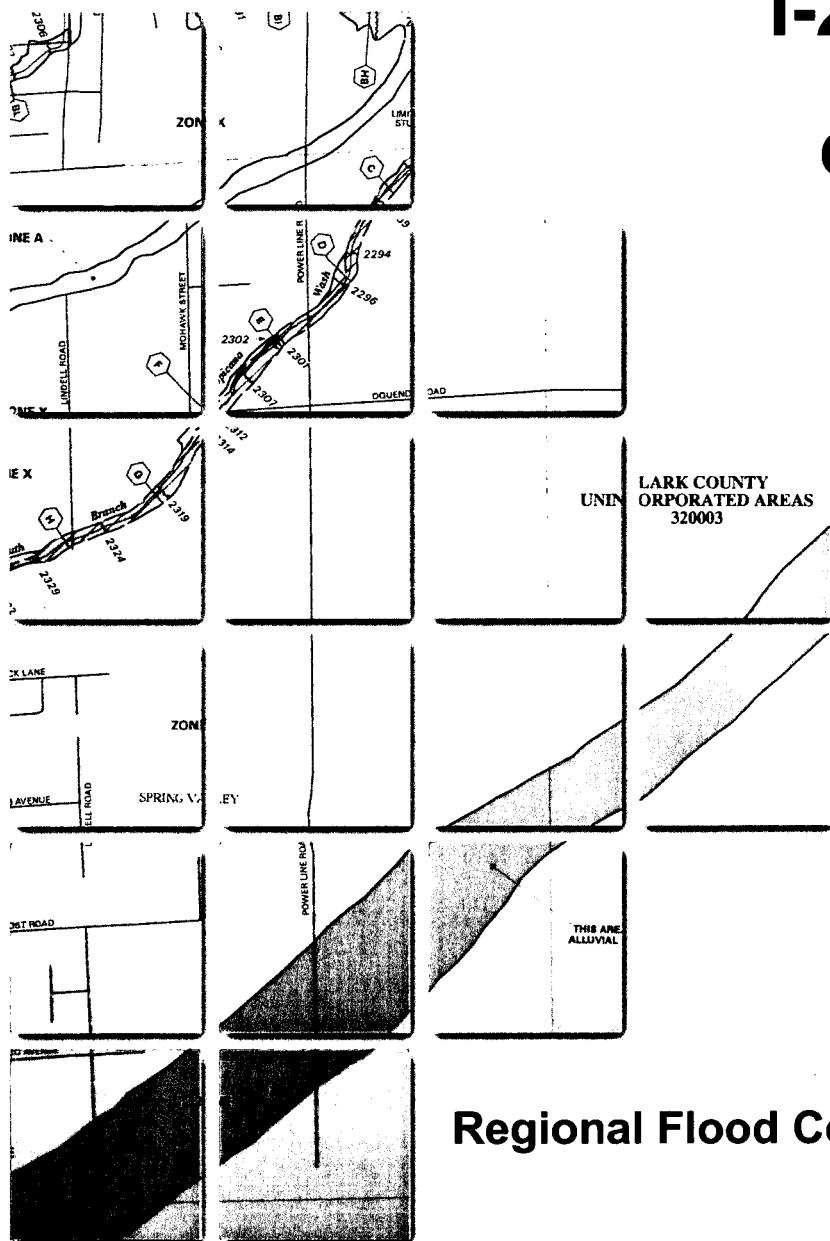


# Request for Letter of Map Revision Blue Diamond Wash I-215 to Tropicana Detention Basin Clark County, Nevada



Submitted to:

**Clark County** REGIONAL FLOOD CONTROL DISTRICT  
**Regional Flood Control District** 

Submitted by:

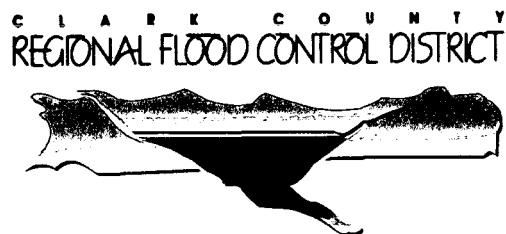
**The Louis Berger Group, Inc.**  
Las Vegas, Nevada



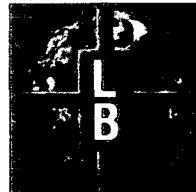
04-09-0021P

**Request for Letter of Map Revision  
Blue Diamond Wash  
I-215 to Tropicana Detention Basin  
Clark County, Nevada**

Prepared for:  
Clark County  
Regional Flood Control District  
500 S. Grand Central Parkway  
Las Vegas, NV 89155



Prepared by:  
**THE Louis Berger Group, INC.**  
500 Amigo Court, Suite 100  
Las Vegas, Nevada 89119  
(702) 736-6632



October 1, 2003



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## I. INTRODUCTION

This submittal package provides hydrologic and hydraulic modeling in support of the request for a Letter of Map Revision for the Blue Diamond Wash between I-215 and the Tropicana Detention Basin in Clark County Nevada.

The Blue Diamond Wash and alluvial fan historically conveyed storm water emanating from the Spring Mountains in the southwestern part of the Las Vegas Valley. The Blue Diamond Detention Basin captures flow at the apex of the fan. Runoff downstream of the detention basin is collected and conveyed by the Blue Diamond Channel to the Tropicana Detention Basin (see Figure 1 in Appendix B). These facilities were constructed as part of the Tropicana and Flamingo Washes Project constructed under the jurisdiction of the US Army Corps of Engineers and Clark County Regional Flood Control District.

## II. PREVIOUS STUDIES

The Blue Diamond Wash was recently studied in the "CLOMR for Area Downstream of Upper Blue Diamond Detention Basin" prepared by PBS&J in February 2000, FEMA Case # 01-09-581P (herein after referred to as Blue Diamond CLOMR). The results of this study were used as the basis of the effective Flood Insurance Rate Map (FIRM) for Clark County, dated September 27, 2002. Relevant excerpts from the Blue Diamond CLOMR are contained in Appendix D including a copy of the HEC-1 hydrologic analysis, drainage basin map, and flow depth calculation.

A copy of the FIRM for the study area is provided as Figure 1 in Appendix B. On the FIRM, the alluvial fan area is designated as flood Zone X (shaded) and the Blue Diamond Wash is designated as Zone A.

Flow conveyed in the Blue Diamond Wash is intercepted by the Blue Diamond Channel at I-215 and conveyed within a concrete-lined channel to the Tropicana Detention Basin as shown on Figure 1 in Appendix B. The effects of the Blue Diamond Channel were recognized in the "Flood Insurance Restudy for the Tropicana Wash and Tributaries" prepared by HDR Engineering, Inc. dated November 2001, FEMA Case # 02-09-718P.

## III. HYDROLOGIC/HYDRAULIC ANALYSIS

Figure 4 in Appendix D is the drainage basin and cross-section map copied from the Blue Diamond CLOMR. Basin 13 on Figure 4 lies between I-215 and the Tropicana Detention Basin and represents the section of Blue Diamond Wash of

interest to this study. Cross-section 34 on Figure 4 was taken at the downstream end of Basin 13.

Since the construction of the Blue Diamond Channel, all flow conveyed in the Blue Diamond Wash south of I-215 is intercepted at freeway. Therefore, the only area contributing flow to the study area is generated from rainfall directly on Basin 13. For the purposes of this study, Basin 13 was divided into subbasins 13A and 13B to evaluate the revised 100-year flow depth and velocity in the wash (see Figure C-1, Appendix C). Subbasin 13A drains to the Lower Blue Diamond Channel and generates a 100-year flow of 12 cfs. Subbasin 13B drains to the Tropicana Detention Basin and generates a 100-year flow of 66 cfs. Backup material for the revised hydrologic analysis and the HEC-1 model output for basins 13A and 13B are provided in Appendix C.

Normal depth calculations were performed to estimate the 100-year flow depth for the Blue Diamond Wash at the downstream ends of Basins 13A and 13B (see Appendix C). Figure 3 is a work map showing the locations of cross-sections, 13A and 34, used for calculating the flow depth. The flow depth at cross-section 13A is 0.72 ft based on the flow rate of 12 cfs. Hydraulic input for cross-section 34 was adopted from the Blue Diamond CLOMR, although the flow rate was revised to 66 cfs. The revised flow depth is 0.42 ft.

#### IV. CONCLUSION

The calculated flow depths for the Blue Diamond Wash between I-215 and the Tropicana Detention Basin are less than one-foot. This submittal is to request that the Zone A designation for the study area be revised to Zone X (shaded).

---

## **APPENDIX A**

FEMA Forms

FEDERAL EMERGENCY MANAGEMENT AGENCY  
OVERVIEW & CONCURRENCE FORM

O.M.B No. 3067-0148  
Expires September 30, 2005

PAPERWORK BURDEN DISCLOSURE NOTICE

Public reporting burden for this form is estimated to average 1 hour per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless a valid OMB control number appears in the upper right corner of this form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, Federal Emergency Management Agency, 500 C Street, SW, Washington DC 20472, Paperwork Reduction Project (3067-0148). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. Please do not send your completed survey to the above address.

A. REQUESTED RESPONSE FROM FEMA

This request is for a (check one):

- CLOMR: A letter from FEMA commenting on whether a proposed project, if built as proposed, would justify a map revision, or proposed hydrology changes (See 44 CFR Ch. 1, Parts 60, 65 & 72).
- LOMR: A letter from FEMA officially revising the current NFIP map to show the changes to floodplains, regulatory floodway or flood elevations. (See Parts 60 & 65 of the NFIP Regulations.)

B. OVERVIEW

1. The NFIP map panel(s) affected for all impacted communities is (are):

Community No.	Community Name	State	Map No.	Panel No.	Effective Date
Ex: 480301 480287	City of Katy Harris County	TX TX	480301 48201C	0005D 0220G	02/08/83 09/28/90
320003	Clark County Incorporated Areas	NV	32003C	2553E	9/27/02
320003	Clark County Incorporated Areas	NV	32003C	2554E	9/27/02

2. Flooding Source: Blue Diamond Watershed

3. Project Name/Identifier: Blue Diamond Channel

4. FEMA zone designations affected: A (choices: A, AH, AO, A1-A30, A99, AE, AR, V, V1-V30, VE, B, C, D, X)

5. Basis for Request and Type of Revision:

- a. The basis for this revision request is (check all that apply)

- Physical Change       Improved Methodology/Data  
 Regulatory Floodway Revision       Other (Attach Description)

Note: A photograph and narrative description of the area of concern is not required, but is very helpful during review.

- b. The area of revision encompasses the following types of flooding and structures (check all that apply)

- |                    |  |  |   |
|--------------------|--|--|---|
| Types of Flooding: | <input checked="" type="checkbox"/> Riverine | <input type="checkbox"/> Coastal         | <input type="checkbox"/> Shallow Flooding (e.g., Zones AO and AH) |
|                    | <input type="checkbox"/> Alluvial fan        | <input type="checkbox"/> Lakes           | <input type="checkbox"/> Other (Attach Description)               |
| Structures:        | <input type="checkbox"/> Channelization      | <input type="checkbox"/> Levee/Floodwall | <input type="checkbox"/> Bridge/Culvert                           |
|                    | <input type="checkbox"/> Dam                 | <input type="checkbox"/> Fill            | <input type="checkbox"/> Other, Attach Description                |

### C. REVIEW FEE

Has the review fee for the appropriate request category been included?  Yes Fee amount: \$EXEMPT  
 No, Attach Explanation

Please see the FEMA Web site at [http://www.fema.gov/fhm/frm\\_fees.shtm](http://www.fema.gov/fhm/frm_fees.shtm) for Fee Amounts and Exemptions.

### D. SIGNATURE

All documents submitted in support of this request are correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 1001.

Name: Syndi J. Flippin-Dudley, Ph.D, P.E.	Company: The Louis Berger Group, Inc.	
Mailing Address: 500 East Amigo Court Suite 100 Las Vegas, NV 89119	Daytime Telephone No.: (702)-736-6632	Fax No.: (702)-736-0704
	E-Mail Address: <a href="mailto:sdudley@louisberger.com">sdudley@louisberger.com</a>	

Signature of Requester (required):

Date:

10/1/03

As the community official responsible for floodplain management, I hereby acknowledge that we have received and reviewed this Letter of Map Revision (LOMR) or conditional LOMR request. Based upon the community's review, we find the completed or proposed project meets or is designed to meet all of the community floodplain management requirements, including the requirement that no fill be placed in the regulatory floodway, and that all necessary Federal, State, and local permits have been, or in the case of a conditional LOMR, will be obtained. In addition, we have determined that the land and any existing or proposed structures to be removed from the SFHA are or will be reasonably safe from flooding as defined in 44CFR 65.2(c), and that we have available upon request by FEMA, all analyses and documentation used to make this determination.

Community Official's Name and Title: Dave Betley, P.E., Principal Engineer, Civil Engineering Division	Telephone No.: (702)-455-4808
Community Name: Clark County Unincorporated Areas	Community Official's Signature (required): 
	Date: 10/1/03

### CERTIFICATION BY REGISTERED PROFESSIONAL ENGINEER AND/OR LAND SURVEYOR

This certification is to be signed and sealed by a licensed land surveyor, registered professional engineer, or architect authorized by law to certify elevation information. All documents submitted in support of this request are correct to the best of my knowledge. I understand that any false statement may be punishable by fine or imprisonment under Title 18 of the United States Code, Section 1001.

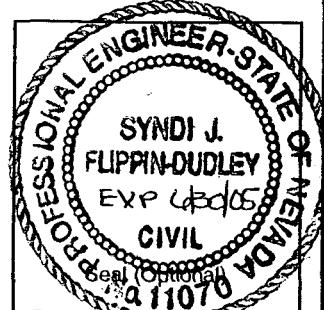
Certifier's Name: Syndi J. Flippin-Dudley	License No.: 11070	Expiration Date: 6/30/05
Company Name: The Louis Berger Group, Inc.	Telephone No.: (702)-733-6632	Fax No.: (702)-736-0704
Signature: 	Date: 10/1/03	

Ensure the forms that are appropriate to your revision request are included in your submittal.

#### Form Name and (Number)

#### Required if...

- Riverine Hydrology and Hydraulics Form (Form 2) New or revised discharges or water-surface elevations
- Riverine Structures Form (Form 3) Channel is modified, addition/revision of bridge/culverts, addition/revision of levee/floodwall, addition/revision of dam
- Coastal Analysis Form (Form 4) New or revised coastal elevations
- Coastal Structures Form (Form 5) Addition/revision of coastal structure
- Alluvial Fan Flooding Form (Form 6) Flood control measures on alluvial fans



FEDERAL EMERGENCY MANAGEMENT AGENCY  
**RIVERINE HYDROLOGY & HYDRAULICS FORM**

O.M.B No. 3067-0148  
Expires September 30, 2005

**PAPERWORK REDUCTION ACT**

Public reporting burden for this form is estimated to average 3 hours per response. The burden estimate includes the time for reviewing instructions, searching existing data sources, gathering and maintaining the needed data, and completing, reviewing, and submitting the form. You are not required to respond to this collection of information unless a valid OMB control number appears in the upper right corner of this form. Send comments regarding the accuracy of the burden estimate and any suggestions for reducing this burden to: Information Collections Management, Federal Emergency Management Agency, 500 C Street, SW, Washington DC 20472, Paperwork Reduction Project (3067-0148). Submission of the form is required to obtain or retain benefits under the National Flood Insurance Program. Please do not send your completed survey to the above address.

Flooding Source: Blue Diamond Watershed

Note: Fill out one form for each flooding source studied

**A. HYDROLOGY**

1. Reason for New Hydrologic Analysis (check all that apply)

- |  |  |   |
|--|--|---|
| <input type="checkbox"/> Not revised (skip to section 2) | <input type="checkbox"/> No existing analysis        | <input type="checkbox"/> Improved data                                      |
| <input type="checkbox"/> Alternative methodology         | <input type="checkbox"/> Proposed Conditions (CLOMR) | <input checked="" type="checkbox"/> Changed physical condition of watershed |

2. Comparison of Representative 1%-Annual-Chance Discharges

Location	Drainage Area (Sq. Mi.)	FIS (cfs)	Revised (cfs)
Basin 13A	0.099	75(Combined)	12
Basin 13B	0.105		66

3. Methodology for New Hydrologic Analysis (check all that apply)

- |   |   |
|---|---|
| <input type="checkbox"/> Statistical Analysis of Gage Records | <input checked="" type="checkbox"/> Precipitation/Runoff Model [TR-20, HEC-1, HEC-HMS etc.] |
| <input type="checkbox"/> Regional Regression Equations        | <input type="checkbox"/> Other (please attach description)                                  |

Please enclose all relevant models in digital format, maps, computations (including computation of parameters) and documentation to support the new analysis. The document, "Numerical Models Accepted by FEMA for NFIP Usage" lists the models accepted by FEMA. This document can be found at: [http://www.fema.gov/fhm/en\\_modl.shtm](http://www.fema.gov/fhm/en_modl.shtm).

4. Review/Approval of Analysis

If your community requires a regional, state, or federal agency to review the hydrologic analysis, please attach evidence of approval/review.

5. Impacts of Sediment Transport on Hydrology

Was sediment transport considered?  Yes  No If yes, then fill out Section F (Sediment Transport) of Form 3. If No, then attach your explanation for why sediment transport was not considered.

**B. HYDRAULICS**

1. Reach to be Revised

	Description	Cross Section	Water-Surface Elevations (ft.)	
			Effective	Proposed/Revised
Downstream Limit	Tropicana Detention Basin	34	N/A	2305.42
Upstream Limit	I-215 Beltway and Jones	13A	N/A	2392.85

2. Hydraulic Method Used

Hydraulic Analysis Flowmaster [HEC-2 , HEC-RAS, Other (Attach description)]

## B. HYDRAULICS (CONTINUED)

### 3. Pre-Submittal Review of Hydraulic Models

FEMA has developed two review programs, CHECK-2 and CHECK-RAS, to aid in the review of HEC-2 and HEC-RAS hydraulic models, respectively. These review programs verify that the hydraulic estimates and assumptions in the model data are in accordance with NFIP requirements, and that the data are comparable with the assumptions and limitations of HEC-2/HEC-RAS. CHECK-2 and CHECK-RAS identify areas of potential error or concern. These tools do not replace engineering judgment. CHECK-2 and CHECK-RAS can be downloaded from [http://www.fema.gov/fhm/frm\\_soft.shtm](http://www.fema.gov/fhm/frm_soft.shtm). We recommend that you review your HEC-2 and HEC-RAS models with CHECK-2 and CHECK-RAS. If you disagree with a message, please attach an explanation of why the message is not valid in this case. Review of your submittal and resolution of valid modeling discrepancies will result in reduced review time.

HEC-2/HEC-RAS models reviewed with CHECK-2/CHECK-RAS?  Yes  No

### 4. Models Submitted

Duplicate Effective Model*	Natural File Name:	Floodway File Name:
Corrected Effective Model*	Natural File Name:	Floodway File Name:
Existing or Pre-Project Conditions Model	Natural File Name: N/A	Floodway File Name:
Revised or Post-Project Conditions Model	Natural File Name: N/A	Floodway File Name:
Other - (attach description)	Natural File Name:	Floodway File Name:

\*Not required for revisions to approximate 1%-annual-chance floodplains (Zone A) – for details, refer to the corresponding section of the instructions.

