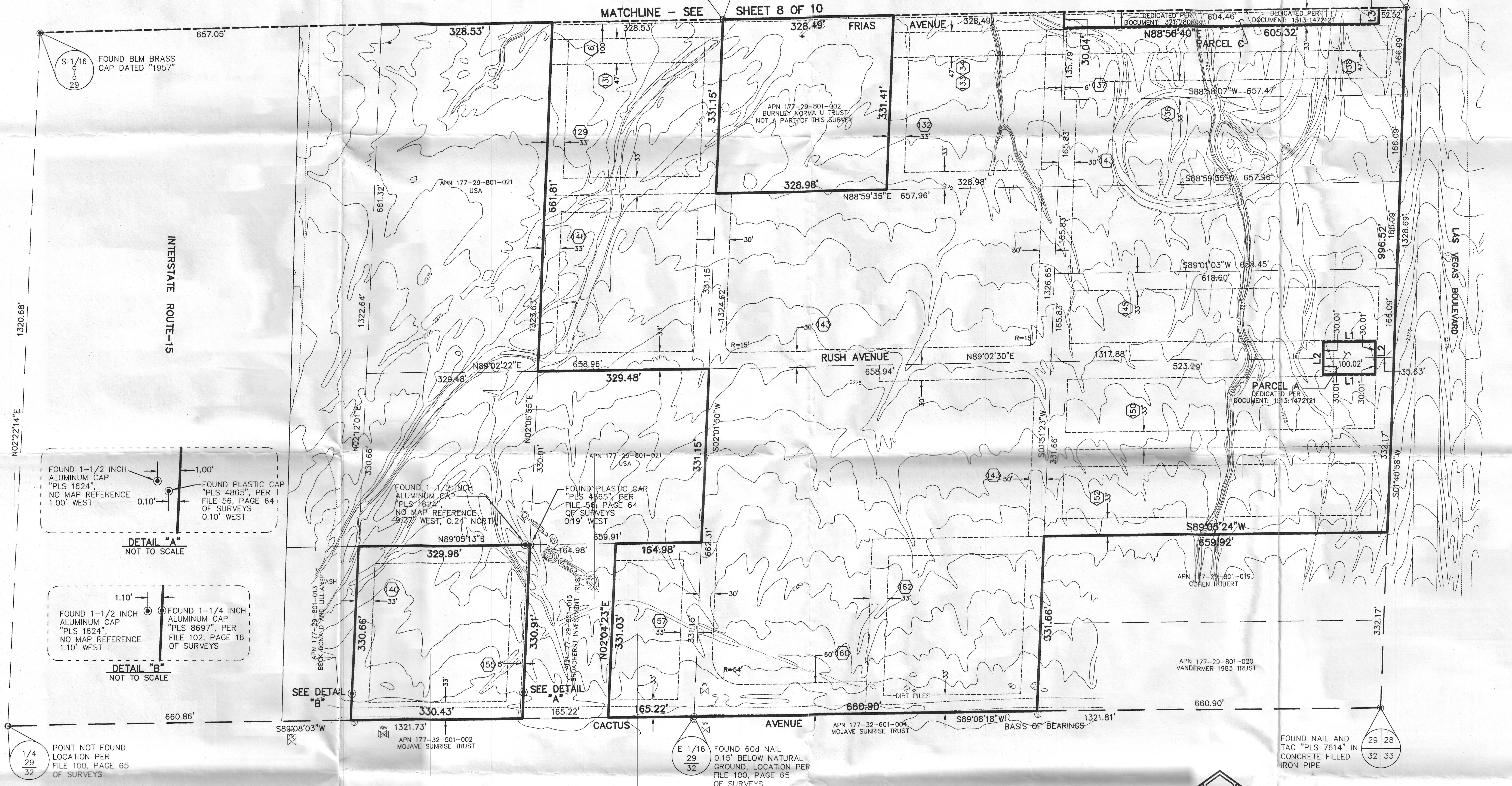


A PORTION OF THE WEST HALF (W 1/2) OF SECTION 29,  
TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.M., CLARK COUNTY, NEVADA

POINT NOT FOUND  
LOCATION PER  
FILE 92, PAGE 48  
OF SURVEYS



















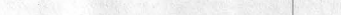


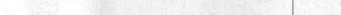



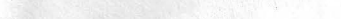

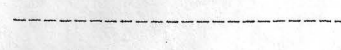


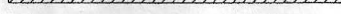
MATCHLINE - SEE SHEET 8 OF 10

FOUND NAIL & TAG  
"RLS 2051" IN A  
1-1/4" DIAMETER IRON  
PIPE



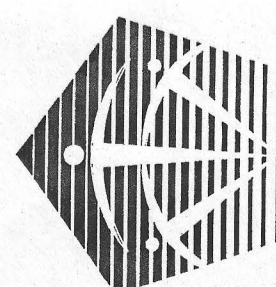
LINE	BEARING	DISTANCE
L1	N89°02'30"E	100.02'
L2	N00°13'35"E	60.01'
L3	N00°13'35"E	60.02'

NORTH  
SCALE: 1" = 100'

- |   |                        |   |  |   |       |                               |
|---|------------------------|---|--|---|-------|-------------------------------|
|  | BOUNDARY LINE          |  | FOUND MONUMENT AS DESCRIBED  |  | ELEC  | ELECTRICAL PULLBOX            |
|  | RIGHT-OF-WAY LINE      |  | POINT NOT FOUND OR SET<br>LOCATION PER FILE 92, PAGE 48<br>OF SURVEYS        |  | TELE  | TELEPHONE PULL BOX            |
|  | SECTION LINE           |  | FOUND 1-INCH PLASTIC CAP<br>"RE 2002" PER FILE 70, PAGE 89<br>OF PARCEL MAPS |  | CONC. | CONCRETE PAD                  |
|  | ALIQUOT PART LINE      |  | STREET LIGHT   |  | WMH   | WATER MANHOLE                 |
|  | EASEMENT LINE          |  | SIGN   |  | APN   | ASSESSOR'S PARCEL NUMBER      |
|  | ASSESSOR'S PARCEL LINE |  | WATER VALVE  |  | L1    | CURVE DATA                    |
|  | MATCHLINE              |  | FIRE HYDRANT   |  | 6     | COURSE NUMBER                 |
|  | EDGE OF ASPHALT        |  | POWER POLE   |   |       | TITLE REPORT EXCEPTION NUMBER |
|  | CURB AND GUTTER        |  | SEWER MANHOLE  |   |       |                               |
|  | DIRT ROAD              |  | TRANSFORMER PAD  |   |       |                               |
|  | BUILDING               |  | TRANSFORMER PAD (NO TRANSFORMER)   |   |       |                               |
|  | BLOCK WALL             |   |  |   |       |                               |
|  | CHAIN LINK FENCE       |   |  |   |       |                               |

ELEC	ELECTRICAL PULLBOX
TELE	TELEPHONE PULL BOX
CONC.	CONCRETE PAD
WMH	WATER MANHOLE
APN	ASSESSOR'S PARCEL NUMBER
④	CURVE DATA
L1	CURVE NUMBER
⑥	TITLE REPORT EXCEPTION NUMBER

# PENTACORE



CIVIL ENGINEERING • LAND SURVEYING • PLANNING  
CONSTRUCTION MANAGEMENT • ADA CONSULTING  
6763 WEST CHARLESTON BOULEVARD  
LAS VEGAS, NEVADA 89146 (702)258-0115

GREEN PARK HOLDINGS, INC  
ALTA/ACSM LAND TITLE SURVEY

SHEET

7

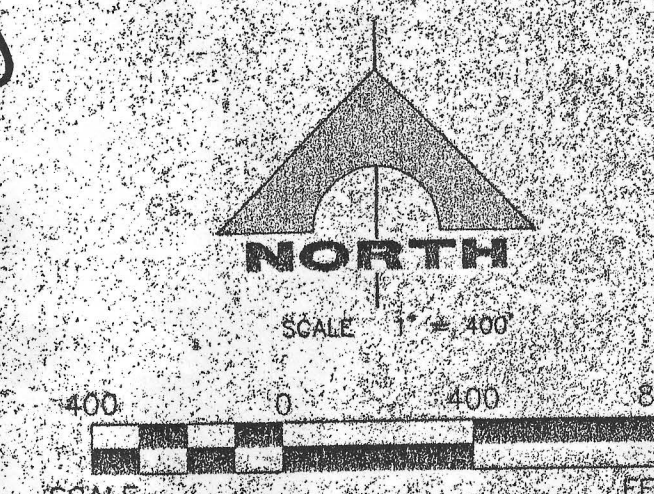
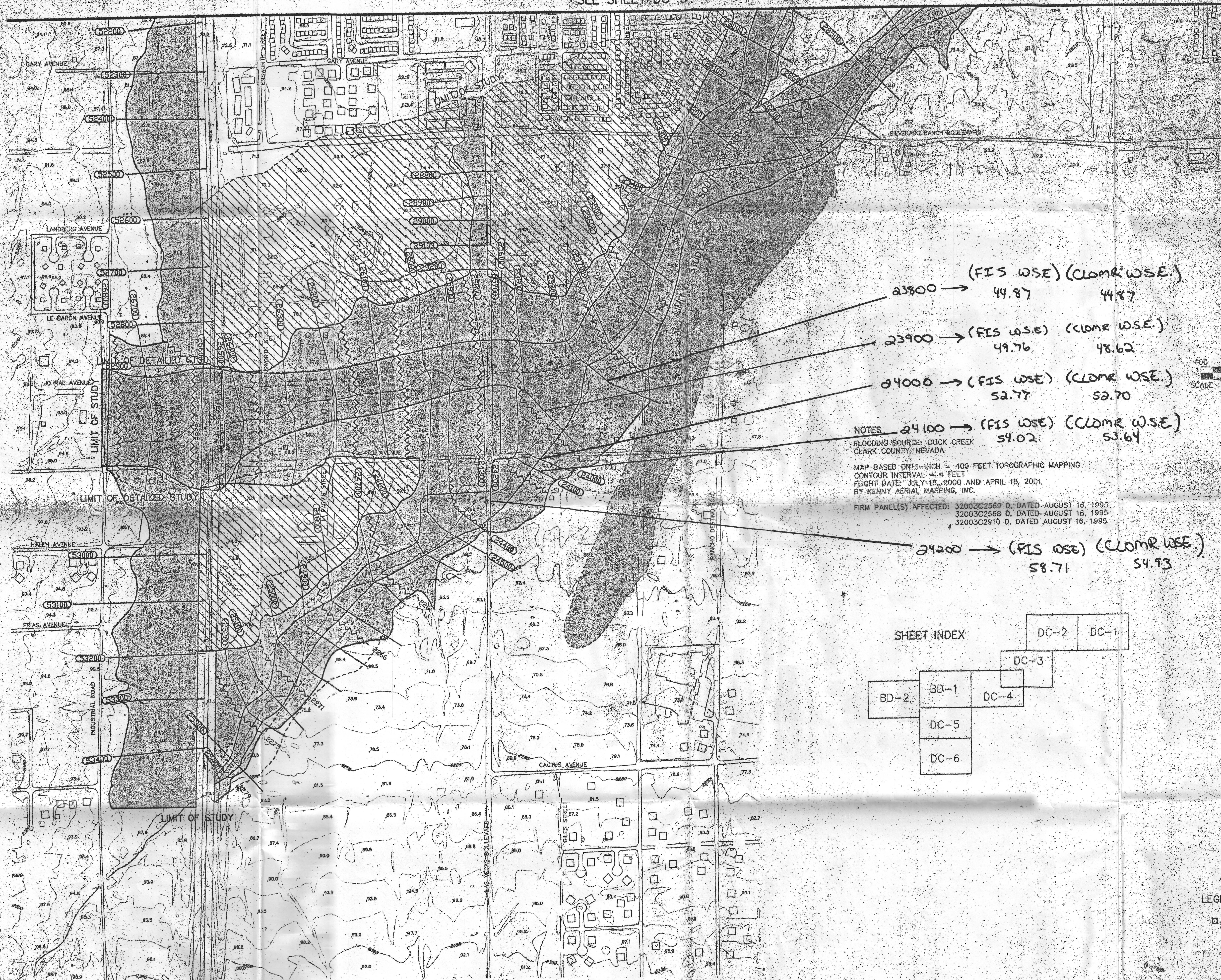
MAR 26 2002