The document "Numerical Models Accepted by FEMA for NFIP Usage" lists the models accepted by FEMA. This document can be found at: [http://www.fema.gov/fhm/en\\_modl.shtm](http://www.fema.gov/fhm/en_modl.shtm).

## C. MAPPING REQUIREMENTS

A certified topographic map must be submitted showing the following information (where applicable): the boundaries of the effective, existing, and proposed conditions 1%-annual-chance floodplain (for approximate Zone A revisions) or the boundaries of the 1%- and 0.2%-annual-chance floodplains and regulatory floodway (for detailed Zone AE, AO, and AH revisions); location and alignment of all cross sections with stationing control indicated; stream, road, and other alignments (e.g., dams, levees, etc.); current community easements and boundaries; boundaries of the requester's property; certification of a registered professional engineer registered in the subject State; location and description of reference marks; and the referenced vertical datum (NGVD, NAVD, etc.).

Note that the boundaries of the existing or proposed conditions floodplains and regulatory floodway to be shown on the revised FIRM and/or FBFM must tie-in with the effective floodplain and regulatory floodway boundaries. Please attach a copy of the effective FIRM and/or FBFM, annotated to show the boundaries of the revised 1%- and 0.2%-annual-chance floodplains and regulatory floodway that tie-in with the boundaries of the effective 1%- and 0.2%-annual-chance floodplain and regulatory floodway at the upstream and downstream limits of the area of revision.

## D. COMMON REGULATORY REQUIREMENTS

1. For CLOMR requests, do Base Flood Elevations (BFEs) increase?  Yes  No

For CLOMR requests, if either of the following is true, please submit evidence of compliance with Section 65.12 of the NFIP regulations:

- The proposed project encroaches upon a regulatory floodway and would result in increases above 0.00 foot.
- The proposed project encroaches upon a SFHA with BFEs established and would result in increases above 1.00 foot.

2. Does the request involve the placement or proposed placement of fill?  Yes  No

If Yes, the community must be able to certify that the area to be removed from the special flood hazard area, to include any structures or proposed structures, meets all of the standards of the local floodplain ordinances, and is reasonably safe from flooding in accordance with the NFIP regulations set forth at 44 CFR 60.3(a)(3), 65.5(a)(4), and 65.6(a)(14). Please see the MT-2 instructions for more information.

3. For LOMR requests, is the regulatory floodway being revised?  Yes  No

If Yes, attach evidence of regulatory floodway revision notification. As per Paragraph 65.7(b)(1) of the NFIP Regulations, notification is required for requests involving revisions to the regulatory floodway. (Not required for revisions to approximate 1%-annual-chance floodplains [studied Zone A designation] unless a regulatory floodway is being added. Elements and examples of regulatory floodway revision notification can be found in the MT-2 Form 2 Instructions.)

4. For LOMR requests, does this request require property owner notification and acceptance of BFE increases?  Yes  No

If Yes, please attach proof of property owner notification and acceptance (if available). Elements of and examples of property owner notification can be found in the MT-2 Form 2 Instructions.

## **EXPLANATIONS**

### **Review Fee (Form 1, Section C)**

This submittal is exempt from the fee due to being based on the federally funded, US Army Corps of Engineers' Blue Diamond Channel.

### **Effective Hydrology (Form 2, Section A-1)**

The effective hydrology was taken from the CLOMR for the Upper Blue Diamond Detention Basin, FEMA Case #00-09-170R.

### **Sediment Transport (Form 2, Section A-5)**

Sediment transport was not considered because it was an approximate study.

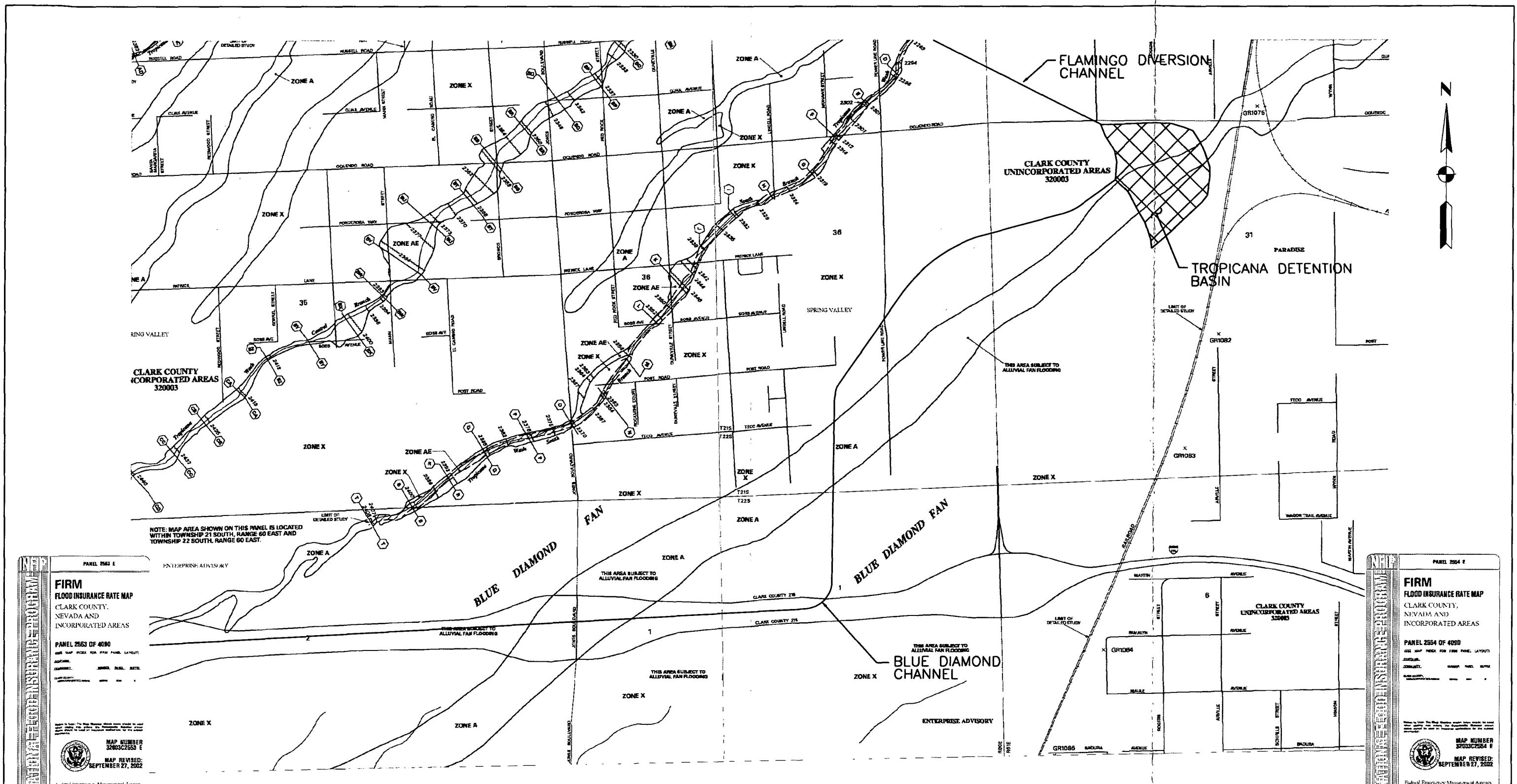
### **Hydraulic Calculation Method (Form 2, Section B-2)**

FlowMaster was used for the hydraulic calculations to determine the 100-year water surface elevations. Manning's Equation was used for the hydraulic calculations. It was used for hydraulic calculations in the CLOMR for the Upper Blue Diamond Detention Basin, FEMA Case #00-09-170R.

FlowMaster performs hydraulics calculations and analysis of pipes, ditches and open channels. To do this, FlowMaster computes flows and pressures based on user selected formulas such as Darcy-Weisbach (Colebrook-White), Hazen-Williams, Kutter's, and Manning's. The program calculates a user selected unknown variable after parameters have been entered. FlowMaster also calculates rating tables, and plots curves and cross sections.

## **APPENDIX B**

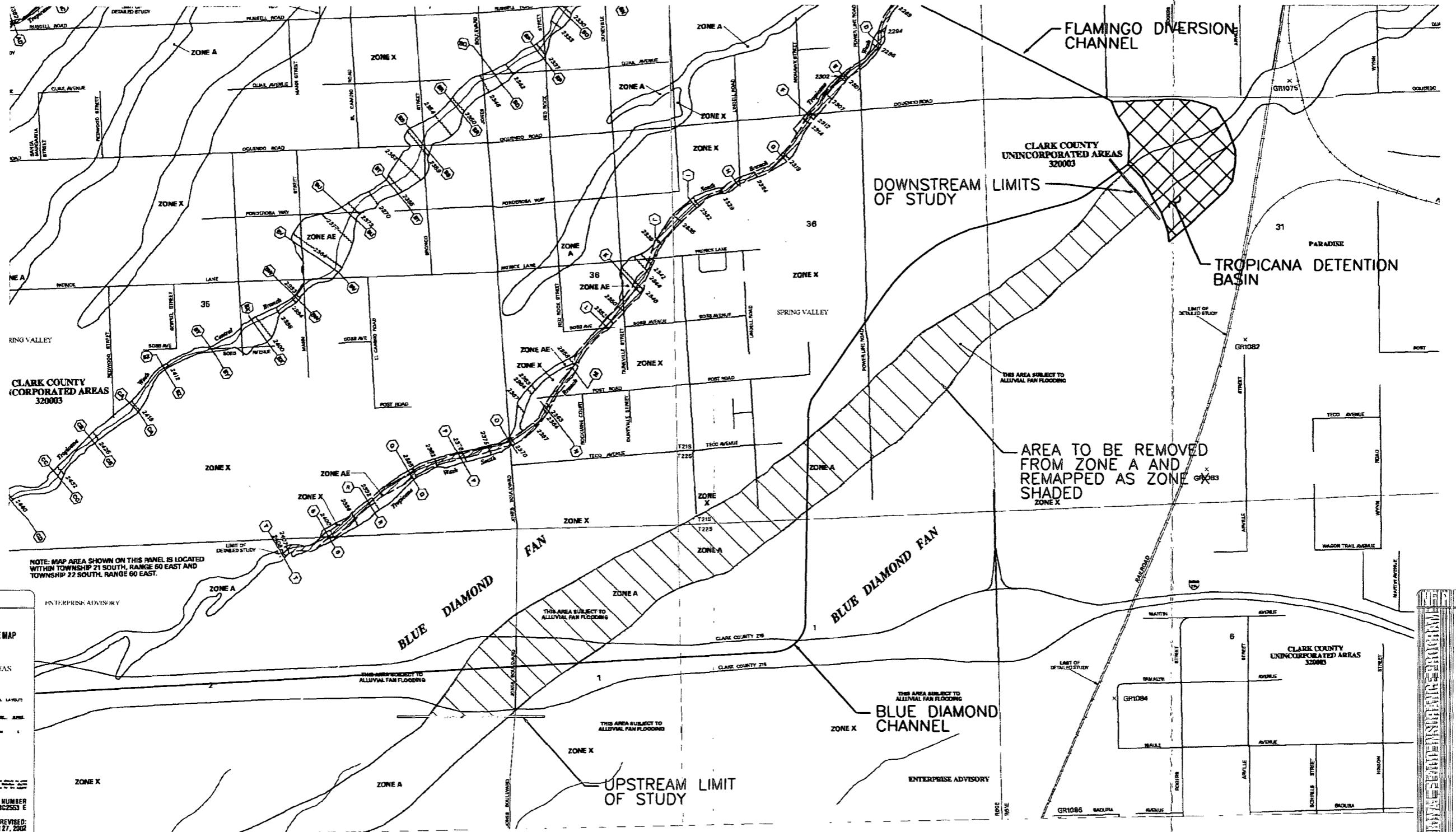
Figures



**FEMA FLOOD ZONE MAP  
FROM FIRM MAP  
NUMBER 32003C-2553E AND  
NUMBER 32003C-2554E  
FIGURE 1**



**THE LOUIS BERGER GROUP, INC.**  
LAS VEGAS, NEVADA



## ANNOTATED FLOOD ZONE MAP

FIGURE 2

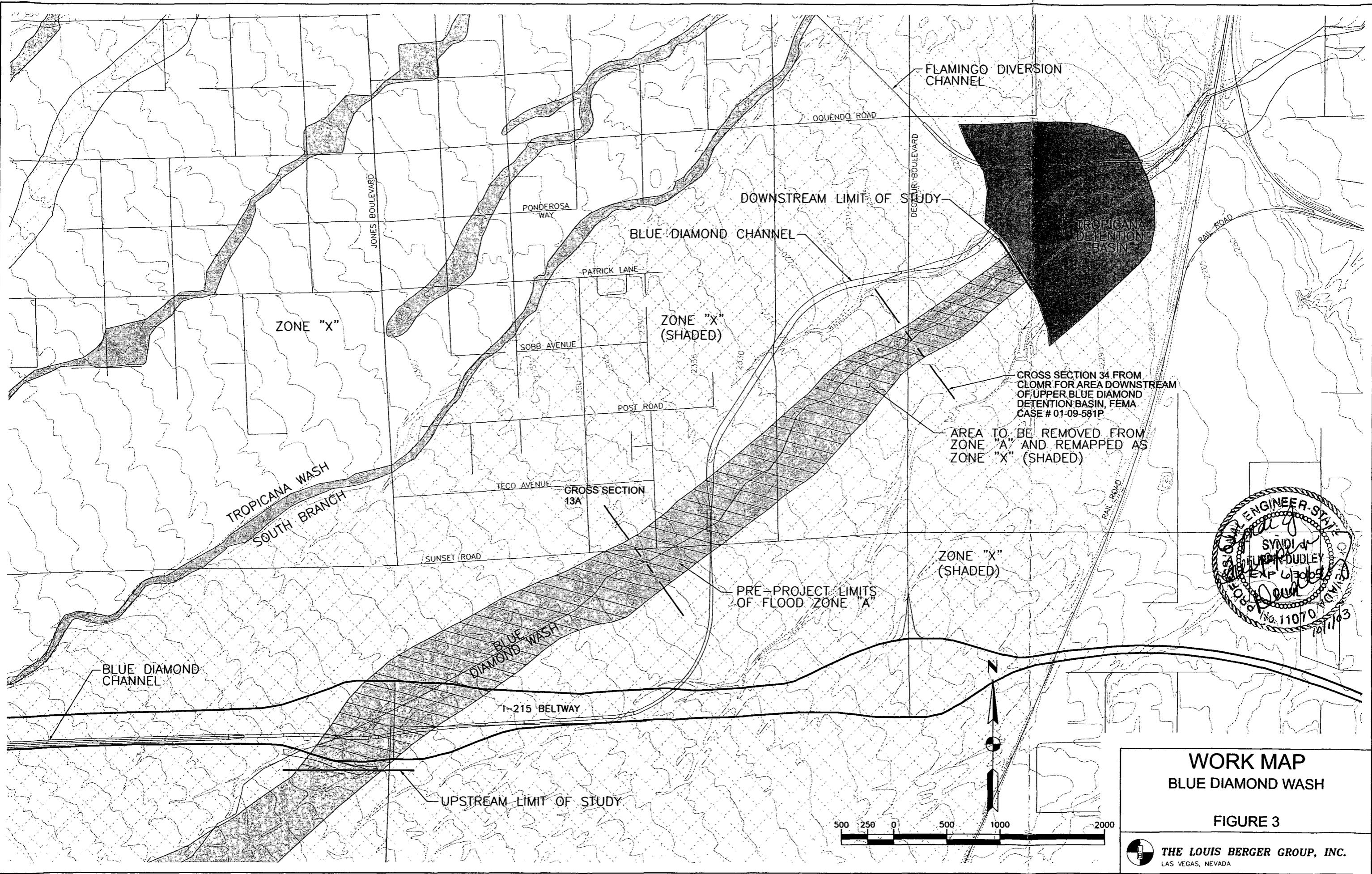




Photo 1: Blue Diamond Wash near Cross Section 13A

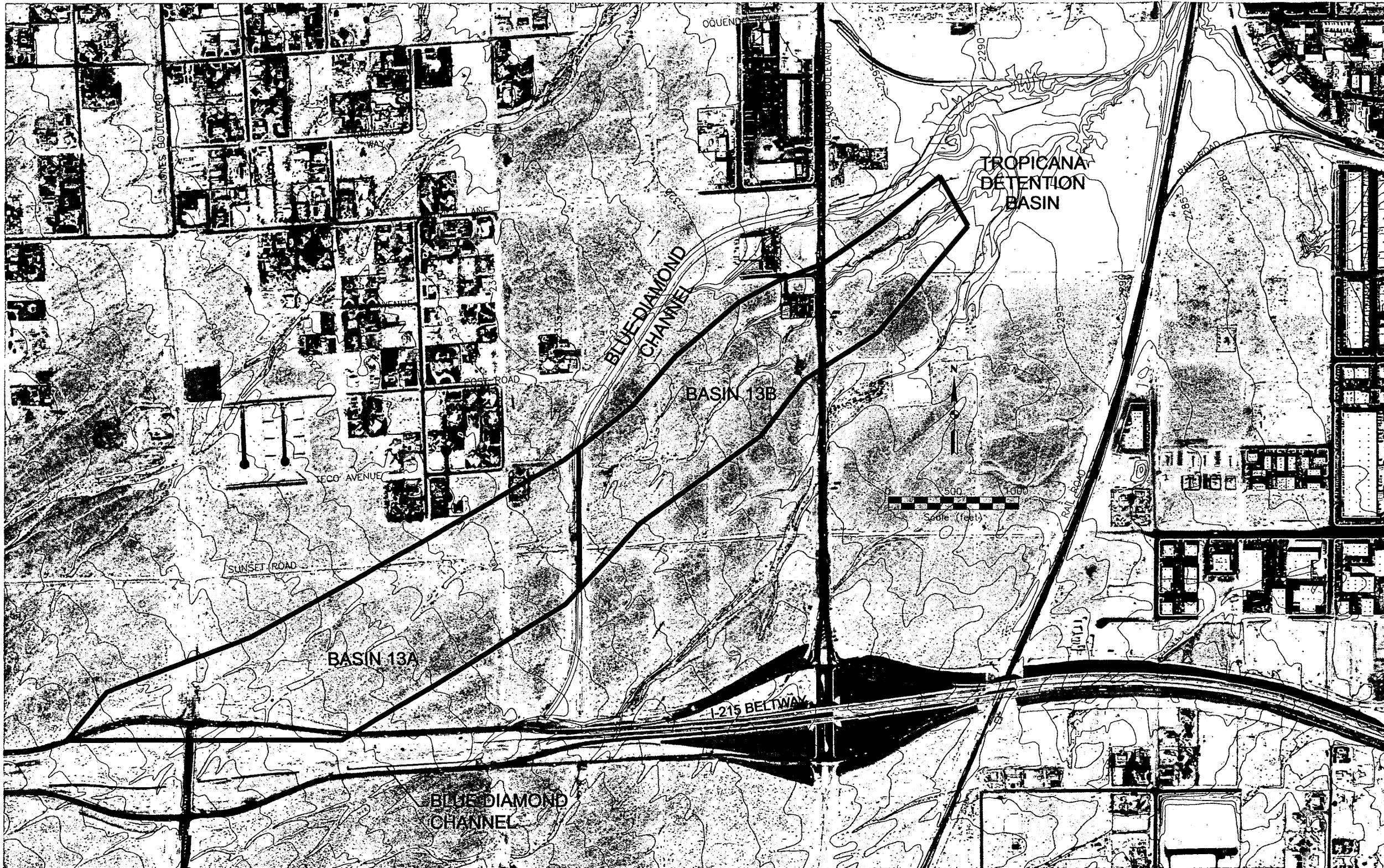


Photo 2: Blue Diamond Wash near Cross Section 34

---

## **APPENDIX C**

### Hydrologic Model and Flow Depth Calculation



DRAINAGE BASIN MAP

BLUE DIAMOND WASH

FIGURE C-1



THE LOUIS BERGER GROUP, INC.  
LAS VEGAS, NEVADA

**STANDARD FORM 4**  
**HYDROLOGIC CRITERIA AND DRAINAGE DESIGN MANUAL**  
**TIME OF CONCENTRATION**

Blue Diamond Wash  
File: 879 time of concentration.xls  
Project No.: CB-879

BY: VS  
DATE: 9/15/2003

SUB-BASIN DATA						INITIAL / OVERLAND TIME (T <sub>i</sub> )			TRAVEL TIME (T <sub>t</sub> )						T <sub>c</sub>	T <sub>c</sub> Check	T <sub>lag</sub>
BASIN (1)	DEV./UN. (D or U)	CN	K	AREA Ac (2)	AREA mi <sup>2</sup> (3)	L <sub>o</sub> INITIAL LENGTH (ft) (4)	S SLOPE % (5)	T <sub>i</sub> (min) (6)	TRAVEL LENGTH (ft) (7)	SLOPE % (8)	V <sub>1</sub> VELOCITY FPS (9a)	V <sub>2</sub> VELOCITY FPS (9b)	T <sub>t</sub> (min) (10)	(min) (13)	(min) (14)	T <sub>lag</sub> = 0.6T <sub>c</sub> /60 (hours)	
Basin 13A	U	63.0	0.4416	63.4	0.0990	500	0.5	33.4	3757	1.5	2.0	2.0	31.3	64.7	N/A	0.647	
Basin 13B	U	83.0	0.7056	67.1	0.1049	500	0.5	20.0	3596	1.5	2.0	2.0	30.0	50.0	N/A	0.500	

For Developed basins  $V_1 = 20.2 (\text{Slope}/100)^{0.5}$  and  $V_2 = 30.6 (\text{Slope}/100)^{0.5}$

For Existing basins  $V_1 = 14.8 (\text{Slope}/100)^{0.5}$  and  $V_2 = 29.4 (\text{Slope}/100)^{0.5}$

If the Travel Length is greater than 500 ft then  $T_t = 500/(60 V_1) + (\text{Travel Length} - 500)/(60 V_2)$

If the Travel Length is less than 500 ft then  $T_t = \text{Travel Length}/(60 V_2)$

$$T_c = T_i + T_t$$

Table: Curve number calculations for Basins

Basin ID	Basin Area		Soil Class	Soil Type	Percentage	Existing CN
	(ft <sup>2</sup> )	(mi <sup>2</sup> )				
Basin 13A	2,748,406	0.099	263 - Jean	A	100	63
Basin 13B	2,922,253	0.105	260 - Jean	A	20	63
			240 - Goodsprings	D	80	88
					Weighted =	83



United States  
Department of  
Agriculture

Soil  
Conservation  
Service

In cooperation with  
United States Department  
of the Interior, Bureau  
of Land Management, and  
University of Nevada  
Agricultural Experiment  
Station

All programs of the U.S. Department  
of Agriculture are available to everyone  
without regard to race, color, national  
origin, religion, sex, age, marital  
status, or handicap.

# Soil Survey of Las Vegas Valley Area Nevada

## Part of Clark County

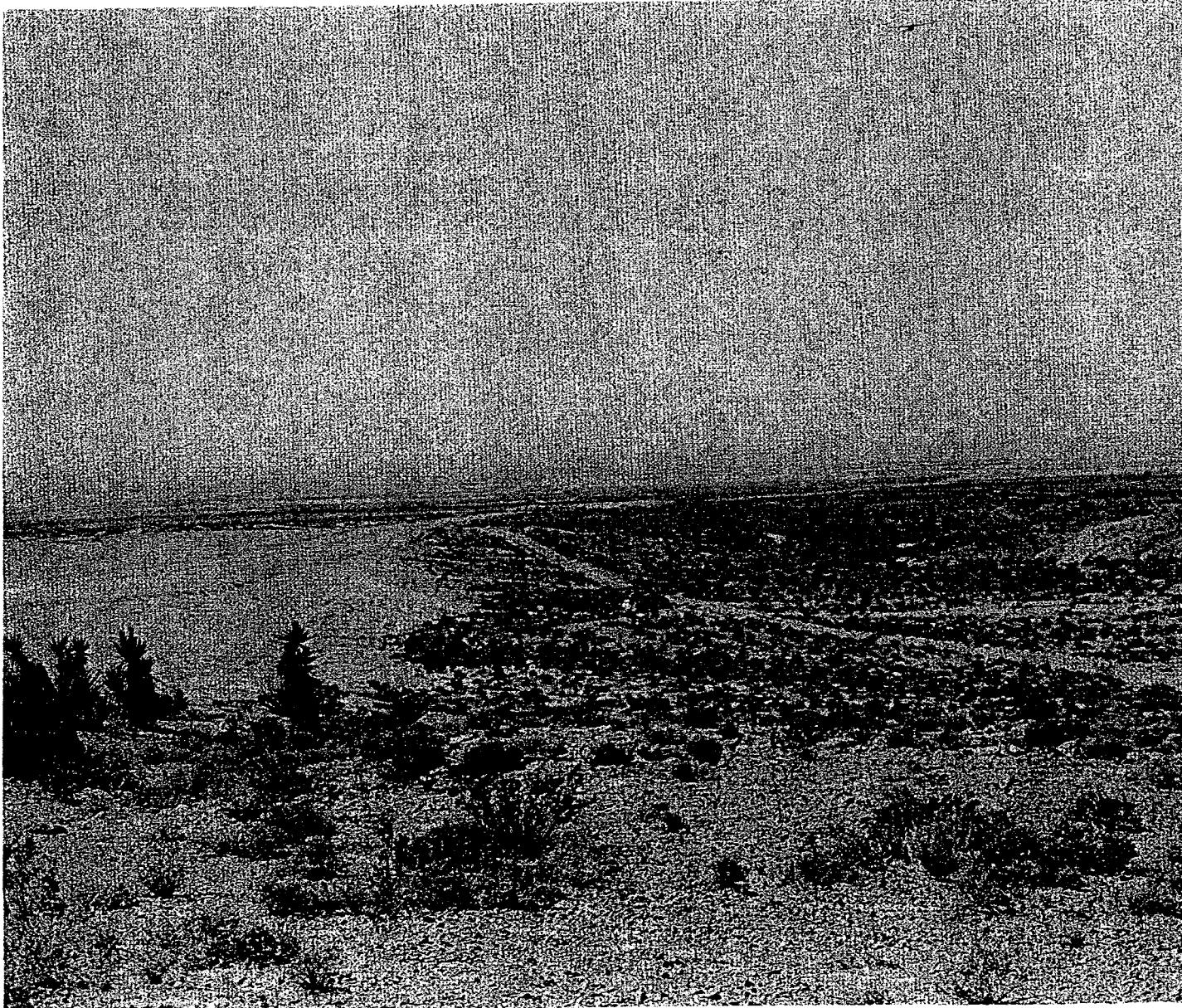


TABLE 15.--SOIL AND WATER FEATURES--Continued

Soil name and map symbol	Hydro-logic group	Flooding		High water table		Bedrock		Cemented pan		Risk of corrosion	
		Frequency	Months	Depth	Months	Depth	Thickness	In	Depth	Hardness	Uncoated steel
182*: Caliza-----	B	None-----	---	>6.0	---	>60	---	---	---	High-----	Low.
Pittman-----	C	None-----	---	>6.0	---	>60	---	20-30	Thick	High-----	Low.
Arizo-----	A	Occasional	Mar-Sep	>6.0	---	>60	---	---	---	High-----	Low.
183 Caliza-----	B	None-----	---	>6.0	---	>60	---	---	---	High-----	Low.
184 Caliza-----	B	Rare-----	---	>6.0	---	>60	---	---	---	High-----	Low.
187 Caliza-----	B	None-----	---	>6.0	---	>60	---	---	---	High-----	Low.
190 Dalian-----	B	None-----	---	>6.0	---	>60	---	---	---	High-----	Low.
191 Dalian-----	B	Rare-----	---	>6.0	---	>60	---	---	---	High-----	Low.
192*: Dalian-----	B	Rare-----	---	>6.0	---	>60	---	---	---	High-----	Low.
McCullough-----	B	Rare-----	---	>6.0	---	>60	---	---	---	High-----	Low.
200 Glencarb-----	B	Rare-----	---	>6.0	---	>60	---	---	---	High-----	Moderate.
206 Glencarb-----	C	Occasional	Jul-Sep	3.0-5.0	Jul-Jun	>60	---	---	---	High-----	High.
222 Glencarb-----	C	Rare-----	---	3.0-5.0	Jul-Jun	>60	---	---	---	High-----	High.
236 Glencarb-----	B	Rare-----	---	>6.0	---	>60	---	---	---	High-----	High.
237 Glencarb-----	B	Rare-----	---	>6.0	---	>60	---	40-60	Thick	High-----	Low.
(240) Goodsprings-----	D	None-----	---	>6.0	---	>60	---	9-20	Thick	High-----	Low.
252, 255 Grapevine-----	B	Rare-----	---	>6.0	---	>60	---	---	---	High-----	High.
(260) Jean-----	A	Rare-----	---	>6.0	---	>60	---	---	---	High-----	Low.
262*: Jean-----	A	Rare-----	---	>6.0	---	>60	---	---	---	High-----	Low.
Jean-----	A	Occasional	Jun-Sep	>6.0	---	>60	---	---	---	High-----	Low.
Goodsprings-----	D	Rare-----	---	>6.0	---	>60	---	9-20	Thick	High-----	Low.
(263*): Jean-----	A	Rare-----	---	>6.0	---	>60	---	---	---	High-----	Low.
Jean-----	A	Occasional	Jun-Sep	>6.0	---	>60	---	---	---	High-----	Low.
264 Jean-----	A	Rare-----	---	>6.0	---	>60	---	---	---	High-----	Low.

See footnote at end of table.

# HYDROLOGIC CRITERIA AND DRAINAGE DESIGN MANUAL

## RUNOFF CURVE NUMBERS (SEMIARID RANGELANDS<sup>1</sup>)

Cover type	Cover description	Hydrologic condition <sup>2</sup>	Curve numbers for hydrologic soil group—			
			A <sup>3</sup>	B	C	D
Herbaceous—mixture of grass, weeds, and low-growing brush, with brush the minor element.		Poor	80	87	93	
		Fair	71	81	89	
		Good	62	74	85	
Oak-aspen—mountain brush mixture of oak brush, aspen, mountain mahogany, bitter brush, maple, and other brush.		Poor	66	74	79	
		Fair	48	57	63	
		Good	30	41	48	
Pinyon-juniper—pinyon, juniper, or both; grass understory.		Poor	75	85	89	
		Fair	58	73	80	
		Good	41	61	71	
Sagebrush with grass understory.		Poor	67	80	85	
		Fair	51	63	70	
		Good	35	47	55	
Desert shrub—major plants include saltbush, greasewood, creosotebush, blackbrush, bursage, palo verde, mesquite, and cactus.		Poor	63	77	85	88
		Fair	55	72	81	86
		Good	49	68	79	84

<sup>1</sup>Average runoff condition, and  $I_a = 0.2S$ .

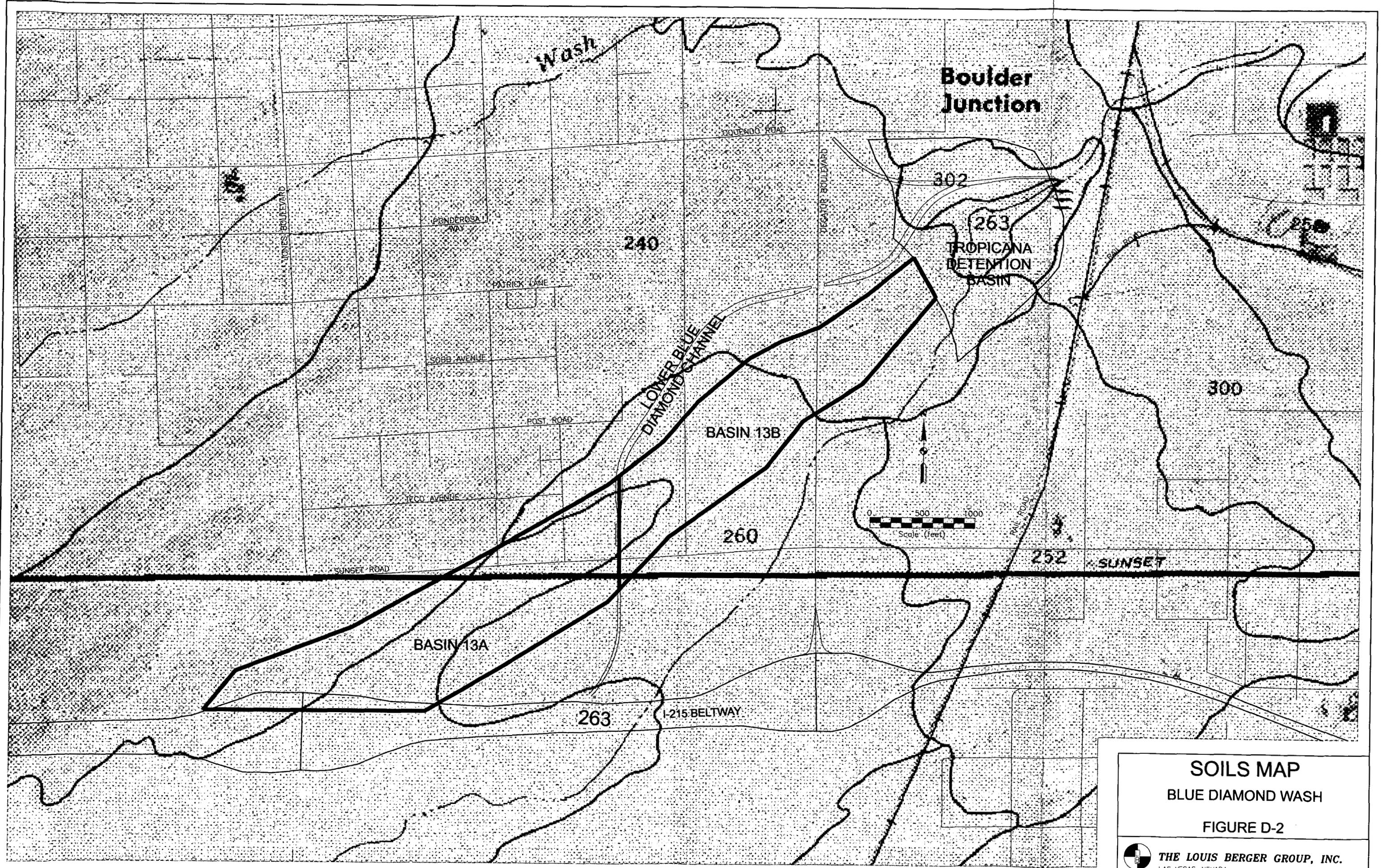
<sup>2</sup>Poor: <30% ground cover (litter, grass, and brush overstory).

Fair: 30 to 70% ground cover.

Good: >70% ground cover.

<sup>3</sup>Curve numbers for group A have been developed only for desert shrub.

Revision	Date



FILE NAME: X:\879-F\amigo\Exhibit\879-FIGURE-11.dgn - 9/17/2003 - 12:24:35 JPS

```
*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 11SEP03 TIME 10:26:06 *
*****
```

```
*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
```

```
      X   X   XXXXXXXX   XXXXX   X
      X   X   X   X   XX
      X   X   X   X
      XXXXXX XXXX   X   XXXXX X
      X   X   X   X
      X   X   X   X   X
      X   X   XXXXXXXX   XXXXX   XXX
```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.  
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE , SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,  
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
------	---

\*DIAGRAM

1 ID	CB 879	DATE: 07/01/03
2 ID	FILE NAME: 879B13	
3 ID	879B13 = PROJECT 879 BASIN 13	
4 ID	PROJECT NAME: BLUE DIAMOND WASH	
5 ID	I-215 TO TROPICANA DETENTION BASIN LOMR	
6 ID	PREPARED BY: THE LOUIS BERGER GROUP	
7 ID	FOR: THE CLARK COUNTY REGIONAL FLOOD CONTROL DISTRICT	
8 ID		
9 IT	5 0 0 300	
10 IO	5	
11 IN	5 0 0	
12 JR	PRBC 1.0	
	*	

13 KK 13A

14 KM	RUNOFF FROM THE SUBBASIN 13A
15 BA	0.099
16 PB	2.77
17 PC	0.000 0.020 0.057 0.070 0.087 0.108 0.124 0.130 0.130 0.130
18 PC	0.130 0.130 0.130 0.133 0.140 0.142 0.148 0.158 0.172 0.181
19 PC	0.190 0.197 0.199 0.200 0.201 0.204 0.214 0.229 0.241 0.249
20 PC	0.251 0.256 0.270 0.278 0.281 0.283 0.295 0.322 0.352 0.409
21 PC	0.499 0.590 0.710 0.744 0.781 0.812 0.819 0.835 0.851 0.856
22 PC	0.860 0.868 0.876 0.888 0.910 0.926 0.937 0.950 0.970 0.976
23 PC	0.982 0.985 0.987 0.989 0.990 0.993 0.993 0.994 0.995 0.998
24 PC	0.998 0.999 1.000
25 LS	0 63
26 UD	0.65
	*

27 KK 13B

28 KM	RUNOFF FROM THE SUBBASIN 13B
29 BA	0.105
30 PB	2.77
31 LS	0 83
32 UD	0.50
	*

33 ZZ

1 SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(-->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW

13 13A

27 . 13B

(\*\*\*) RUNOFF ALSO COMPUTED AT THIS LOCATION

```
*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* JUN 1998 *
* VERSION 4.1 *
* RUN DATE 11SEP03 TIME 10:26:06 *
*****
```

```
*****
* U.S. ARMY CORPS OF ENGINEERS *
* HYDROLOGIC ENGINEERING CENTER *
* 609 SECOND STREET *
* DAVIS, CALIFORNIA 95616 *
* (916) 756-1104 *
*****
```

CB 879  
FILE NAME: 879B13  
879B13 = PROJECT 879 BASIN 13  
PROJECT NAME: BLUE DIAMOND WASH  
I-215 TO TROPICANA DETENTION BASIN LOMR  
PREPARED BY; THE LOUIS BERGER GROUP  
FOR: THE CLARK COUNTY REGIONAL FLOOD CONTROL DISTRICT

10 IO        OUTPUT CONTROL VARIABLES  
IPRNT            5 PRINT CONTROL  
IPLOT            0 PLOT CONTROL  
QSCAL           0. HYDROGRAPH PLOT SCALE

IT        HYDROGRAPH TIME DATA  
NMIN            5 MINUTES IN COMPUTATION INTERVAL  
IDATE          1 0 STARTING DATE  
ITIME          0000 STARTING TIME  
NQ              300 NUMBER OF HYDROGRAPH ORDINATES  
NDDATE        2 0 ENDING DATE  
NDTIME        0055 ENDING TIME  
ICENT          19 CENTURY MARK

COMPUTATION INTERVAL     .08 HOURS  
TOTAL TIME BASE    24.92 HOURS

ENGLISH UNITS  
DRAINAGE AREA            SQUARE MILES  
PRECIPITATION DEPTH    INCHES  
LENGTH, ELEVATION       FEET  
FLOW               CUBIC FEET PER SECOND  
STORAGE VOLUME           ACRE-FEET  
SURFACE AREA            ACRES  
TEMPERATURE            DEGREES FAHRENHEIT

JP        MULTI-PLAN OPTION  
NPLAN          1 NUMBER OF PLANS

JR        MULTI-RATIO OPTION  
RATIOS OF PRECIPITATION  
1.00

1

PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS  
FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES  
TIME TO PEAK IN HOURS

RATIOS APPLIED TO PRECIPITATION					
OPERATION	STATION	AREA	PLAN	RATIO 1	
				1.00	
HYDROGRAPH AT					
+	13A	.10	1	FLOW TIME	12. 4.25
HYDROGRAPH AT					
+	13B	.10	1	FLOW TIME	66. 4.00

\*\*\* NORMAL END OF HEC-1 \*\*\*

**CROSS-SECTION 13A**  
**Worksheet for Irregular Channel**

<b>Project Description</b>	
Project File	e:\cb879\xs.fm2
Worksheet	CROSS SECTION 13A
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Water Elevation

**Input Data**

Channel Slope                    0.014910 ft/ft

Elevation range: 2,392.13 ft to 2,403.12 ft.