2	REVISED LOCATION OF EX
1	REVISED AREAS

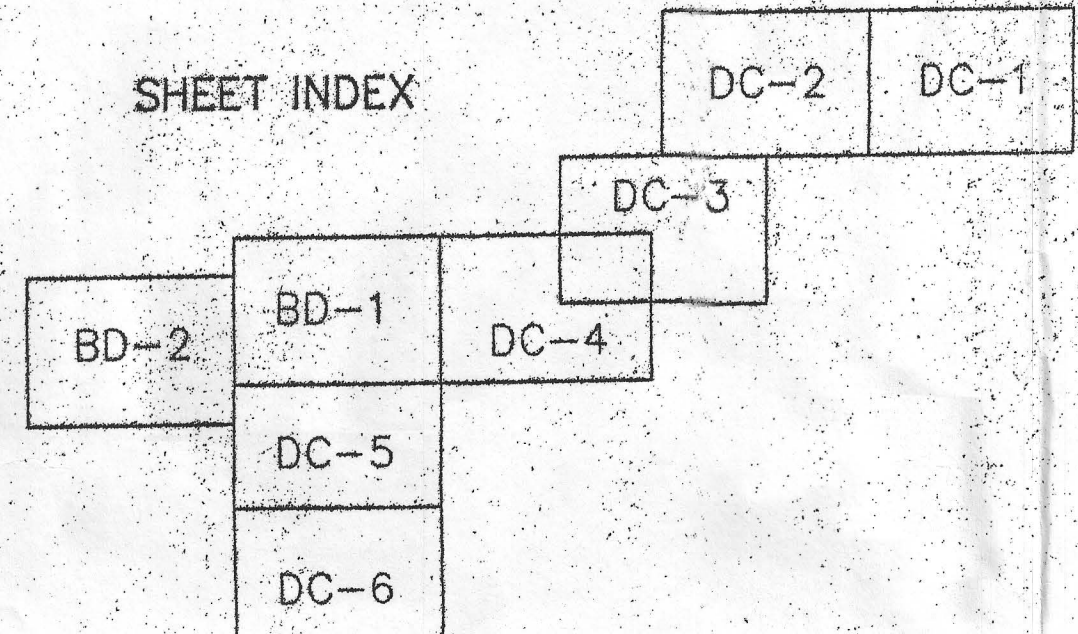
DATE :	11/5/00
P.M. :	
DRAWN BY :	PD
DESIGNED BY :	
CHECKED BY :	FWO
JOB NO. :	0288.0020



SEE SHEET DC-5



NOTES: 24100 → (FIS WSE) (CLOMR WSE.) 54.02 53.64  
FLOODING SOURCE: DUCK CREEK  
CLARK COUNTY, NEVADA  
MAP BASED ON 1-INCH = 400-FOOT TOPOGRAPHIC MAPPING  
CONTOUR INTERVAL = 4 FEET  
FLIGHT DATE: JULY 18, 2000 AND APRIL 18, 2001  
BY KENNY AERIAL MAPPING, INC.  
FIRM PANEL(S) AFFECTED: 32003C2569 D, DATED AUGUST 16, 1995  
32003C2568 D, DATED AUGUST 16, 1995  
32003C2910 D, DATED AUGUST 16, 1995