Station (ft)	Elevation (ft)	Start Station	End Station	Roughness
0.00	2,399.78	0.00	1,299.20	0.030
3.87	2,399.54			
25.46	2,397.27			
55.80	2,395.00			
61.36	2,395.00			
68.59	2,395.00			
93.20	2,395.00			
98.38	2,395.00			
101.45	2,396.15			
162.19	2,397.79			
170.96	2,395.08			
171.30	2,395.00			
179.81	2,392.95			
191.17	2,392.13			
192.72	2,393.22			
192.99	2,395.00			
197.75	2,396.16			
213.10	2,395.97			
214.41	2,395.00			
224.44	2,395.00			
233.91	2,395.00			
240.92	2,396.28			
248.16	2,398.13			
259.23	2,399.66			
278.53	2,400.85			
298.19	2,401.34			
318.54	2,402.16			
337.99	2,402.89			
354.30	2,403.12			
409.03	2,401.68			
425.05	2,401.58			
451.26	2,401.20			
471.38	2,400.58			
473.73	2,400.45			
479.77	2,400.22			
484.92	2,400.31			

**CROSS-SECTION 13A**  
**Worksheet for Irregular Channel**

491.46	2,400.33
515.90	2,400.11
533.98	2,399.44
554.27	2,398.25
577.11	2,397.88
604.51	2,397.48
622.66	2,395.00
625.85	2,395.00
638.44	2,395.00
642.67	2,394.02
647.18	2,395.00
659.54	2,397.25
670.56	2,398.49
697.15	2,398.51
705.24	2,398.10
725.97	2,396.82
745.65	2,395.36
750.26	2,395.00
768.39	2,393.49
777.99	2,393.27
815.24	2,394.26
826.33	2,395.00
829.92	2,395.35
846.33	2,398.03
862.72	2,400.09
881.77	2,401.79
958.08	2,400.47
971.37	2,399.81
977.08	2,399.58
982.68	2,399.39
989.36	2,399.45
993.69	2,399.36
1,014.27	2,400.49
1,037.02	2,401.58
1,057.89	2,401.47
1,079.78	2,401.37
1,086.30	2,401.14
1,095.02	2,400.85
1,099.20	2,400.84
1,102.91	2,400.94
1,110.44	2,401.29
1,117.48	2,401.33
1,138.69	2,401.73
1,155.70	2,402.17
1,178.85	2,402.42
1,203.89	2,402.49
1,219.60	2,402.38
1,230.86	2,402.16
1,256.08	2,401.76
1,263.66	2,401.64
1,298.95	2,401.30

CROSS-SECTION 13A  
Worksheet for Irregular Channel

1,299.20            2,401.30  
Discharge        12.00      cfs

---

Results

Wtd. Mannings Coefficient	0.030
Water Surface Elevation	2,392.85      ft
Flow Area	3.98      ft <sup>2</sup>
Wetted Perimeter	11.27      ft
Top Width	11.02      ft
Height	0.72      ft
Critical Depth	2,392.82      ft
Critical Slope	0.019301 ft/ft
Velocity	3.02      ft/s
Velocity Head	0.14      ft
Specific Energy	2,392.99      ft
Froude Number	0.89

---

Flow is subcritical.

**CROSS-SECTION 13A**  
**Cross Section for Irregular Channel**

---

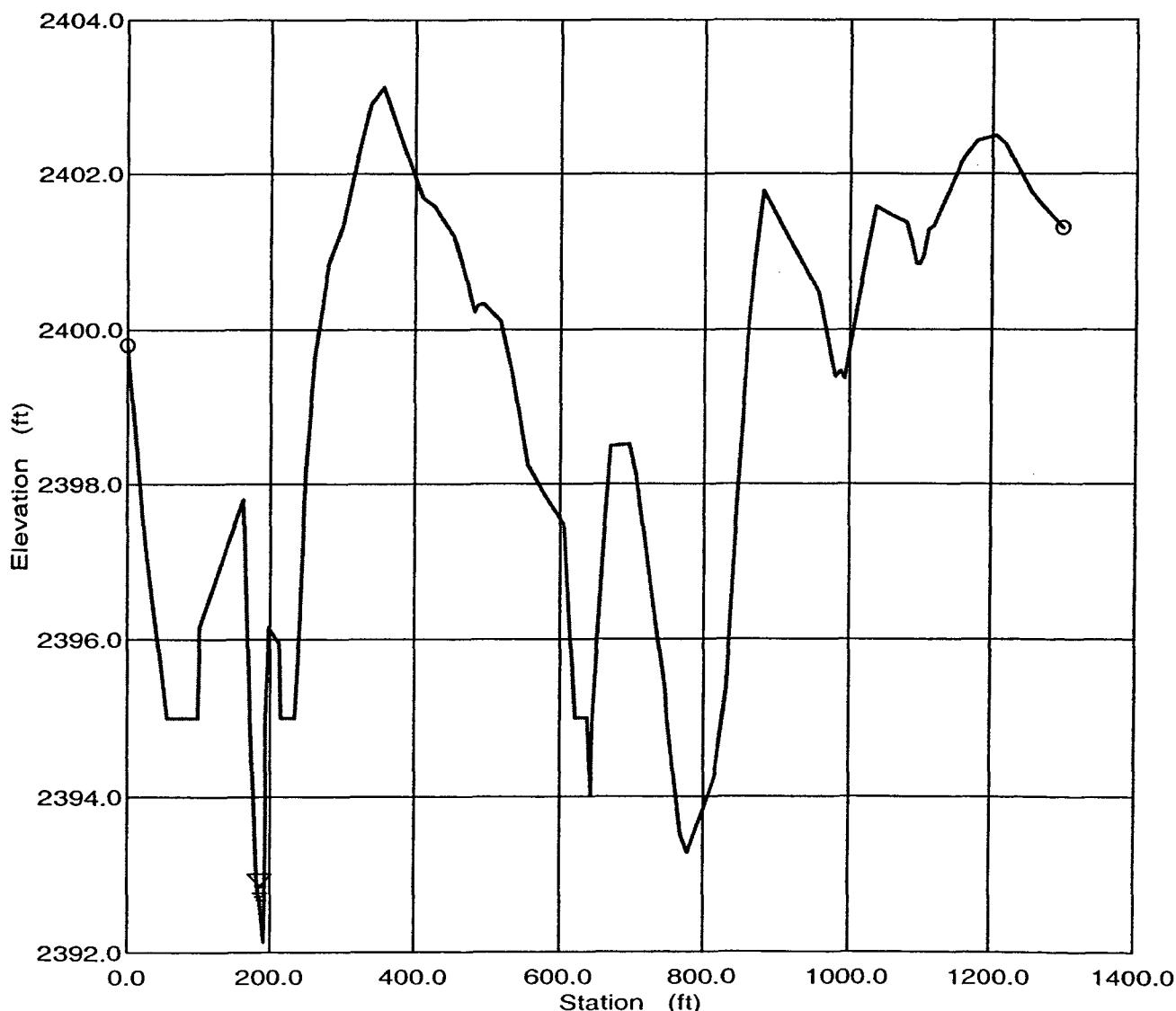
Project Description	
Project File	e:\cb879\xs.fm2
Worksheet	CROSS SECTION 13A
Flow Element	Irregular Channel
Method	Manning's Formula
Solve For	Water Elevation

---

---

Section Data	
Wtd. Mannings Coefficient	0.030
Channel Slope	0.014910 ft/ft
Water Surface Elevation	2,392.85 ft
Discharge	12.00 cfs

---



**REVISED CROSS-SECTION 34**  
**Worksheet for Irregular Channel**

---

**Project Description**

---

Project File e:\cb879\xs.fm2  
Worksheet CROSS SECTION 13B - XS 34 - LB Flows  
Flow Element Irregular Channel  
Method Manning's Formula  
Solve For Water Elevation

---

---

**Input Data**

---

Channel Slope 1.5000 %  
Elevation range: 2,305.00 ft to 2,310.00 ft.

Station (ft)	Elevation (ft)	Start Station	End Station	Roughness
0.00	2,310.00	0.00	612.00	0.030
198.00	2,305.00			
231.00	2,305.00			
612.00	2,310.00			

Discharge 66.00 cfs

---

---

**Results**

---

Wtd. Mannings Coefficient 0.030  
Water Surface Elevation 2,305.42 ft  
Flow Area 24.42 ft<sup>2</sup>  
Wetted Perimeter 82.14 ft  
Top Width 82.13 ft  
Height 0.42 ft  
Critical Depth 2,305.39 ft  
Critical Slope 0.020055 ft/ft  
Velocity 2.70 ft/s  
Velocity Head 0.11 ft  
Specific Energy 2,305.54 ft  
Froude Number 0.87  
Flow is subcritical.

---

REVISED CROSS-SECTION 34  
Cross Section for Irregular Channel

---

Project Description

---

Project File e:\cb879\xs.fm2  
Worksheet CROSS SECTION 13B - XS 34 - LB Flows  
Flow Element Irregular Channel  
Method Manning's Formula  
Solve For Water Elevation

---

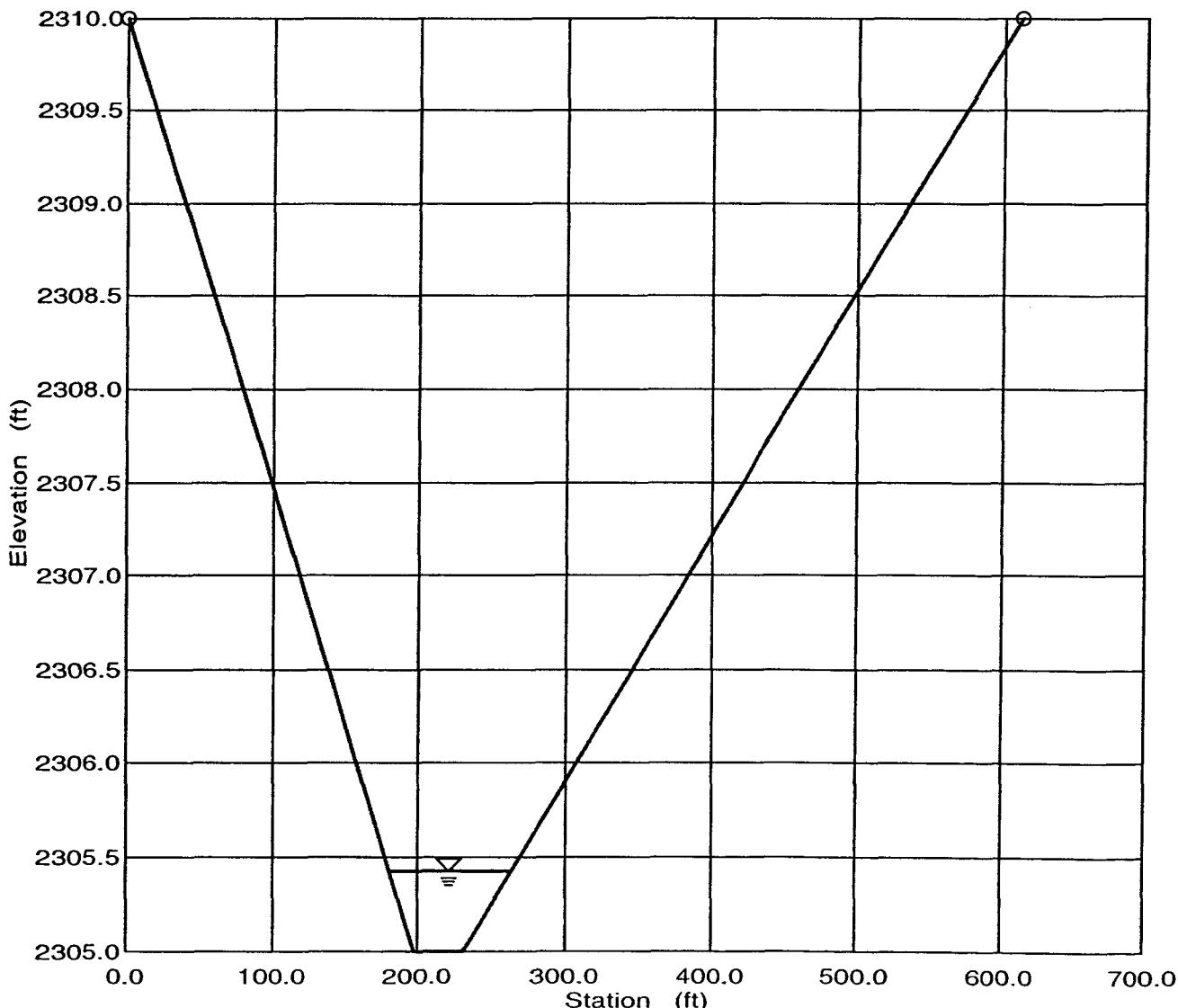
---

Section Data

---

Wtd. Mannings Coefficient 0.030  
Channel Slope 1.5000 %  
Water Surface Elevation 2,305.42 ft  
Discharge 66.00 cfs

---



## **APPENDIX D**

---

Excerpts from the CLOMR for Area Downstream of Upper Blue  
Diamond Detention Basin

---

CLOMR HEC-1 Model

```

*****
* FLOOD HYDROGRAPH PACKAGE (HEC-1) *
* MAY 1991 *
* VERSION 4.0.1E *
* Lahey F77L-EM/32 version 5.01 *
* Dodson & Associates, Inc. *
* RUN DATE 02/19/00 TIME 14:51:22 *
*****

```

```

X   X   XXXXXXXX  XXXXX      X
X   X   X           X   X      XX
X   X   X           X
XXXXXX  XXXX      X           XXXXX  X
X   X   X           X
X   X   X           X   X      X
X   X   XXXXXXXX  XXXXX      XXX

```

THIS PROGRAM REPLACES ALL PREVIOUS VERSIONS OF HEC-1 KNOWN AS HEC1 (JAN 73), HEC1GS, HEC1DB, AND HEC1KW.

THE DEFINITIONS OF VARIABLES -RTIMP- AND -RTIOR- HAVE CHANGED FROM THOSE USED WITH THE 1973-STYLE INPUT STRUCTURE.  
 THE DEFINITION OF -AMSKK- ON RM-CARD WAS CHANGED WITH REVISIONS DATED 28 SEP 81. THIS IS THE FORTRAN77 VERSION  
 NEW OPTIONS: DAMBREAK OUTFLOW SUBMERGENCE, SINGLE EVENT DAMAGE CALCULATION, DSS:WRITE STAGE FREQUENCY,  
 DSS:READ TIME SERIES AT DESIRED CALCULATION INTERVAL LOSS RATE:GREEN AND AMPT INFILTRATION  
 KINEMATIC WAVE: NEW FINITE DIFFERENCE ALGORITHM

1 HEC-1 INPUT PAGE 1

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
1	ID      File: BD.DAT
2	ID
3	ID Post, Buckley, Schuh & Jernigan, Inc.
4	ID
5	ID EXISTING DRAINAGE CONDITIONS RUN
6	ID
7	ID *****
8	ID *****
9	ID *
10	ID * CLOMR *
11	ID *
12	ID *
13	ID * 02/19/00 *
14	ID *
15	ID * FILE : BD.DAT *
16	ID *
17	ID *
18	ID *
19	ID *
20	ID * EXISTING *
21	ID *
22	ID * CONDITIONS *
23	ID *****
24	ID *****
25	ID
26	ID THIS ANALYSIS IS TO SIMULATE A 55%/45% FLOW SPLIT
27	ID *DIAGRAM
*** FREE ***	
28	IT      5      0      0      300
29	IO      5      0      0
30	IN      5      0      0
31	JR      PREC    1.00    0.98    0.57
32	ID * BLUE DIAMOND DETENTION BASINS REFERENCED FROM MPU BASIN DELINEATION BY COE
33	ID *****
34	ID BLUE DIAMOND DETENTION BASIN DESIGN.....FILE = BDDB.LAI
35	ID SOURCE OF FILE= NEWBDDB1.LAI.....
36	ID *****
37	ID MODEL CREATED TO DUPLICATE THE BLUE DIAMOND DETENTION BASIN DESIGN
38	ID AS OF JAN 31 1996.
39	ID FILE NEWBDDB1.LAI DATED 6-23-96 WAS USED, AND RAINFALL & PATTERN ADJUSTED
40	ID RAIN = 1.76 (CURVE) X 1.26 (TF) = 2.22 IN. AS STATED IN TROP. DM DRAFT
41	ID PATTERN # = 4.95 DA =68.25 DARF =
42	ID JLF 2/1/96
43	ID *****
44	ID (A REVISED EL-VOL-STO IS COMING SOON. THIS DOES NOT INCLUDE IT)
45	ID ELEVATIONS ARE NGVD
46	ID *****
47	ID COMMENTS BELOW THIS POINT WERE IN FILE NEWBDDB1.LAI AND THEREFORE
48	ID RAIN, DARF & PATTERNS BELOW THIS POINT ARE NOT APPLICABLE TO THIS FILE
49	ID *****
50	ID REVISED TO ESTIMATE RUNOFF FROM 0.6 SQ.MI. AREA CONTRIBUTING TO A CP AT
51	ID BLUE DIAMOND ROAD, JUST U/S OF BLUE DIAMOND DB.
52	ID

1 HEC-1 INPUT PAGE 2

LINE	ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10
53	ID

54 ID DEPTH-AREA AND PATTERN FOR AREA ABOVE BLUE DIAMOND DB.  
 55 ID DA = 68.25 MI<sup>2</sup>. DARP = . PATTERN # = 4.9.  
 56 ID RAIN = 1.75 (CURVE) X 1.26 (TF) = 2.20 IN.  
 57 ID  
 58 ID HN 6/22/95  
 59 ID ======  
 60 ID  
 61 KK 71  
 62 KM RUNOFF FROM SCS SUBAREA 71  
 63 BA 6.9  
 64 KM LAS VEGAS STANDARD PROJECT SUMMER THUNDERSTORM  
 65 PB 2.22  
 66 PI .533 1.039 .559 .786 .891 .165 .261 .204 .077 .232  
 67 PI .055 .131 .081 .185 .139 .212 .264 .268 .293 .237  
 68 PI .233 .157 .386 .270 .268 .140 .109 .029 .188 .105  
 69 PI .157 .174 .344 .388 .514 .718 .589 1.256 1.023 1.255  
 70 PI 1.842 2.158 .282 .302 .574 .710 .293 .135 .233 .161  
 71 PI .258 .168 .373 .514 .789 .554 .735 .741 .108 .108  
 72 PI .132 .053 .053 .053 .105 .025 .053 .027 .080 .025  
 73 PI .053 .027  
 74 LU 0 0.50 0  
 75 UD 0.42  
 76 KK 71R  
 77 KM ROUTE 71 THRU 501  
 78 RM 5 0.39 0.15  
 79 KK 501  
 80 KM RUNOFF FROM SCS SUBAREA 501  
 81 BA 11.28  
 82 KM LAS VEGAS STANDARD PROJECT SUMMER THUNDERSTORM  
 83 PB 2.22  
 84 PI .533 1.039 .559 .786 .891 .165 .261 .204 .077 .232  
 85 PI .055 .131 .081 .185 .139 .212 .264 .268 .293 .237  
 86 PI .233 .157 .386 .270 .268 .140 .109 .029 .188 .105  
 87 PI .157 .174 .344 .388 .514 .718 .589 1.256 1.023 1.255  
 88 PI 1.842 2.158 .282 .302 .574 .710 .293 .135 .233 .161  
 89 PI .258 .168 .373 .514 .789 .554 .735 .741 .108 .108  
 90 PI .132 .053 .053 .053 .105 .025 .053 .027 .080 .025  
 91 PI .053 .027  
 92 LU 0 0.50 0  
 93 UD 0.45  
 94 KK 501C  
 95 KM COMBINE 71 AND 501  
 96 HC 2  
 97 KK 501R  
 98 KM ROUTE 501C THRU 502  
 99 RM 5 0.40 0.15

1

HEC-1 INPUT

PAGE 3

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

100 KK 502  
 101 KM RUNOFF FROM SCS SUBAREA 502  
 102 KM DRAINAGE AREA CORRECTED BY JP 2/21/92  
 103 BA 27.40  
 104 KM LAS VEGAS STANDARD PROJECT SUMMER THUNDERSTORM  
 105 PB 2.22  
 106 PI .533 1.039 .559 .786 .891 .165 .261 .204 .077 .232  
 107 PI .055 .131 .081 .185 .139 .212 .264 .268 .293 .237  
 108 PI .233 .157 .386 .270 .268 .140 .109 .029 .188 .105  
 109 PI .157 .174 .344 .388 .514 .718 .589 1.256 1.023 1.255  
 110 PI 1.842 2.158 .282 .302 .574 .710 .293 .135 .233 .161  
 111 PI .258 .168 .373 .514 .789 .554 .735 .741 .108 .108  
 112 PI .132 .053 .053 .053 .105 .025 .053 .027 .080 .025  
 113 PI .053 .027  
 114 LU 0 0.50 0  
 115 UD 0.95  
 116 KK 502C  
 117 KM COMBINE 501R AND 502  
 118 HC 2  
 119 KK 72  
 120 KM RUNOFF FROM SCS SUBAREA 72  
 121 BA 6.58  
 122 KM LAS VEGAS STANDARD PROJECT SUMMER THUNDERSTORM  
 123 PB 2.22  
 124 PI .533 1.039 .559 .786 .891 .165 .261 .204 .077 .232  
 125 PI .055 .131 .081 .185 .139 .212 .264 .268 .293 .237  
 126 PI .233 .157 .386 .270 .268 .140 .109 .029 .188 .105  
 127 PI .157 .174 .344 .388 .514 .718 .589 1.256 1.023 1.255  
 128 PI 1.842 2.158 .282 .302 .574 .710 .293 .135 .233 .161  
 129 PI .258 .168 .373 .514 .789 .554 .735 .741 .108 .108  
 130 PI .132 .053 .053 .053 .105 .025 .053 .027 .080 .025  
 131 PI .053 .027  
 132 LU 0 0.50 0  
 133 UD 0.46  
 134 KK 502CC  
 135 KM COMBINE 72 AND 502C  
 136 HC 2  
 137 KK 502R  
 138 KM ROUTE 502CC THRU 503  
 139 RM 4 0.36 0.15  
 140 KK 73  
 141 KM RUNOFF FROM SCS SUBAREA 73  
 142 BA 4.02  
 143 KM LAS VEGAS STANDARD PROJECT SUMMER THUNDERSTORM  
 144 PB 2.22  
 145 PI .533 1.039 .559 .786 .891 .165 .261 .204 .077 .232

146 PI .055 .131 .081 .185 .139 .212 .264 .268 .293 .237  
147 PI .233 .157 .386 .270 .268 .140 .109 .029 .188 .105  
148 PI .157 .174 .344 .388 .514 .718 .589 1.256 1.023 1.255

HEC-1 INPUT

PAGE 4

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

149 PI 1.842 2.158 .282 .302 .574 .710 .293 .135 .233 .161  
150 PI .258 .168 .373 .514 .789 .554 .735 .741 .108 .108  
151 PI .132 .053 .053 .053 .105 .025 .053 .027 .080 .025  
152 PI .053 .027  
153 LU 0 0.50 0  
154 UD 0.47

155 KK 73R  
156 KM ROUTE 73 THRU 503  
157 RM 4 0.36 0.15

158 KK 503  
159 KM RUNOFF FROM SCS SUBAREA 503  
160 BA 5.54  
161 KM LAS VEGAS STANDARD PROJECT SUMMER THUNDERSTORM  
162 PB 2.22  
163 PI .533 1.039 .559 .786 .891 .165 .261 .204 .077 .232  
164 PI .055 .131 .081 .185 .139 .212 .264 .268 .293 .237  
165 PI .233 .157 .386 .270 .268 .140 .109 .029 .188 .105  
166 PI .157 .174 .344 .388 .514 .718 .589 1.256 1.023 1.255  
167 PI 1.842 2.158 .282 .302 .574 .710 .293 .135 .233 .161  
168 PI .258 .168 .373 .514 .789 .554 .735 .741 .108 .108  
169 PI .132 .053 .053 .053 .105 .025 .053 .027 .080 .025  
170 PI .053 .027  
171 LU 0 0.50 0  
172 UD 0.94

173 KK 503C  
174 KM COMBINE 502R, 73, AND 503  
175 HC 3

176 KK 503R  
177 KM ROUTE 503 THRU 504  
178 RM 4 0.36 0.15

179 KK 504  
180 KM RUNOFF FROM SCS SUBAREA 504  
181 BA 6.53  
182 KM LAS VEGAS STANDARD PROJECT SUMMER THUNDERSTORM  
183 PB 2.22  
184 PI .533 1.039 .559 .786 .891 .165 .261 .204 .077 .232  
185 PI .055 .131 .081 .185 .139 .212 .264 .268 .293 .237  
186 PI .233 .157 .386 .270 .268 .140 .109 .029 .188 .105  
187 PI .157 .174 .344 .388 .514 .718 .589 1.256 1.023 1.255  
188 PI 1.842 2.158 .282 .302 .574 .710 .293 .135 .233 .161  
189 PI .258 .168 .373 .514 .789 .554 .735 .741 .108 .108  
190 PI .132 .053 .053 .053 .105 .025 .053 .027 .080 .025  
191 PI .053 .027  
192 LU 0 0.50 0  
193 UD 0.91

HEC-1 INPUT

PAGE 5

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

194 KK INBD  
195 KM COMBINE 503R AND 504 - FLOW ABOVE BLUE DIAMOND  
196 KM CHANNEL CAPACITY = 12300 CFS  
197 HC 2

\* =====

198 KK BD  
199 KM BLUE DIAMOND DETENTION BASIN  
200 KM 100YR COMPUTED PROBABILITY DETENTION DESIGN ... ON-LINE  
201 KM BROAD-CRESTED WEIR SPILLWAY - 1000 FT LONG  
202 KM 30" DIA OUTLET  
203 KM SPILLWAY CREST = 2915 FT NGVD  
204 KM TOP OF DAM = 2930 FT NGVD  
205 RS 1 STOR -1  
206 SV 0 1.6 12.8 37.2 79.3 150.8 264.7 433.2 668.8 973.4  
207 SV 1340.8 1775.1 2281.8 2398.2 2514.5 2630.9 2747.2 2863.6 3444.8 4026.3  
208 SQ 0 38 74 97 116 132 147 160 172 183  
209 SQ 194 204 214 3716 10117 18406 28221 39354 110911 203572  
210 SE 2857 2860 2865 2870 2875 2880 2885 2890 2895 2900  
211 SE 2905 2910 2915 2916 2917 2918 2919 2920 2925 2930

\* =====

\* END OF COE BASINS, BEGIN CLOMR BASINS

212 KK OFFSITEBASIN  
213 KM EXISTING OFFSITE BASIN  
214 BA 0.5953  
215 PB 2.77  
216 LS 0 88  
217 UD 0.307

\*

218 KK COMB-1  
219 KM COMBINED BD AND OFFSITE BASIN  
220 HC 2

\*

221 KK R0  
222 KM ROUTE BD THROUGH BASIN-40  
223 RK 11200 0.014 0.030 0 TRAP 100

\*

224 KK BASIN-40  
225 KM EXISTING BASIN BASIN-40  
226 BA 0.8775  
227 PB 2.77

228 PC .000 .020 .057 .070 .087 .108 .124 .130 .130 .130  
 229 PC .130 .130 .130 .133 .140 .142 .148 .158 .172 .181  
 230 PC .190 .197 .199 .200 .201 .204 .214 .229 .241 .249  
 231 PC .251 .256 .270 .278 .281 .283 .295 .322 .352 .409  
 232 PC .499 .590 .710 .744 .781 .812 .819 .835 .851 .856  
 233 PC .860 .868 .876 .888 .910 .926 .937 .950 .970 .976  
 234 PC .982 .985 .987 .989 .990 .993 .993 .994 .995 .998  
 235 PC .998 .999 1.00  
 236 LS 0 84

1

HEC-1 INPUT

PAGE 6

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

237 UD 0.451

238 KK C-0  
239 KM COMBINED R0 AND BASIN-40  
240 HC 2

241 KK SPLIT1  
242 KM 45% FLOW DIVERTED FROM BASIN 38 TO BASIN 34  
243 DT SPLIT1  
244 DI 0 288 577 1153  
245 DQ 0 131 262 523

246 KKKBASIN-38  
247 KM EXISTING BASIN-38  
248 BA 0.1326  
249 PB 2.77  
250 LS 0 73  
251 UD 0.472

252 KKKBASIN-36  
253 KM EXISTING BASIN-36  
254 BA 0.0585  
255 PB 2.77  
256 LS 0 88  
257 UD 0.294

258 KK C-1A  
259 KM COMBINED BASIN-36 AND BASIN-38  
260 HC 2

261 KK C-1  
262 KM COMBINED C-1A AND C-0  
263 HC 2

264 KK R2  
265 KM ROUTE C-1 THROUGH BASIN-26  
266 RK 6100 0.017 0.030 0 TRAP 100

267 KKKBASIN-26  
268 KM EXISTING BASIN-26  
269 BA 0.1432  
270 PB 2.77  
271 LS 0 66  
272 UD 0.589

1

HEC-1 INPUT

PAGE 7

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

273 KK C-2  
274 KM COMBINED R2 AND BASIN-26  
275 HC 2

276 KK R3  
277 KM ROUTE C-2 THROUGH BASIN-17  
278 RK 6400 0.017 0.030 0 TRAP 100

279 KKKBASIN-17  
280 KM EXISTING BASIN-17  
281 BA 0.2861  
282 PB 2.77  
283 LS 0 66  
284 UD 0.602

285 KK C-3  
286 KM COMBINED R3 AND BASIN-17  
287 HC 2

288 KK R4  
289 KM ROUTE C-3 THROUGH BASIN-13  
290 RK 6400 0.015 0.030 0 TRAP 100

291 KKKBASIN-13  
292 KM EXISTING BASIN-13  
293 BA 0.2241  
294 PB 2.77  
295 LS 0 73  
296 UD 0.533

297 KK C-27  
298 KM COMBINED R4 AND BASIN-13  
299 HC 2  
\*

300 KKBASIN-28  
301 KM EXISTING BASIN-28  
302 BA 0.0648  
303 PB 2.77  
304 LS 0 63  
305 UD 0.527  
\*

1

HEC-1 INPUT

PAGE 8

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

306 KK R7  
307 KM ROUTE BASIN-28 THROUGH BASIN-19  
308 RK 5900 0.017 0.030 0 TRAP 100  
\*

309 KKBASIN-19  
310 KM EXISTING BASIN-19  
311 BA 0.1372  
312 PB 2.77  
313 LS 0 63  
314 UD 0.640  
\*

315 KK C-7  
316 KM COMBINED R7 AND BASIN-19  
317 HC 2  
\*

318 KK R8  
319 KM ROUTE C-7 THROUGH BASIN-3  
320 RK 4200 0.016 0.030 0 TRAP 100  
\*

321 KK BASIN-3  
322 KM EXISTING BASIN-3  
323 BA 0.1105  
324 PB 2.77  
325 LS 0 63  
326 UD 0.595  
\*

327 KK C-8  
328 KM COMBINED R8 AND BASIN-3  
329 HC 2  
\*

330 KKBASIN-27  
331 KM EXISTING BASIN-27  
332 BA 0.1462  
333 PB 2.77  
334 LS 0 63  
335 UD 0.795  
\*

336 KK R9  
337 KM ROUTE BASIN 27 THROUGH BASIN-18  
338 RK 5900 0.017 0.030 0 TRAP 100  
\*

1

HEC-1 INPUT

PAGE 9

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

339 KKBASIN-18  
340 KM EXISTING BASIN-18  
341 BA 0.1838  
342 PB 2.77  
343 LS 0 63  
344 UD 0.683  
\*

345 KK C-9  
346 KM COMBINED R9 AND BASIN-18  
347 HC 2  
\*

348 KK R10  
349 KM ROUTE C-9 THROUGH BASIN-2  
350 RK 2000 0.016 0.030 0 TRAP 100  
\*

351 KK BASIN-2  
352 KM EXISTING BASIN-2  
353 BA 0.0872  
354 PB 2.77  
355 LS 0 63  
356 UD 0.479  
\*

357 KK C-10  
358 KM COMBINED R10 AND BASIN-2  
359 HC 2  
\*

360 KK C-29  
361 KM COMBINED C-10 AND C-8  
362 HC 2  
\*

363 KK R18

364 KM ROUTE C-27 THROUGH BASIN-22  
365 RK 4200 0.015 0.030 0 TRAP 100  
\*  
366 KKBASIN-22  
367 KM EXISTING BASIN-22  
368 BA 0.2131  
369 PB 2.77  
370 LS 0 69  
371 UD 0.489  
\*

1 HEC-1 INPUT PAGE 10

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

372 KK C-18  
373 KM COMBINED R18 AND BASIN-22  
374 HC 2  
\*

375 KKBASIN-37  
376 KM EXISTING BASIN-37  
377 BA 0.1229  
378 PB 2.77  
379 LS 0 83  
380 UD 0.396  
\*

381 KK R22  
382 KM ROUTE BASIN 37 THROUGH BASIN-25  
383 RK 6100 0.017 0.030 0 TRAP 100  
\*

384 KKBASIN-25  
385 KM EXISTING BASIN-25  
386 BA 0.1428  
387 PB 2.77  
388 LS 0 83  
389 UD 0.373  
\*

390 KK C-22  
391 KM COMBINED R22 AND BASIN-25  
392 HC 2  
\*

393 KK R23  
394 KM ROUTE C-22 THROUGH BASIN-15  
395 RK 4900 0.017 0.030 0 TRAP 100  
\*

396 KKBASIN-15  
397 KM EXISTING BASIN-15  
398 BA 0.0883  
399 PB 2.77  
400 LS 0 78  
401 UD 0.406  
\*

402 KK C-23  
403 KM COMBINED R23 AND BASIN-15  
404 HC 2  
\*

1 HEC-1 INPUT PAGE 11

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

405 KK R28  
406 KM ROUTE C-23 THROUGH BASIN-1  
407 RK 7900 0.014 0.030 0 TRAP 100  
\*

408 KK BASIN-1  
409 KM EXISTING BASIN-1  
410 BA 0.2419  
411 PB 2.77  
412 LS 0 83  
413 UD 0.444  
\*

414 KK C-28  
415 KM COMBINED R28 AND BASIN-1  
416 HC 2  
\*

417 KK C-25  
418 KM COMBINED C-28, C-18, C-27  
419 HC 3  
\*

420 KK R33  
421 KM ROUTE C-25 THROUGH BASIN-33  
422 RK 2500 0.015 0.030 0 TRAP 100  
\*

423 KKBASIN-33  
424 KM EXISTING BASIN-33  
425 BA 0.0954  
426 PB 2.77  
427 LS 0 82  
428 UD 0.310  
\*

429 KK C-33  
430 KM COMBINED C-25 AND BASIN-33

431 HC 2  
 \*  
 432 KKBASIN-29  
 433 KM EXISTING BASIN-29  
 434 BA 0.0422  
 435 PB 2.77  
 436 LS 0 63  
 437 UD 0.533  
 \*  
 1 HEC-1 INPUT PAGE 12  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10  
 438 KK R14  
 439 KM ROUTE BASIN 29 THROUGH BASIN-20  
 440 RK 6100 0.018 0.030 0 TRAP 100  
 \*  
 441 KKBASIN-20  
 442 KM EXISTING BASIN-20  
 443 BA 0.0812  
 444 PB 2.77  
 445 LS 0 63  
 446 UD 0.675  
 \*  
 447 KK C-14  
 448 KM COMBINED R14 AND BASIN-20  
 449 HC 2  
 \*  
 450 KK R15  
 451 KM ROUTE C-14 THROUGH BASIN-4  
 452 RK 6000 0.016 0.030 0 TRAP 100  
 \*  
 453 KK BASIN-4  
 454 KM EXISTING BASIN-4  
 455 BA 0.1402  
 456 PB 2.77  
 457 LS 0 63  
 458 UD 0.600  
 \*  
 459 KK C-15  
 460 KM COMBINED R15 AND BASIN-4  
 461 HC 2  
 \*  
 462 KK R19  
 463 KM ROUTE C-15 THROUGH BASIN-30  
 464 RK 4300 0.012 0.030 0 TRAP 100  
 \*  
 465 KKBASIN-30  
 466 KM EXISTING BASIN-30  
 467 BA 0.2100  
 468 PB 2.77  
 469 LS 0 74  
 470 UD 0.448  
 \*  
 1 HEC-1 INPUT PAGE 13  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10  
 471 KK C-19  
 472 KM COMBINED R19 AND BASIN-30  
 473 HC 2  
 \*  
 474 KK C-34  
 475 KM COMBINED C-19 AND C-34  
 476 HC 2  
 \*  
 477 KKBASIN-24  
 478 KM EXISTING BASIN-24  
 479 BA 0.0875  
 480 PB 2.77  
 481 LS 0 63  
 482 UD 0.713  
 \*  
 483 KK R11  
 484 KM ROUTE BASIN 33 THROUGH BASIN-11  
 485 RK 7850 0.016 0.030 0 TRAP 100  
 \*  
 486 KKBASIN-11  
 487 KM EXISTING BASIN-11  
 488 BA 0.2885  
 489 PB 2.77  
 490 LS 0 63  
 491 UD 0.729  
 \*  
 492 KK C-11  
 493 KM COMBINED R11 AND BASIN-11  
 494 HC 2  
 \*  
 495 KKBASIN-21  
 496 KM EXISTING BASIN-21  
 497 BA 0.3303