LEGEND  
□ ELEVATION REFERENCE MARKER

G.C. WALLACE, INC.  
 Engineers/Planners/Surveyors  
 1555 SOUTH RAINBOW BLVD., LAS VEGAS, NEVADA 89108  
 PHONE: (702) 735-1111  
 FAX: (702) 735-1112  
 E-MAIL: GFWALLACE@GFWALLACE.COM  
 WWW.GFWALLACE.COM

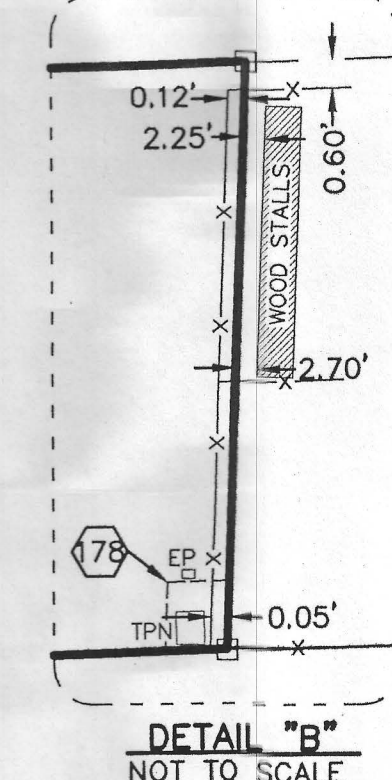
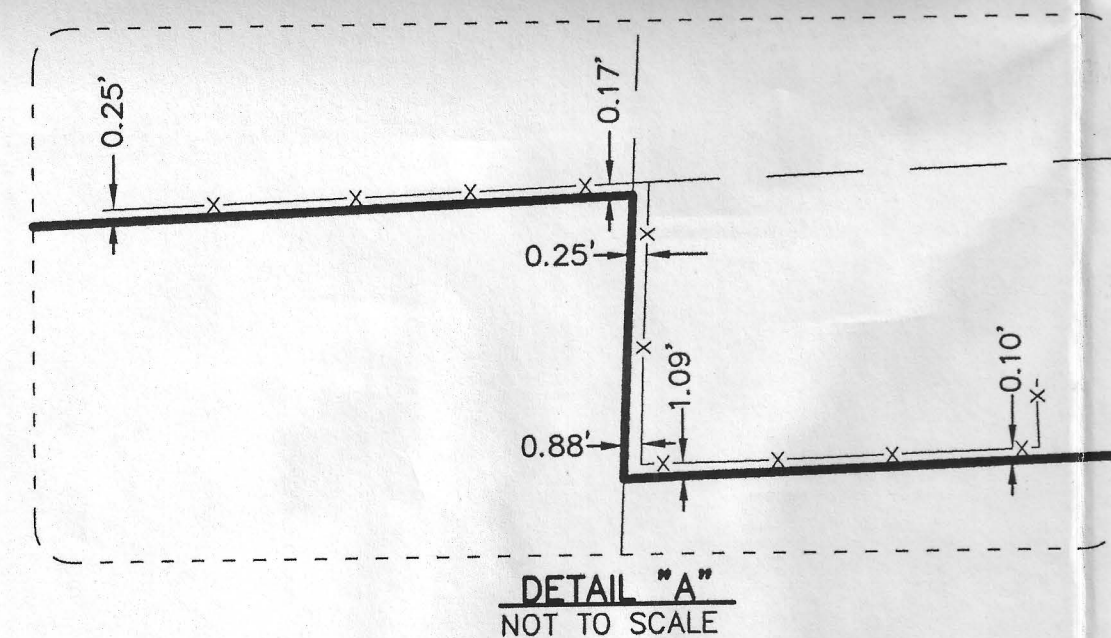
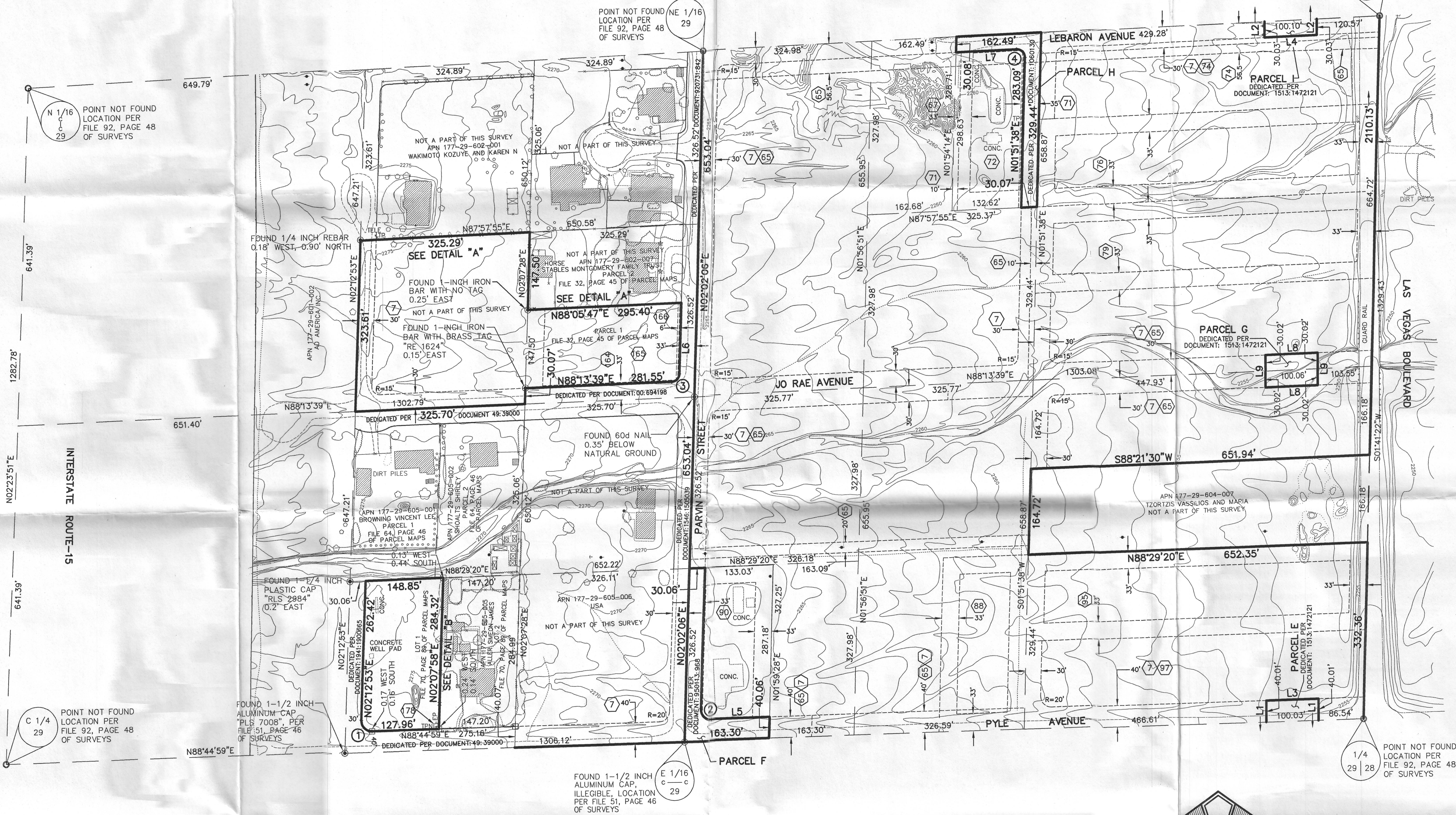
RECEIVED  
 FEB 01 2002  
 STANTEC