498 PB 2.77  
 499 LS 0 63  
 500 UD 0.687  
 \*  
 501 KK R12  
 502 KM ROUTE BASIN 21 THROUGH BASIN-6  
 503 RK 5500 0.017 0.030 0 TRAP 100  
 \*  
 1 HEC-1 INPUT PAGE 14  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10  
 504 KK BASIN-6  
 505 KM EXISTING BASIN-6  
 506 BA 0.4666  
 507 PB 2.77  
 508 LS 0 63  
 509 UD 0.564  
 \*  
 510 KK C-12  
 511 KM COMBINED R12 AND BASIN-6  
 512 HC 2  
 \*  
 513 KK R13  
 514 KM ROUTE C-12 THROUGH BASIN-5  
 515 RK 5700 0.016 0.030 0 TRAP 100  
 \*  
 516 KK BASIN-5  
 517 KM EXISTING BASIN-5  
 518 BA 0.5443  
 519 PB 2.77  
 520 LS 0 65  
 521 UD 0.537  
 \*  
 522 KK C-13  
 523 KM COMBINED R13 AND BASIN-5  
 524 HC 2  
 \*  
 525 KRBASIN-32  
 526 KM EXISTING BASIN-32  
 527 BA 0.0378  
 528 PB 2.77  
 529 LS 0 63  
 530 UD 0.489  
 \*  
 531 KK R16  
 532 KM ROUTE BASIN 32 THROUGH BASIN-23  
 533 RK 5350 0.017 0.030 0 TRAP 100  
 \*  
 534 KKBASIN-23  
 535 KM EXISTING BASIN-23  
 536 BA 0.1472  
 537 PB 2.77  
 538 LS 0 63  
 539 UD 0.612  
 \*  
 1 HEC-1 INPUT PAGE 15  
 LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10  
 540 KK C-16  
 541 KM COMBINED R16 AND BASIN-23  
 542 HC 2  
 \*  
 543 KK R17  
 544 KM ROUTE C-16 THROUGH BASIN-7  
 545 RK 4400 0.016 0.030 0 TRAP 100  
 \*  
 546 KK BASIN-7  
 547 KM EXISTING BASIN-7  
 548 BA 0.1380  
 549 PB 2.77  
 550 LS 0 63  
 551 UD 0.584  
 \*  
 552 KK C-17  
 553 KM COMBINED R17 AND BASIN-7  
 554 HC 2  
 \*  
 555 KKBASIN-10  
 556 KM EXISTING BASIN-10  
 557 BA 0.1594  
 558 PB 2.77  
 559 LS 0 63  
 560 UD 0.801  
 \*  
 561 KK BASIN-8  
 562 KM EXISTING BASIN-8  
 563 BA 0.0536  
 564 PB 2.77  
 565 LS 0 63  
 566 UD 0.569

\*  
567 KK BASIN-9  
568 KM EXISTING BASIN-9  
569 BA 0.0490  
570 PB 2.77  
571 LS 0 63  
572 UD 0.527

573 KK C-26  
574 KM COMBINED C-11, C-13, C-17, C-19, BASINS 8, 9, 10  
575 HC 7

1

HEC-1 INPUT

PAGE 16

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

576 KKBASIN-34  
577 KM EXISTING BASIN-34  
578 BA 0.1932  
579 PB 2.77  
580 LS 0 69  
581 UD 0.654

582 KK SPLIT1  
583 KM RETREIVE FLOW SPLIT FROM BASIN 38  
584 DR SPLIT1

585 KKBASIN-35  
586 KM EXISTING BASIN-35  
587 BA 0.1091  
588 PB 2.77  
589 LS 0 78  
590 UD 0.657

591 KK C-1B  
592 KM COMBINED SPLIT1, BASIN-35, AND BASIN-34  
593 HC 3

594 KK R5  
595 KM ROUTE C-1B THROUGH BASIN-12  
596 RK 7100 0.018 0.030 0 TRAP 100

597 KKBASIN-12  
598 KM EXISTING BASIN-12  
599 BA 0.3271  
600 PB 2.77  
601 LS 0 64  
602 UD 0.646

603 KK C-5  
604 KM COMBINED R5 AND BASIN-12  
605 HC 2

606 KKBASIN-14  
607 KM EXISTING BASIN-14  
608 BA 0.2730  
609 PB 2.77  
610 LS 0 88  
611 UD 0.578

1

HEC-1 INPUT

PAGE 17

LINE ID.....1.....2.....3.....4.....5.....6.....7.....8.....9.....10

612 KKBASIN-16  
613 KM EXISTING BASIN-16  
614 BA 0.2562  
615 PB 2.77  
616 LS 0 88  
617 UD 0.542

618 KKBASIN-31  
619 KM EXISTING BASIN-31  
620 BA 0.1536  
621 PB 2.77  
622 LS 0 88  
623 UD 0.291

624 ZZ

1

SCHEMATIC DIAGRAM OF STREAM NETWORK

INPUT LINE	(V) ROUTING	(--->) DIVERSION OR PUMP FLOW
NO.	(.) CONNECTOR	(<---) RETURN OF DIVERTED OR PUMPED FLOW
61	71 V V	
76	71R . .	
79	. 501 . .	

94 501C.....  
V  
V  
97 501R  
  
100 . 502  
. .  
116 502C.....  
. .  
119 . 72  
. .  
134 502CC.....  
V  
V  
137 502R  
. .  
140 . 73  
. V  
. V  
155 . 73R  
. .  
158 . . 503  
. .  
173 503C.....  
V  
V  
176 503R  
. .  
179 . 504  
. .  
194 INBD.....  
V  
V  
198 BD  
  
212 . OFFSITE  
. .  
218 COMB-1.....  
V  
V  
221 R0  
  
224 . BASIN-40  
  
238 C-0.....  
. .  
243 .-----> SPLIT1  
241 SPLIT1  
. .  
246 . BASIN-38  
. .  
252 . . BASIN-36  
. .  
258 . C-1A.....  
. .  
261 C-1.....  
V  
V  
264 R2  
  
267 . BASIN-26  
. .  
273 C-2.....  
V  
V  
276 R3  
  
279 . BASIN-17  
. .  
285 C-3.....  
V  
V  
288 R4  
  
291 . BASIN-13  
. .  
297 C-27.....  
. .  
300 . BASIN-28  
. V  
. V  
306 . R7

309 . . . . . BASIN-19  
315 . . . . . C-7 . . . .  
  V  
  V  
318 . . . . . R8  
  
321 . . . . . BASIN-3  
  
327 . . . . . C-8 . . . .  
  
330 . . . . . BASIN-27  
  V  
  V  
336 . . . . . R9  
  
339 . . . . . BASIN-18  
  
345 . . . . . C-9 . . . .  
  V  
  V  
348 . . . . . R10  
  
351 . . . . . BASIN-2  
  
357 . . . . . C-10 . . . .  
  
360 . . . . . C-29 . . . .  
  V  
  V  
363 . . . . . R18  
  
366 . . . . . BASIN-22  
  
372 . . . . . C-18 . . . .  
  
375 . . . . . BASIN-37  
  V  
  V  
381 . . . . . R22  
  
384 . . . . . BASIN-25  
  
390 . . . . . C-22 . . . .  
  V  
  V  
393 . . . . . R23  
  
396 . . . . . BASIN-15  
  
402 . . . . . C-23 . . . .  
  V  
  V  
405 . . . . . R28  
  
408 . . . . . BASIN-1  
  
414 . . . . . C-28 . . . .  
  
417 C-25 . . . . .  
  V  
  V  
420 R33  
  
423 . . . . . BASIN-33  
  
429 C-33 . . . . .  
  
432 . . . . . BASIN-29  
  V  
  V  
438 . . . . . R14  
  
441 . . . . . BASIN-20  
  
447 C-14 . . . . .  
  V  
  V  
450 . . . . . R15

453 . . . . . BASIN-4  
459 . . . . .  
460 C-15.....  
461 V  
462 V  
463 R19  
464 . . . . .  
465 . . . . . BASIN-30  
466 . . . . .  
467 C-19.....  
468 . . . . .  
469 C-34.....  
470 . . . . .  
471 . . . . .  
472 BASIN-24  
473 V  
474 V  
475 R11  
476 . . . . .  
477 . . . . .  
478 . . . . .  
479 . . . . .  
480 . . . . .  
481 . . . . .  
482 . . . . .  
483 . . . . .  
484 . . . . .  
485 . . . . .  
486 . . . . . BASIN-11  
487 . . . . .  
488 C-11.....  
489 . . . . .  
490 . . . . .  
491 BASIN-21  
492 V  
493 V  
494 R12  
495 . . . . .  
496 . . . . .  
497 . . . . .  
498 . . . . .  
499 . . . . .  
500 . . . . .  
501 . . . . . R12  
502 . . . . .  
503 . . . . .  
504 . . . . .  
505 . . . . .  
506 . . . . .  
507 . . . . .  
508 . . . . .  
509 . . . . .  
510 . . . . . C-12.....  
511 V  
512 V  
513 R13  
514 . . . . .  
515 . . . . .  
516 . . . . . BASIN-5  
517 . . . . .  
518 . . . . .  
519 . . . . .  
520 . . . . .  
521 . . . . .  
522 . . . . . C-13.....  
523 . . . . .  
524 . . . . .  
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591 . . . . . C-1B.....  
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594 R5  
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612 . . . . . BASIN-16  
 618 . . . . . BASIN-31  
 (\*\*\* RUNOFF ALSO COMPUTED AT THIS LOCATION  
 1\*\*\*\*\*  
 \*\*\*\*  
 \* FLOOD HYDROGRAPH PACKAGE (HEC-1) \* \* U.S. ARMY CORPS OF ENGINEERS  
 \* MAY 1991 \* \* HYDROLOGIC ENGINEERING CENTER  
 \* VERSION 4.0.1E \* \* 609 SECOND STREET  
 \* Lahey F77L-EM/32 version 5.01 \* \* DAVIS, CALIFORNIA 95616  
 \* Dodson & Associates, Inc. \* \* (916) 551-1748  
 \* RUN DATE 02/19/00 TIME 14:51:22 \* \*  
 \*\*\*\*

File: BD.DAT

Post, Buckley, Schuh & Jernigan, Inc.

EXISTING DRAINAGE CONDITIONS RUN

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 \* CLOMR \*  
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 \* 02/19/00 \*  
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THIS ANALYSIS IS TO SIMULATE A 55%/45% FLOW SPLIT

29 IO OUTPUT CONTROL VARIABLES  
 IPRNT 5 PRINT CONTROL  
 IPLOT 0 PLOT CONTROL  
 QSCAL 0. HYDROGRAPH PLOT SCALE  
 \* BLUE DIAMOND DETENTION BASINS REFERENCED FROM MPU BASIN DELINEATION BY COE  
 \*\*\*\*\*  
 BLUE DIAMOND DETENTION BASIN DESIGN.....FILE = BDDB.LA1  
 SOURCE OF FILE: NEWBDB1.LAI.....  
 ======  
 MODEL CREATED TO DUPLICATE THE BLUE DIAMOND DETENTION BASIN DESIGN  
 AS OF JAN 31 1996.  
 FILE NEWBDB1.LAI DATED 6-23-96 WAS USED, AND RAINFALL & PATTERN ADJUSTED  
 RAIN = 1.76 (CURVE) X 1.26 (TF) = 2.22 IN. AS STATED IN TROP. DM DRAFT  
 PATTERN # = 4.95 DA = 68.25 DARF =  
 JLF 2/1/96  
 \*\*\*\*\*  
 (A REVISED EL-VOL-STO IS COMING SOON. THIS DOES NOT INCLUDE IT)  
 ELEVATIONS ARE NGVD  
 \*\*\*\*\*  
 COMMENTS BELOW THIS POINT WERE IN FILE NEWBDB1.LAI AND THEREFORE  
 RAIN, DARF & PATTERNS BELOW THIS POINT ARE NOT APPLICABLE TO THIS FILE  
 ======  
 REVISED TO ESTIMATE RUNOFF FROM 0.6 SQ.MI. AREA CONTRIBUTING TO A CP AT  
 BLUE DIAMOND ROAD, JUST U/S OF BLUE DIAMOND DB.  
 DEPTH-AREA AND PATTERN FOR AREA ABOVE BLUE DIAMOND DB.  
 DA = 68.25 MI2. DARF = . PATTERN # = 4.9.  
 RAIN = 1.75 (CURVE) X 1.26 (TF) = 2.20 IN.

HN 6/22/95

IT HYDROGRAPH TIME DATA  
 NMIN 5 MINUTES IN COMPUTATION INTERVAL  
 IDATE 1 0 STARTING DATE  
 ITIME 0000 STARTING TIME  
 NQ 300 NUMBER OF HYDROGRAPH ORDINATES  
 NDDATE 2 0 ENDING DATE  
 NTIME 0055 ENDING TIME  
 ICENT 19 CENTURY MARK  
 COMPUTATION INTERVAL 0.08 HOURS  
 TOTAL TIME BASE 24.92 HOURS

ENGLISH UNITS  
DRAINAGE AREA SQUARE MILES

PRECIPITATION DEPTH	INCHES
LENGTH, ELEVATION	FEET
FLOW	CUBIC FEET PER SECOND
STORAGE VOLUME	ACRE-FEET
SURFACE AREA	ACRES
TEMPERATURE	DEGREES FAHRENHEIT

JP            MULTI-PLAN OPTION  
NPLAN            1    NUMBER OF PLANS

JR            MULTI-RATIO OPTION  
RATIOS OF PRECIPITATION  
1.00      0.98      0.57

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PEAK FLOW AND STAGE (END-OF-PERIOD) SUMMARY FOR MULTIPLE PLAN-RATIO ECONOMIC COMPUTATIONS  
FLOWS IN CUBIC FEET PER SECOND, AREA IN SQUARE MILES  
TIME TO PEAK IN HOURS

OPERATION	STATION	AREA	PLAN	RATIOS APPLIED TO PRECIPITATION		
				RATIO 1 1.00	RATIO 2 0.98	RATIO 3 0.57
HYDROGRAPH AT						
+ 71	6.90	1	FLOW TIME	2790.	2699.	984.
ROUTED TO						
+ 71R	6.90	1	FLOW TIME	2456.	2375.	843.
HYDROGRAPH AT						
+ 501	11.28	1	FLOW TIME	4356.	4215.	1534.
2 COMBINED AT						
+ 501C	18.18	1	FLOW TIME	5713.	5518.	1934.
ROUTED TO						
+ 501R	18.18	1	FLOW TIME	5324.	5139.	1785.
HYDROGRAPH AT						
+ 502	27.40	1	FLOW TIME	5951.	5737.	1956.
2 COMBINED AT						
+ 502C	45.58	1	FLOW TIME	11275.	10876.	3741.
HYDROGRAPH AT						
+ 72	6.58	1	FLOW TIME	2511.	2429.	880.
2 COMBINED AT						
+ 502CC	52.16	1	FLOW TIME	12220.	11783.	4015.
ROUTED TO						
+ 502R	52.16	1	FLOW TIME	11697.	11278.	3848.
HYDROGRAPH AT						
+ 73	4.02	1	FLOW TIME	1513.	1464.	528.
ROUTED TO						
+ 73R	4.02	1	FLOW TIME	1336.	1291.	459.
HYDROGRAPH AT						
+ 503	5.54	1	FLOW TIME	1212.	1169.	399.
3 COMBINED AT						
+ 503C	61.72	1	FLOW TIME	13507.	13020.	4423.
ROUTED TO						
+ 503R	61.72	1	FLOW TIME	13013.	12542.	4243.
HYDROGRAPH AT						
+ 504	6.53	1	FLOW TIME	1470.	1418.	484.
2 COMBINED AT						
+ INBD	68.25	1	FLOW TIME	13818.	13313.	4489.
ROUTED TO						
+ BD	68.25	1	FLOW TIME	213.	211.	164.
				8.00	7.92	6.92
				** PEAK STAGES IN FEET **		
				1 STAGE	2914.67	2913.62
				TIME	8.00	7.00
HYDROGRAPH AT						
+ OFFSITE	0.60	1	FLOW TIME	431.	419.	175.
2 COMBINED AT						
+ COMB-1	68.85	1	FLOW TIME	586.	572.	253.
						5.00

ROUTED TO							
+ R0	68.85	1	FLOW TIME	578. 3.92	560. 3.92	250. 5.33	
HYDROGRAPH AT							
+ BASIN-40	0.88	1	FLOW TIME	613. 3.92	593. 3.92	211. 3.92	
2 COMBINED AT							
+ C-0	69.72	1	FLOW TIME	1191. 3.92	1153. 3.92	450. 4.00	
DIVERSION TO							
+ SPLIT1	69.72	1	FLOW TIME	540. 3.92	523. 3.92	204. 4.00	
HYDROGRAPH AT							
+ SPLIT1	69.72	1	FLOW TIME	651. 3.92	630. 3.92	245. 4.00	
HYDROGRAPH AT							
+ BASIN-38	0.13	1	FLOW TIME	48. 4.00	46. 4.00	9. 4.08	
HYDROGRAPH AT							
+ BASIN-36	0.06	1	FLOW TIME	60. 3.75	59. 3.75	25. 3.75	
2 COMBINED AT							
+ C-1A	0.19	1	FLOW TIME	99. 3.83	95. 3.83	29. 3.83	
2 COMBINED AT							
+ C-1	69.91	1	FLOW TIME	745. 3.92	721. 3.92	270. 4.00	
ROUTED TO							
+ R2	69.91	1	FLOW TIME	738. 4.00	715. 4.00	269. 4.17	
HYDROGRAPH AT							
+ BASIN-26	0.14	1	FLOW TIME	25. 4.17	24. 4.17	3. 5.17	
2 COMBINED AT							
+ C-2	70.06	1	FLOW TIME	761. 4.00	737. 4.00	270. 4.17	
ROUTED TO							
+ R3	70.06	1	FLOW TIME	751. 4.08	725. 4.17	266. 4.33	
HYDROGRAPH AT							
+ BASIN-17	0.29	1	FLOW TIME	50. 4.17	47. 4.17	6. 5.25	
2 COMBINED AT							
+ C-3	70.34	1	FLOW TIME	800. 4.08	772. 4.17	270. 4.33	
ROUTED TO							
+ R4	70.34	1	FLOW TIME	798. 4.25	770. 4.25	267. 4.50	
HYDROGRAPH AT							
+ BASIN-13	0.22	1	FLOW TIME	75. 4.00	71. 4.08	13. 4.17	
2 COMBINED AT							
+ C-27	70.57	1	FLOW TIME	863. 4.25	833. 4.25	277. 4.50	
HYDROGRAPH AT							
+ BASIN-28	0.06	1	FLOW TIME	9. 4.08	8. 4.08	1. 5.17	
ROUTED TO							
+ R7	0.06	1	FLOW TIME	9. 4.92	8. 4.92	1. 7.33	
HYDROGRAPH AT							
+ BASIN-19	0.14	1	FLOW TIME	17. 4.25	16. 4.25	2. 5.33	
2 COMBINED AT							
+ C-7	0.20	1	FLOW TIME	22. 4.92	20. 4.92	2. 5.33	
ROUTED TO							
+ R8	0.20	1	FLOW TIME	21. 5.25	20. 5.33	2. 6.42	
HYDROGRAPH AT							
+ BASIN-3	0.11	1	FLOW TIME	14. 4.17	13. 4.17	1. 5.25	
2 COMBINED AT							
+ C-8	0.31	1	FLOW TIME	32. 5.25	30. 5.25	2. 6.42	
HYDROGRAPH AT							
+ BASIN-27	0.15	1	FLOW TIME	16. 4.42	15. 4.50	1. 5.42	
ROUTED TO							
+ R9	0.15	1	FLOW	16.	15.	1.	

			TIME	5.08	5.08	7.08
HYDROGRAPH AT						
+ BASIN-18	0.18	1	FLOW TIME	22. 4.33	20. 4.33	2. 5.33
2 COMBINED AT						
+ C-9	0.33	1	FLOW TIME	33. 5.08	31. 5.08	2. 5.33
ROUTED TO						
+ R10	0.33	1	FLOW TIME	33. 5.17	31. 5.25	2. 5.75
HYDROGRAPH AT						
+ BASIN-2	0.09	1	FLOW TIME	13. 4.08	12. 4.08	1. 5.17
2 COMBINED AT						
+ C-10	0.42	1	FLOW TIME	41. 5.17	38. 5.25	3. 5.67
2 COMBINED AT						
+ C-29	0.73	1	FLOW TIME	72. 5.25	68. 5.25	4. 5.58
ROUTED TO						
+ R18	0.73	1	FLOW TIME	71. 5.42	67. 5.42	4. 6.42
HYDROGRAPH AT						
+ BASIN-22	0.21	1	FLOW TIME	56. 4.00	53. 4.00	7. 5.08
2 COMBINED AT						
+ C-18	0.94	1	FLOW TIME	92. 5.42	86. 5.42	7. 5.08
HYDROGRAPH AT						
+ BASIN-37	0.12	1	FLOW TIME	88. 3.83	85. 3.83	29. 3.92
ROUTED TO						
+ R22	0.12	1	FLOW TIME	85. 4.17	83. 4.08	29. 4.33
HYDROGRAPH AT						
+ BASIN-25	0.14	1	FLOW TIME	105. 3.83	102. 3.83	35. 3.83
2 COMBINED AT						
+ C-22	0.27	1	FLOW TIME	161. 4.08	156. 4.08	45. 4.33
ROUTED TO						
+ R23	0.27	1	FLOW TIME	157. 4.17	154. 4.25	43. 4.67
HYDROGRAPH AT						
+ BASIN-15	0.09	1	FLOW TIME	47. 3.92	45. 3.92	12. 3.92
2 COMBINED AT						
+ C-23	0.35	1	FLOW TIME	191. 4.17	182. 4.17	48. 4.67
ROUTED TO						
+ R28	0.35	1	FLOW TIME	185. 4.42	182. 4.50	46. 5.25
HYDROGRAPH AT						
+ BASIN-1	0.24	1	FLOW TIME	162. 3.92	157. 3.92	54. 3.92
2 COMBINED AT						
+ C-28	0.60	1	FLOW TIME	260. 4.42	251. 4.42	66. 5.25
3 COMBINED AT						
+ C-25	72.11	1	FLOW TIME	1159. 4.25	1110. 4.33	307. 4.50
ROUTED TO						
+ R33	72.11	1	FLOW TIME	1159. 4.33	1108. 4.33	304. 4.58
HYDROGRAPH AT						
+ BASIN-33	0.10	1	FLOW TIME	73. 3.75	71. 3.75	23. 3.75
2 COMBINED AT						
+ C-33	72.20	1	FLOW TIME	1181. 4.33	1130. 4.33	311. 4.58
HYDROGRAPH AT						
+ BASIN-29	0.04	1	FLOW TIME	6. 4.17	5. 4.17	1. 5.17
ROUTED TO						
+ R14	0.04	1	FLOW TIME	6. 5.08	5. 5.08	1. 7.75
HYDROGRAPH AT						
+ BASIN-20	0.08	1	FLOW TIME	10. 4.33	9. 4.33	1. 5.33
2 COMBINED AT						

*	C-14	0.12	1	FLOW TIME	13. 5.08	13. 5.08	1. 5.33
ROUTED TO							
+	R15	0.12	1	FLOW TIME	13. 5.75	13. 5.83	1. 7.42
HYDROGRAPH AT							
+	BASIN-4	0.14	1	FLOW TIME	18. 4.17	17. 4.25	2. 5.25
2 COMBINED AT							
+	C-15	0.26	1	FLOW TIME	23. 5.08	21. 5.08	2. 5.25
ROUTED TO							
+	R19	0.26	1	FLOW TIME	23. 5.50	21. 5.50	2. 6.58
HYDROGRAPH AT							
+	BASIN-30	0.21	1	FLOW TIME	83. 3.92	80. 3.92	16. 4.00
2 COMBINED AT							
+	C-19	0.47	1	FLOW TIME	83. 3.92	80. 3.92	16. 4.00
2 COMBINED AT							
+	C-34	72.68	1	FLOW TIME	1232. 4.33	1178. 4.33	320. 4.58
HYDROGRAPH AT							
+	BASIN-24	0.09	1	FLOW TIME	10. 4.33	9. 4.33	1. 5.33
ROUTED TO							
+	R11	0.09	1	FLOW TIME	10. 5.33	9. 5.42	1. 8.08
HYDROGRAPH AT							
+	BASIN-11	0.29	1	FLOW TIME	33. 4.33	30. 4.33	3. 5.42
2 COMBINED AT							
+	C-11	0.38	1	FLOW TIME	36. 5.25	34. 5.33	3. 5.42
HYDROGRAPH AT							
+	BASIN-21	0.33	1	FLOW TIME	39. 4.33	36. 4.33	4. 5.33
ROUTED TO							
+	R12	0.33	1	FLOW TIME	39. 4.67	36. 4.67	4. 6.42
HYDROGRAPH AT							
+	BASIN-6	0.47	1	FLOW TIME	63. 4.17	58. 4.17	6. 5.25
2 COMBINED AT							
+	C-12	0.80	1	FLOW TIME	83. 4.58	76. 4.67	6. 5.25
ROUTED TO							
+	R13	0.80	1	FLOW TIME	81. 4.83	76. 4.92	6. 6.17
HYDROGRAPH AT							
+	BASIN-5	0.54	1	FLOW TIME	93. 4.08	87. 4.08	10. 5.17
2 COMBINED AT							
+	C-13	1.34	1	FLOW TIME	139. 4.92	131. 5.00	10. 5.17
HYDROGRAPH AT							
+	BASIN-32	0.04	1	FLOW TIME	6. 4.08	5. 4.08	0. 5.17
ROUTED TO							
+	R16	0.04	1	FLOW TIME	5. 4.92	5. 4.92	0. 7.58
HYDROGRAPH AT							
+	BASIN-23	0.15	1	FLOW TIME	19. 4.25	17. 4.25	2. 5.25
2 COMBINED AT							
+	C-16	0.19	1	FLOW TIME	19. 5.00	18. 5.00	2. 5.25
ROUTED TO							
+	R17	0.19	1	FLOW TIME	19. 5.42	18. 5.50	2. 6.42
HYDROGRAPH AT							
+	BASIN-7	0.14	1	FLOW TIME	18. 4.17	17. 4.17	2. 5.25
2 COMBINED AT							
+	C-17	0.32	1	FLOW TIME	32. 4.58	29. 4.67	2. 6.42
HYDROGRAPH AT							
+	BASIN-10	0.16	1	FLOW TIME	17. 4.50	16. 4.50	2. 5.42

HYDROGRAPH AT + BASIN-8 0.05 1 FLOW TIME .7. 4.17 .7. 4.17 1. 5.25  
 HYDROGRAPH AT + BASIN-9 0.05 1 FLOW TIME .7. 4.08 .6. 4.08 1. 5.17  
 7 COMBINED AT + C-26 74.98 1 FLOW TIME 1442. 4.33 1370. 4.33 329. 4.58  
 HYDROGRAPH AT + BASIN-34 0.19 1 FLOW TIME 42. 4.17 39. 4.25 6. 5.25  
 HYDROGRAPH AT + SPLIT1 0.00 1 FLOW TIME 540. 3.92 523. 3.92 204. 4.00  
 HYDROGRAPH AT + BASIN-35 0.11 1 FLOW TIME 44. 4.17 42. 4.17 11. 4.25  
 3 COMBINED AT + C-1B 0.30 1 FLOW TIME 609. 3.92 588. 3.92 217. 4.00  
 ROUTED TO + R5 0.30 1 FLOW TIME 605. 4.08 584. 4.08 215. 4.25  
 HYDROGRAPH AT + BASIN-12 0.33 1 FLOW TIME 45. 4.25 42. 4.25 5. 5.25  
 2 COMBINED AT + C-5 0.63 1 FLOW TIME 646. 4.08 623. 4.08 217. 4.25  
 HYDROGRAPH AT + BASIN-14 0.27 1 FLOW TIME 197. 4.00 192. 4.00 79. 4.08  
 HYDROGRAPH AT + BASIN-16 0.26 1 FLOW TIME 193. 4.00 187. 4.00 78. 4.00  
 HYDROGRAPH AT + BASIN-31 0.15 1 FLOW TIME 159. 3.75 155. 3.75 65. 3.75  
 1

SUMMARY OF KINEMATIC WAVE - MUSKINGUM-CUNJE ROUTING  
(FLOW IS DIRECT RUNOFF WITHOUT BASE FLOW)

ISTAQ	ELEMENT	DT	PEAK	TIME TO PEAK	VOLUME	INTERPOLATED TO COMPUTATION INTERVAL			VOLUME		
						DT	PEAK	TIME TO PEAK			
				(MIN)	(CFS)	(MIN)	(IN)	(MIN)	(CPS)	(MIN)	(IN)
FOR PLAN = 1 RATIO= 1.00											
RO MANE 4.90 581.70 235.69 0.12 5.00 578.13 235.00 0.12											

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4484E+03 EXCESS=0.0000E+00 OUTFLOW=0.4394E+03 BASIN STORAGE=0.9989E+01 PERCENT ERROR= -0.2

FOR PLAN = 1 RATIO= 0.98											
RO MANE 4.87 563.83 237.49 0.12 5.00 560.09 235.00 0.12											

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4429E+03 EXCESS=0.0000E+00 OUTFLOW=0.4340E+03 BASIN STORAGE=0.9930E+01 PERCENT ERROR= -0.2

FOR PLAN = 1 RATIO= 0.57											
RO MANE 5.00 252.10 241.64 0.08 5.00 250.21 320.00 0.08											

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3074E+03 EXCESS=0.0000E+00 OUTFLOW=0.2991E+03 BASIN STORAGE=0.8802E+01 PERCENT ERROR= -0.2

FOR PLAN = 1 RATIO= 1.00											
R2 MANE 2.97 738.00 239.35 0.08 5.00 737.86 240.00 0.07											

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2829E+03 EXCESS=0.0000E+00 OUTFLOW=0.2804E+03 BASIN STORAGE=0.3116E+01 PERCENT ERROR= -0.2

FOR PLAN = 1 RATIO= 0.98											
R2 MANE 3.09 718.63 240.85 0.07 5.00 714.81 240.00 0.07											

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2784E+03 EXCESS=0.0000E+00 OUTFLOW=0.2754E+03 BASIN STORAGE=0.3098E+01 PERCENT ERROR= -0.1

FOR PLAN = 1 RATIO= 0.57											
R2 MANE 4.50 268.73 249.98 0.05 5.00 268.71 250.00 0.05											

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1775E+03 EXCESS=0.0000E+00 OUTFLOW=0.1747E+03 BASIN STORAGE=0.2556E+01 PERCENT ERROR= 0.1

FOR PLAN = 1 RATIO= 1.00											
R3 MANE 3.21 755.42 245.65 0.07 5.00 750.95 245.00 0.07											

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2828E+03 EXCESS=0.0000E+00 OUTFLOW=0.2793E+03 BASIN STORAGE=0.3271E+01 PERCENT ERROR= 0.1

FOR PLAN = 1 RATIO= 0.98  
R3 MANE 3.13 733.99 247.08 0.07 5.00 725.43 250.00 0.07

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2782E+03 EXCESS=0.0000E+00 OUTFLOW=0.2751E+03 BASIN STORAGE=0.3251E+01 PERCENT ERROR= -0.1

FOR PLAN = 1 RATIO= 0.57  
R3 MANE 4.80 267.37 258.43 0.05 5.00 266.22 260.00 0.05

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1751E+03 EXCESS=0.0000E+00 OUTFLOW=0.1727E+03 BASIN STORAGE=0.2684E+01 PERCENT ERROR= -0.2

FOR PLAN = 1 RATIO= 1.00  
R4 MANE 3.29 799.55 252.30 0.08 5.00 797.84 255.00 0.08

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2859E+03 EXCESS=0.0000E+00 OUTFLOW=0.2825E+03 BASIN STORAGE=0.3397E+01 PERCENT ERROR= 0.0

FOR PLAN = 1 RATIO= 0.98  
R4 MANE 3.31 770.83 256.28 0.07 5.00 770.43 255.00 0.07

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2810E+03 EXCESS=0.0000E+00 OUTFLOW=0.2775E+03 BASIN STORAGE=0.3376E+01 PERCENT ERROR= 0.0

FOR PLAN = 1 RATIO= 0.57  
R4 MANE 4.87 268.34 268.85 0.05 5.00 267.23 270.00 0.05

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1731E+03 EXCESS=0.0000E+00 OUTFLOW=0.1708E+03 BASIN STORAGE=0.2789E+01 PERCENT ERROR= -0.3

FOR PLAN = 1 RATIO= 1.00  
R7 MANE 5.00 9.01 292.34 0.40 5.00 8.95 295.00 0.40

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1178E+01 EXCESS=0.0000E+00 OUTFLOW=0.1382E+01 BASIN STORAGE=0.5064E-02 PERCENT ERROR= -17.8

FOR PLAN = 1 RATIO= 0.98  
R7 MANE 5.00 8.36 294.76 0.38 5.00 8.36 295.00 0.38

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1106E+01 EXCESS=0.0000E+00 OUTFLOW=0.1300E+01 BASIN STORAGE=0.5093E-02 PERCENT ERROR= -18.0

FOR PLAN = 1 RATIO= 0.57  
R7 MANE 5.00 0.80 438.67 0.04 5.00 0.80 440.00 0.04

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8999E-01 EXCESS=0.0000E+00 OUTFLOW=0.1464E+00 BASIN STORAGE=0.4877E-02 PERCENT ERROR= -68.1

FOR PLAN = 1 RATIO= 1.00  
R8 MANE 5.00 21.50 313.77 0.36 5.00 21.48 315.00 0.36

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3875E+01 EXCESS=0.0000E+00 OUTFLOW=0.3911E+01 BASIN STORAGE=0.1027E-01 PERCENT ERROR= -1.2

FOR PLAN = 1 RATIO= 0.98  
R8 MANE 5.00 20.28 316.42 0.34 5.00 20.24 320.00 0.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3642E+01 EXCESS=0.0000E+00 OUTFLOW=0.3692E+01 BASIN STORAGE=0.1093E-01 PERCENT ERROR= -1.7

FOR PLAN = 1 RATIO= 0.57  
R8 MANE 4.97 1.57 389.03 0.03 5.00 1.56 385.00 0.03

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3369E+00 EXCESS=0.0000E+00 OUTFLOW=0.3596E+00 BASIN STORAGE=0.1170E-01 PERCENT ERROR= -10.2

FOR PLAN = 1 RATIO= 1.00  
R9 MANE 5.00 15.74 303.59 0.34 5.00 15.69 305.00 0.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2657E+01 EXCESS=0.0000E+00 OUTFLOW=0.2680E+01 BASIN STORAGE=0.5574E-02 PERCENT ERROR= -1.1

FOR PLAN = 1 RATIO= 0.98  
R9 MANE 5.00 14.66 306.15 0.32 5.00 14.65 305.00 0.32

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2495E+01 EXCESS=0.0000E+00 OUTFLOW=0.2516E+01 BASIN STORAGE=0.5477E-02 PERCENT ERROR= -1.1

FOR PLAN = 1 RATIO= 0.57  
R9 MANE 5.00 1.49 425.03 0.03 5.00 1.49 425.00 0.03

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2030E+00 EXCESS=0.0000E+00 OUTFLOW=0.2603E+00 BASIN STORAGE=0.5422E-02 PERCENT ERROR= -30.9

FOR PLAN = 1 RATIO= 1.00  
R10 MANE 3.50 32.78 311.37 0.34 5.00 32.76 310.00 0.34

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6021E+01 EXCESS=0.0000E+00 OUTFLOW=0.6016E+01 BASIN STORAGE=0.3565E-02 PERCENT ERROR= 0.0

FOR PLAN = 1 RATIO= 0.98  
 R10 MANE 3.65 30.78 314.39 0.32 5.00 30.75 315.00 0.32  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5650E+01 EXCESS=0.0000E+00 OUTFLOW=0.5651E+01 BASIN STORAGE=0.3749E-02 PERCENT ERROR= -0.1

FOR PLAN = 1 RATIO= 0.57  
 R10 MANE 5.00 2.04 347.20 0.03 5.00 2.03 345.00 0.03  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5156E+00 EXCESS=0.0000E+00 OUTFLOW=0.5169E+00 BASIN STORAGE=0.4346E-02 PERCENT ERROR= -1.1