SHEET  
 DC-6  
 6 OF 8



# ALTA/ACSM LAND TITLE SURVEY

A PORTION OF THE WEST HALF (W 1/2) OF SECTION 29,  
TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.M., CLARK COUNTY, NEVADA

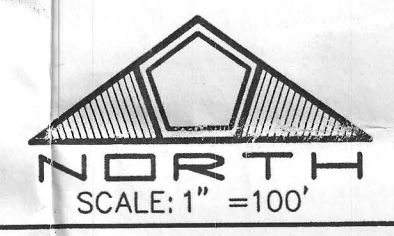


LINE	BEARING	DISTANCE
L1	N00°13'35"E	80.03'
L2	S00°13'35"W	60.06'
L3	N88°44'59"E	100.03'
L4	N87°42'09"E	100.10'
L5	N88°44'59"E	112.03'
L6	N02°02'06"E	134.12'
L7	N87°42'09"E	116.30'
L8	N88°13'39"E	100.06'
L9	N00°13'35"E	60.04'

CURVE	DELTA	RADIUS	LENGTH	TANGENT
1	93°27'54"	20.00'	32.63'	21.25'
2	93°17'08"	20.00'	32.56'	21.18'
3	86°11'32"	15.00'	22.57'	14.03'
4	94°09'29"	15.00'	24.65'	16.13'

- LEGEND**
- BOUNDARY LINE  
RIGHT-OF-WAY LINE  
SECTION LINE  
ALIQUOT PART LINE  
EASEMENT LINE  
ASSESSOR'S PARCEL LINE  
MATCHLINE  
EDGE OF ASPHALT  
CURB AND GUTTER  
DIRT ROAD  
BUILDING  
BLOCK WALL  
CHAIN LINK FENCE
- FOUND MONUMENT AS DESCRIBED  
POINT NOT FOUND OR SET LOCATION PER FILE 92, PAGE 48 OF SURVEYS  
FOUND 1-INCH PLASTIC CAP "RE 2002" PER FILE 70, PAGE 89 OF PARCEL MAPS  
STREET LIGHT  
SIGN  
WATER VALVE  
FIRE HYDRANT  
POWER POLE  
SEWER MANHOLE  
TRANSFORMER PAD  
TRANSFORMER PAD (NO TRANSFORMER)
- ELEC ELECTRICAL PULLBOX  
TELE TELEPHONE PULL BOX  
CONC. CONCRETE PAD  
WMH WATER MANHOLE  
APN ASSESSOR'S PARCEL NUMBER  
L1 CURVE DATA  
L4 COURSE NUMBER  
6 TITLE REPORT EXCEPTION NUMBER



DATE: 11/6/00

P.M.: 11/6/00

DRAWN BY: PD

DESIGNED BY: FNO

CHECKED BY: 1

JOB NO.: 1

SCALE: 1"=100'

03/11/02

11/27/00

11/09/00

APPROVED

2

REVISION LOCATION OF EXCEPTION NO. 65

1

REVISED AREAS

0

ORIGINAL CERTIFICATION

03/11/02

11/27/00

11/09/00

DATE

GREEN PARK HOLDINGS, LLC

ALTA/ACSM LAND TITLE SURVEY

PENTACORE

CIVIL ENGINEERING • LAND SURVEYING • PLANNING  
CONSTRUCTION MANAGEMENT • ADA CONSULTING  
6763 WEST CHARLESTON BOULEVARD  
LAS VEGAS, NEVADA 89146 (702)258-0115

9

OF 10

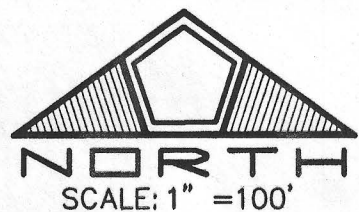
MAR 26 2002











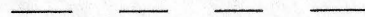











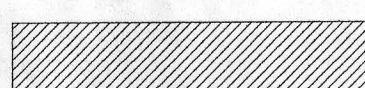


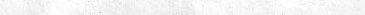


A PORTION OF THE WEST HALF (W 1/2) OF SECTION 29,  
TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.M., CLARK COUNTY, NEVADA



LINE	BEARING	DISTANCE
L1	N00°13'35"E	60.02'
L2	S00°13'35"W	80.03'
L3	N88°44'59"E	100.03'
L4	N00°13'35"E	30.01'
L5	N88°50'50"E	100.03'



- | LEGEND  |                        |   |   |                               |
|---|------------------------|---|---|-------------------------------|
|  | BOUNDARY LINE          |  FOUND MONUMENT AS DESCRIBED                   | ELEC  | ELECTRICAL PULLBOX            |
|  | RIGHT-OF-WAY LINE      |  POINT NOT FOUND OR SET                        | TELE  | TELEPHONE PULL BOX            |
|  | SECTION LINE           |  LOCATION PER FILE 92, PAGE 48 OF SURVEYS      | CONC.   | CONCRETE PAD                  |
|  | ALIQUOT PART LINE      |  FOUND 1-INCH PLASTIC CAP                      | WMH   | WATER MANHOLE                 |
|  | EASEMENT LINE          |  "RE 2002" PER FILE 70, PAGE 89 OF PARCEL MAPS | APN   | ASSESSOR'S PARCEL NUMBER      |
|  | ASSESSOR'S PARCEL LINE |  STREET LIGHT                                  |  4 | CURVE DATA                    |
|  | MATCHLINE              |  SIGN  | L1  | COURSE NUMBER                 |
|  | EDGE OF ASPHALT        |  WATER VALVE                                   |  6 | TITLE REPORT EXCEPTION NUMBER |
|  | CURB AND GUTTER        |  FIRE HYDRANT                                  |   |                               |
|  | DIRT ROAD              |  POWER POLE                                    |   |                               |
|  | BUILDING               |  SEWER MANHOLE                                 |   |                               |
|  | BLOCK WALL             | TP  | TRANSFORMER PAD   |                               |
|  | CHAIN LINK FENCE       | TPN   | TRANSFORMER PAD (NO TRANSFORMER)  |                               |



# ALTA/ACSM LAND TITLE SURVEY

A PORTION OF THE WEST HALF (W 1/2) OF SECTION 29,  
TOWNSHIP 22 SOUTH, RANGE 61 EAST, M.D.M., CLARK COUNTY, NEVADA

LINE	BEARING	DISTANCE
L1	N00°13'35"E	60.06'
L2	N87°42'09"E	100.10'

POINT NOT FOUND  
LOCATION PER  
FILE 92, PAGE 48  
OF SURVEYS

POINT NOT FOUND  
LOCATION PER  
FILE 92, PAGE 48  
OF SURVEYS

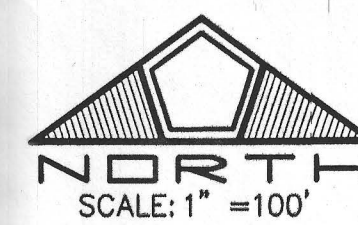
1/4  
20  
29  
FOUND USGLO BRASS  
CAP DATED 1952  
"NCL401L"



POINT NOT FOUND  
LOCATION PER  
FILE 92, PAGE 48  
OF SURVEYS

POINT NOT FOUND  
LOCATION PER  
FILE 92, PAGE 48  
OF SURVEYS

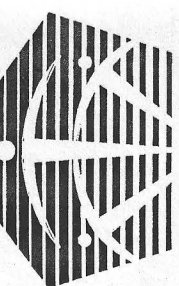
POINT NOT FOUND  
LOCATION PER  
FILE 92, PAGE 48  
OF SURVEYS



## LEGEND

BOUNDARY LINE	FOUND MONUMENT AS DESCRIBED	ELEC	ELECTRICAL PULLBOX
RIGHT-OF-WAY LINE	POINT NOT FOUND OR SET LOCATION PER FILE 92, PAGE 48 OF SURVEYS	TELE	TELEPHONE PULL BOX
SECTION LINE	FOUND 1-INCH PLASTIC CAP "RE 2002" PER FILE 70, PAGE 89 OF PARCEL MAPS	CONC.	CONCRETE PAD
ALIQUOT PART LINE	STREET LIGHT	VMH	WATER MANHOLE
EASEMENT LINE	SIGN	APN	ASSESSOR'S PARCEL NUMBER
ASSESSOR'S PARCEL LINE	WATER VALVE	④	CURVE DATA
MATCHLINE	FIRE HYDRANT	L1	COURSE NUMBER
EDGE OF ASPHALT	POWER POLE	⑥	TITLE REPORT EXCEPTION NUMBER
CURB AND GUTTER	SEWER MANHOLE		
DIRT ROAD	TRANSFORMER PAD		
BUILDING	TRANSFORMER PAD (NO TRANSFORMER)		
BLOCK WALL			
CHAIN LINK FENCE			

PENTACORE



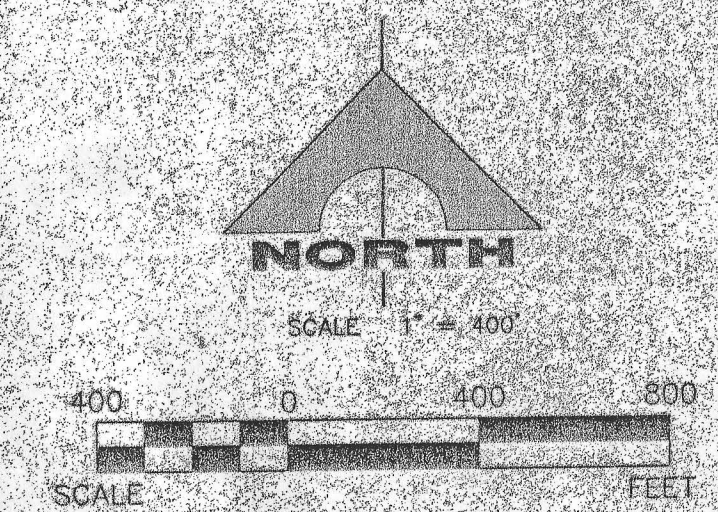
GREEN PARK HOLDINGS, INC  
ALTA/ACSM LAND TITLE SURVEY

SHEET  
10  
OF 10

MAR 26 2002



SEE SHEET DC-5



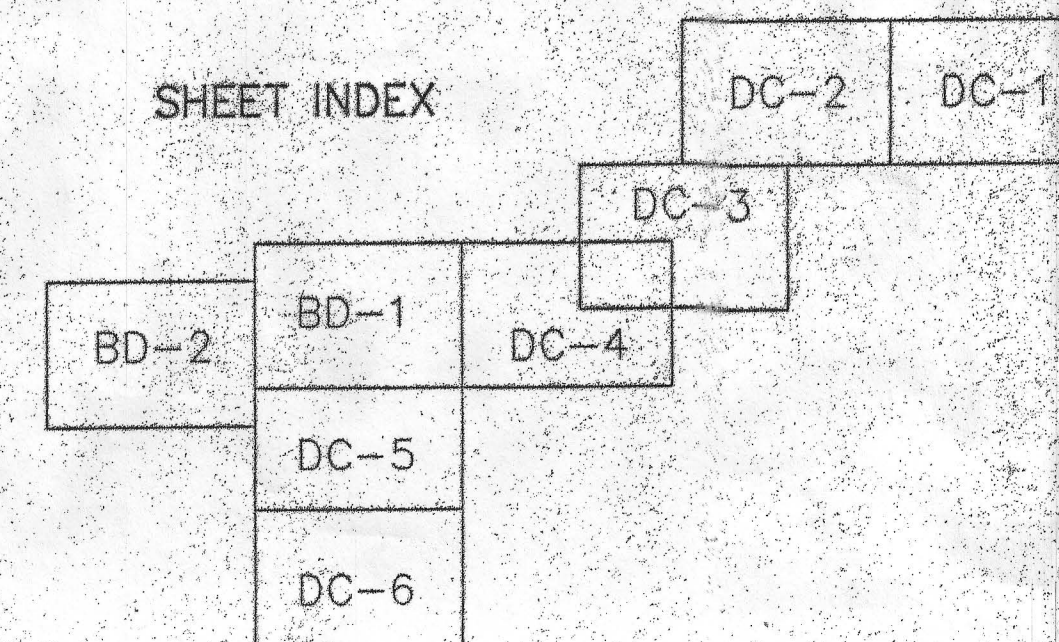
**NOTES**

FLOODING SOURCE: DUCK CREEK  
CLARK COUNTY, NEVADA

MAP BASED ON 1-INCH = 400- FEET TOPOGRAPHIC MAPPING  
CONTOUR INTERVAL = 4 FEET  
FLIGHT DATE: JULY 18, 2000 AND APRIL 18, 2001  
BY KENNY AERIAL MAPPING, INC.

FIRM PANEL(S) AFFECTED: 32003C2569 D, DATED AUGUST 16, 1995  
32003C2568 D, DATED AUGUST 16, 1995  
32003C2910 D, DATED AUGUST 16, 1995

**SHEET INDEX**



**LEGEND**

□ ELEVATION REFERENCE MARKER

PROPOSED CONDITION  
FIRM

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FEB 01 2002  
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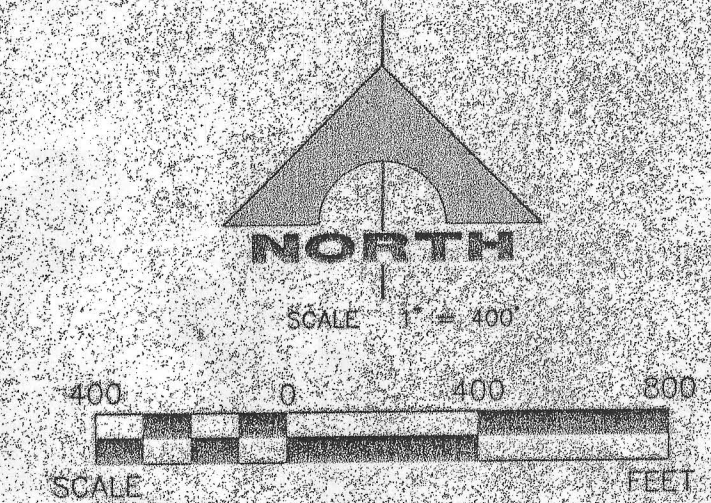
G. C. WALLACE, INC.  
Engineers/Planners/Surveyors

DUCK CREEK/ BLUE DIAMOND WASHES FIS RESTUDY  
DUCK CREEK FROM I-5 TO FICHMAR AVENUE

SHEET  
DC-4  
6 OF 8



SEE SHEET DC-5



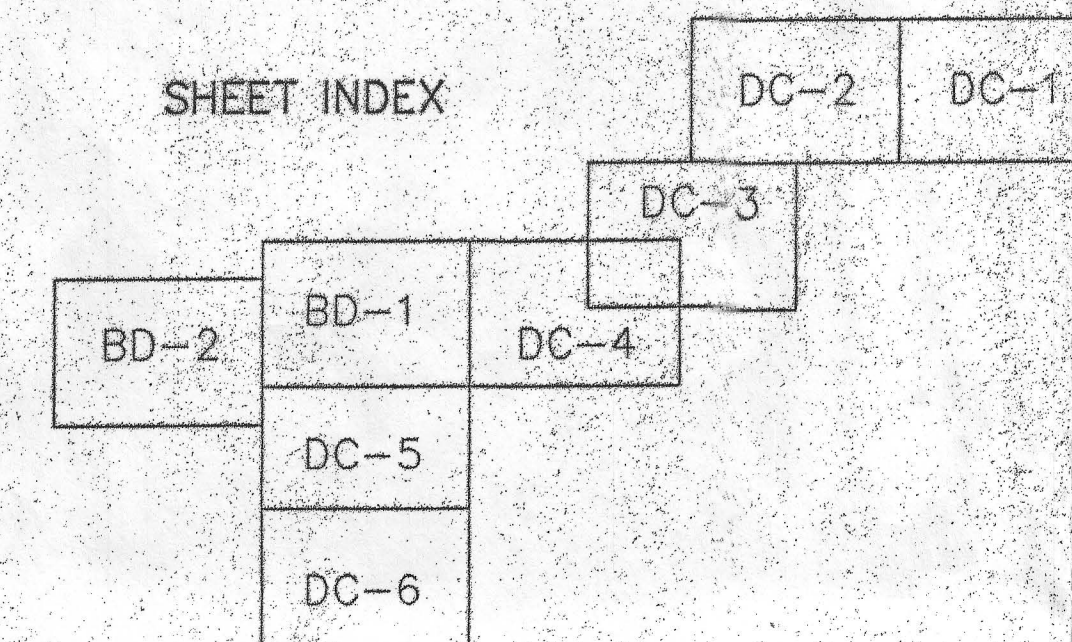
**NOTES**

FLOODING SOURCE: DUCK CREEK  
CLARK COUNTY, NEVADA

MAP BASED ON 1-INCH = 400-FOOT TOPOGRAPHIC MAPPING  
CONTOUR INTERVAL = 4 FEET  
FLIGHT DATE: JULY 18, 2000 AND APRIL 18, 2001  
BY KENNY AERIAL MAPPING, INC.

FIRM PANEL(S) AFFECTED: 32003C2569 D, DATED AUGUST 16, 1995  
32003C2568 D, DATED AUGUST 16, 1995  
32003C2910 D, DATED AUGUST 16, 1995

**SHEET INDEX**



**LEGEND**

□ ELEVATION REFERENCE MARKER

EXISTING CONDITION  
FIRM

RECEIVED  
FEB 01 2002  
STANTEC

G. C. WALLACE, INC.  
Engineers/Planners/Surveyors

DUCK CREEK/ BLUE DIAMOND WASHES FIS RESTUDY  
DUCK CREEK FROM F-15 TO RICHMAR AVENUE

SHEET  
DC-6  
6 OF 8