FOR PLAN = 1 RATIO= 1.00  
 R18 MANE 5.00 71.43 324.31 0.35 5.00 71.31 325.00 0.35  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1352E+02 EXCESS=0.0000E+00 OUTFLOW=0.1351E+02 BASIN STORAGE=0.1980E-01 PERCENT ERROR= -0.1

FOR PLAN = 1 RATIO= 0.98  
 R18 MANE 5.00 67.34 327.38 0.33 5.00 66.78 325.00 0.33  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1271E+02 EXCESS=0.0000E+00 OUTFLOW=0.1272E+02 BASIN STORAGE=0.1817E-01 PERCENT ERROR= -0.3

FOR PLAN = 1 RATIO= 0.57  
 R18 MANE 4.95 3.51 384.91 0.03 5.00 3.51 385.00 0.03  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1150E+01 EXCESS=0.0000E+00 OUTFLOW=0.1125E+01 BASIN STORAGE=0.2628E-01 PERCENT ERROR= -0.1

FOR PLAN = 1 RATIO= 1.00  
 R22 MANE 5.00 87.04 247.39 1.28 5.00 85.36 250.00 1.27  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8283E+01 EXCESS=0.0000E+00 OUTFLOW=0.8362E+01 BASIN STORAGE=0.5664E-02 PERCENT ERROR= -1.0

FOR PLAN = 1 RATIO= 0.98  
 R22 MANE 5.00 83.71 249.17 1.23 5.00 82.95 245.00 1.23  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.8000E+01 EXCESS=0.0000E+00 OUTFLOW=0.8067E+01 BASIN STORAGE=0.5337E-02 PERCENT ERROR= -0.9

FOR PLAN = 1 RATIO= 0.57  
 R22 MANE 4.70 28.89 262.53 0.46 5.00 28.65 260.00 0.46  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2785E+01 EXCESS=0.0000E+00 OUTFLOW=0.3039E+01 BASIN STORAGE=0.5811E-02 PERCENT ERROR= -9.3

FOR PLAN = 1 RATIO= 1.00  
 R23 MANE 4.47 160.29 252.69 1.27 5.00 157.41 250.00 1.27  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1798E+02 EXCESS=0.0000E+00 OUTFLOW=0.1798E+02 BASIN STORAGE=0.1135E-01 PERCENT ERROR= -0.1

FOR PLAN = 1 RATIO= 0.98  
 R23 MANE 4.54 155.13 254.52 1.23 5.00 153.98 255.00 1.23  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1737E+02 EXCESS=0.0000E+00 OUTFLOW=0.1740E+02 BASIN STORAGE=0.1054E-01 PERCENT ERROR= -0.2

FOR PLAN = 1 RATIO= 0.57  
 R23 MANE 5.00 43.35 278.76 0.46 5.00 42.66 280.00 0.46  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6273E+01 EXCESS=0.0000E+00 OUTFLOW=0.6462E+01 BASIN STORAGE=0.1146E-01 PERCENT ERROR= -3.2

FOR PLAN = 1 RATIO= 1.00  
 R28 MANE 5.00 188.26 268.39 1.19 5.00 185.36 265.00 1.19  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2253E+02 EXCESS=0.0000E+00 OUTFLOW=0.2252E+02 BASIN STORAGE=0.4503E-01 PERCENT ERROR= -0.2

FOR PLAN = 1 RATIO= 0.98  
 R28 MANE 5.00 181.86 270.33 1.15 5.00 181.63 270.00 1.15  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2179E+02 EXCESS=0.0000E+00 OUTFLOW=0.2178E+02 BASIN STORAGE=0.4023E-01 PERCENT ERROR= -0.1

FOR PLAN = 1 RATIO= 0.57  
 R28 MANE 5.00 47.44 312.47 0.43 5.00 46.29 315.00 0.43  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7736E+01 EXCESS=0.0000E+00 OUTFLOW=0.8148E+01 BASIN STORAGE=0.4995E-01 PERCENT ERROR= -6.0

FOR PLAN = 1 RATIO= 1.00  
 R33 MANE 1.07 1158.92 257.24 0.09 5.00 1158.55 260.00 0.09  
 CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3495E+03 EXCESS=0.0000E+00 OUTFLOW=0.3485E+03 BASIN STORAGE=0.1328E+01 PERCENT ERROR= -0.1

FOR PLAN = 1 RATIO= 0.98

R33	MANE	1.15	1108.80	261.21	0.09	5.00	1107.75	260.00	0.09
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3417E+03 EXCESS=0.0000E+00 OUTFLOW=0.3406E+03 BASIN STORAGE=0.1320E+01 PERCENT ERROR= -0.1									
FOR PLAN = 1 RATIO= 0.57 R33 MANE 1.88 306.13 273.31 0.05 5.00 304.13 275.00 0.05									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1879E+03 EXCESS=0.0000E+00 OUTFLOW=0.1868E+03 BASIN STORAGE=0.1091E+01 PERCENT ERROR= 0.0									
FOR PLAN = 1 RATIO= 1.00 R14 MANE 5.00 5.82 303.46 0.42 5.00 5.80 305.00 0.42									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7670E+00 EXCESS=0.0000E+00 OUTFLOW=0.9516E+00 BASIN STORAGE=0.5482E-02 PERCENT ERROR= -24.8									
FOR PLAN = 1 RATIO= 0.98 R14 MANE 4.93 5.41 304.61 0.40 5.00 5.40 305.00 0.40									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.7200E+00 EXCESS=0.0000E+00 OUTFLOW=0.9072E+00 BASIN STORAGE=0.5442E-02 PERCENT ERROR= -26.7									
FOR PLAN = 1 RATIO= 0.57 R14 MANE 5.00 0.52 468.25 0.05 5.00 0.52 470.00 0.05									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5861E-01 EXCESS=0.0000E+00 OUTFLOW=0.1054E+00 BASIN STORAGE=0.4953E-02 PERCENT ERROR= -88.2									
FOR PLAN = 1 RATIO= 1.00 R15 MANE 5.00 13.33 344.84 0.40 5.00 13.32 345.00 0.40									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2427E+01 EXCESS=0.0000E+00 OUTFLOW=0.2633E+01 BASIN STORAGE=0.1885E-01 PERCENT ERROR= -9.2									
FOR PLAN = 1 RATIO= 0.98 R15 MANE 5.00 12.55 347.54 0.38 5.00 12.51 350.00 0.38									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2292E+01 EXCESS=0.0000E+00 OUTFLOW=0.2492E+01 BASIN STORAGE=0.1887E-01 PERCENT ERROR= -9.5									
FOR PLAN = 1 RATIO= 0.57 R15 MANE 5.00 0.90 444.62 0.04 5.00 0.90 445.00 0.04									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2181E+00 EXCESS=0.0000E+00 OUTFLOW=0.2639E+00 BASIN STORAGE=0.2020E-01 PERCENT ERROR= -30.3									
FOR PLAN = 1 RATIO= 1.00 R19 MANE 4.74 22.80 329.14 0.37 5.00 22.76 330.00 0.37									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5182E+01 EXCESS=0.0000E+00 OUTFLOW=0.5270E+01 BASIN STORAGE=0.2544E-01 PERCENT ERROR= -2.2									
FOR PLAN = 1 RATIO= 0.98 R19 MANE 4.91 21.37 327.75 0.35 5.00 21.37 330.00 0.35									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4880E+01 EXCESS=0.0000E+00 OUTFLOW=0.4961E+01 BASIN STORAGE=0.2515E-01 PERCENT ERROR= -2.2									
FOR PLAN = 1 RATIO= 0.57 R19 MANE 5.00 1.66 390.67 0.03 5.00 1.65 395.00 0.03									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4581E+00 EXCESS=0.0000E+00 OUTFLOW=0.4772E+00 BASIN STORAGE=0.2995E-01 PERCENT ERROR= -10.7									
FOR PLAN = 1 RATIO= 1.00 R11 MANE 5.00 10.06 322.70 0.41 5.00 10.03 320.00 0.41									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1590E+01 EXCESS=0.0000E+00 OUTFLOW=0.1908E+01 BASIN STORAGE=0.1120E-01 PERCENT ERROR= -20.7									
FOR PLAN = 1 RATIO= 0.98 R11 MANE 5.00 9.34 325.75 0.38 5.00 9.33 325.00 0.38									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1493E+01 EXCESS=0.0000E+00 OUTFLOW=0.1796E+01 BASIN STORAGE=0.1102E-01 PERCENT ERROR= -21.0									
FOR PLAN = 1 RATIO= 0.57 R11 MANE 5.00 0.94 488.38 0.04 5.00 0.94 485.00 0.04									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1215E+00 EXCESS=0.0000E+00 OUTFLOW=0.2049E+00 BASIN STORAGE=0.1054E-01 PERCENT ERROR= -77.3									
FOR PLAN = 1 RATIO= 1.00 R12 MANE 5.00 38.88 277.89 0.34 5.00 38.82 280.00 0.34									
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6004E+01 EXCESS=0.0000E+00 OUTFLOW=0.6044E+01 BASIN STORAGE=0.5619E-02 PERCENT ERROR= -0.8									
FOR PLAN = 1 RATIO= 0.98 R12 MANE 5.00 36.23 279.69 0.32 5.00 36.21 280.00 0.32									

CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5636E+01 EXCESS=0.0000E+00 OUTFLOW=0.5662E+01 BASIN STORAGE=0.4580E-02 PERCENT ERROR= -0.5

FOR PLAN = 1 RATIO= 0.57									
R12	MANE	5.00	3.65	382.10	0.03	5.00	3.63	385.00	0.03
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.4587E+00 EXCESS=0.0000E+00 OUTFLOW=0.4777E+00 BASIN STORAGE=0.4449E-02 PERCENT ERROR= -5.1									
FOR PLAN = 1 RATIO= 1.00									
R13	MANE	5.00	81.60	294.04	0.34	5.00	81.47	290.00	0.34
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1452E+02 EXCESS=0.0000E+00 OUTFLOW=0.1459E+02 BASIN STORAGE=0.1499E-01 PERCENT ERROR= -0.6									
FOR PLAN = 1 RATIO= 0.98									
R13	MANE	5.00	75.94	296.59	0.32	5.00	75.89	295.00	0.32
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1363E+02 EXCESS=0.0000E+00 OUTFLOW=0.1368E+02 BASIN STORAGE=0.1679E-01 PERCENT ERROR= -0.5									
FOR PLAN = 1 RATIO= 0.57									
R13	MANE	5.00	5.66	368.44	0.03	5.00	5.64	370.00	0.03
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1125E+01 EXCESS=0.0000E+00 OUTFLOW=0.1165E+01 BASIN STORAGE=0.1799E-01 PERCENT ERROR= -5.2									
FOR PLAN = 1 RATIO= 1.00									
R16	MANE	5.00	5.51	296.47	0.41	5.00	5.49	295.00	0.41
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6871E+00 EXCESS=0.0000E+00 OUTFLOW=0.8355E+00 BASIN STORAGE=0.4265E-02 PERCENT ERROR= -22.2									
FOR PLAN = 1 RATIO= 0.98									
R16	MANE	5.00	5.10	299.12	0.39	5.00	5.07	295.00	0.39
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.6450E+00 EXCESS=0.0000E+00 OUTFLOW=0.7861E+00 BASIN STORAGE=0.3883E-02 PERCENT ERROR= -22.5									
FOR PLAN = 1 RATIO= 0.57									
R16	MANE	5.00	0.48	452.08	0.05	5.00	0.48	455.00	0.05
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.5249E-01 EXCESS=0.0000E+00 OUTFLOW=0.9222E-01 BASIN STORAGE=0.3684E-02 PERCENT ERROR= -82.7									
FOR PLAN = 1 RATIO= 1.00									
R17	MANE	4.76	19.03	323.72	0.36	5.00	18.99	325.00	0.36
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3511E+01 EXCESS=0.0000E+00 OUTFLOW=0.3512E+01 BASIN STORAGE=0.1006E-01 PERCENT ERROR= -0.3									
FOR PLAN = 1 RATIO= 0.98									
R17	MANE	4.83	17.98	326.25	0.33	5.00	17.87	330.00	0.33
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.3298E+01 EXCESS=0.0000E+00 OUTFLOW=0.3297E+01 BASIN STORAGE=0.9974E-02 PERCENT ERROR= -0.3									
FOR PLAN = 1 RATIO= 0.57									
R17	MANE	5.00	1.72	384.33	0.03	5.00	1.72	385.00	0.03
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2965E+00 EXCESS=0.0000E+00 OUTFLOW=0.3131E+00 BASIN STORAGE=0.1113E-01 PERCENT ERROR= -9.3									
FOR PLAN = 1 RATIO= 1.00									
R5	MANE	3.68	606.85	243.65	14.63	5.00	604.93	245.00	14.58
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2386E+03 EXCESS=0.0000E+00 OUTFLOW=0.2359E+03 BASIN STORAGE=0.3199E+01 PERCENT ERROR= -0.2									
FOR PLAN = 1 RATIO= 0.98									
R5	MANE	3.90	586.30	243.87	14.38	5.00	584.46	245.00	14.34
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.2347E+03 EXCESS=0.0000E+00 OUTFLOW=0.2318E+03 BASIN STORAGE=0.3180E+01 PERCENT ERROR= -0.1									
FOR PLAN = 1 RATIO= 0.57									
R5	MANE	5.00	215.98	254.43	9.03	5.00	215.06	255.00	9.00
CONTINUITY SUMMARY (AC-FT) - INFLOW=0.1480E+03 EXCESS=0.0000E+00 OUTFLOW=0.1456E+03 BASIN STORAGE=0.2623E+01 PERCENT ERROR= -0.2									

\*\*\* NORMAL END OF HEC-1 \*\*\*

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CLOMR Cross Section 34

CLOMR - CROSS-SECTION 34  
Worksheet for Irregular Channel

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Project Description

Project File e:\cb879\xs.fm2  
Worksheet CROSS-SECTION 34  
Flow Element Irregular Channel  
Method Manning's Formula  
Solve For Water Elevation

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Input Data

Channel Slope 1.5000 %  
Elevation range: 2,305.00 ft to 2,310.00 ft.

Station (ft)	Elevation (ft)	Start Station	End Station	Roughness
0.00	2,310.00	0.00	612.00	0.030
198.00	2,305.00			
231.00	2,305.00			
612.00	2,310.00			
Discharge	833.00 cfs			

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Results

Wtd. Mannings Coefficient	0.030
Water Surface Elevation	2,306.39 ft
Flow Area	157.65 ft <sup>2</sup>
Wetted Perimeter	193.94 ft
Top Width	193.91 ft
Height	1.39 ft
Critical Depth	2,306.41 ft
Critical Slope	0.013992 ft/ft
Velocity	5.28 ft/s
Velocity Head	0.43 ft
Specific Energy	2,306.82 ft
Froude Number	1.03

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Flow is supercritical.

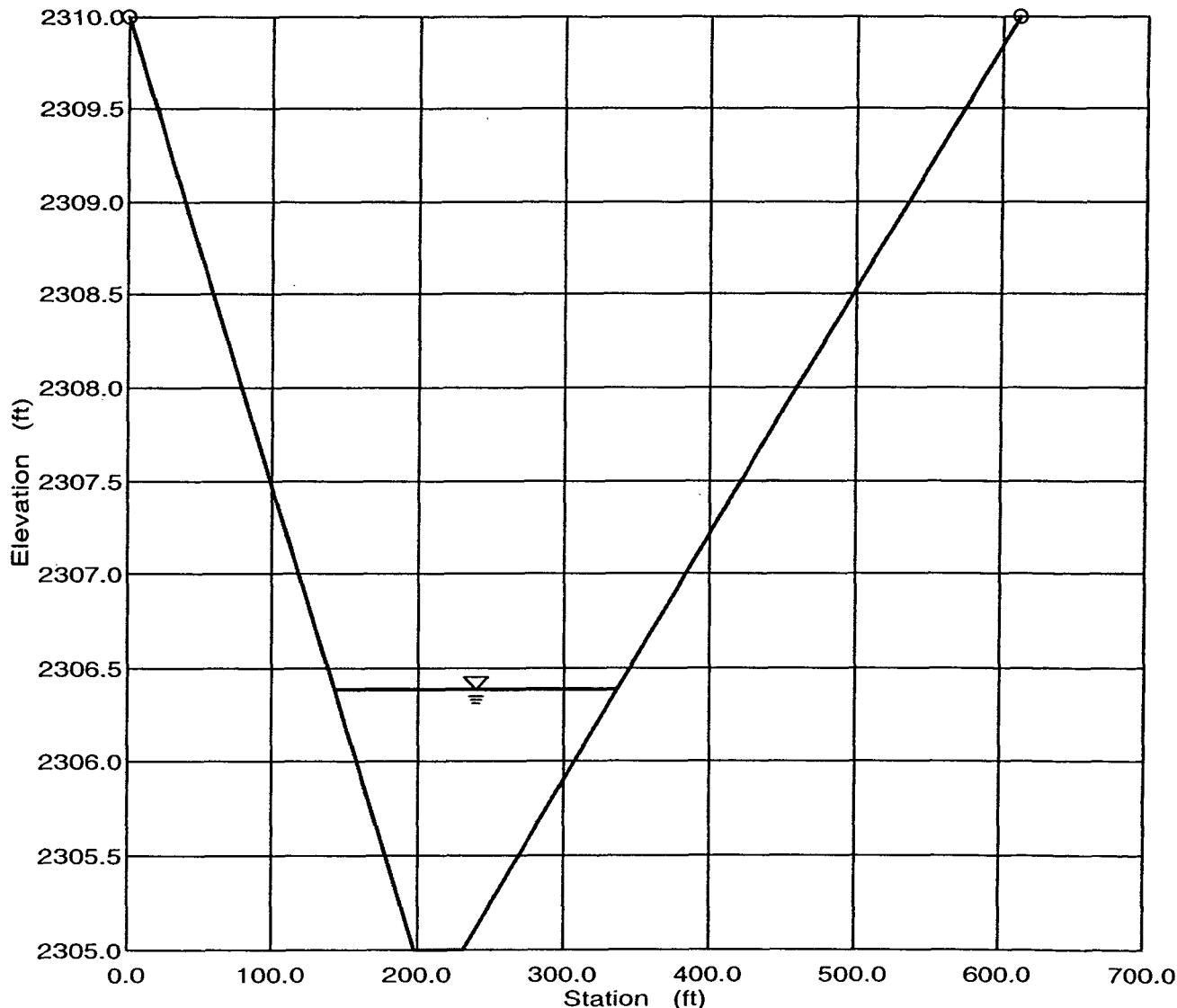
CLOMR - CROSS-SECTION 34  
Cross Section for Irregular Channel

Project Description

Project File e:\cb879\xs.fm2  
Worksheet CROSS-SECTION 34  
Flow Element Irregular Channel  
Method Manning's Formula  
Solve For Water Elevation

Section Data

Wtd. Mannings Coefficient 0.030  
Channel Slope 1.5000 %  
Water Surface Elevation 2,306.39 ft  
Discharge 833.00 cfs



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**CLOMR Figure 4 – Basin and Cross Section Map**



**FIGURE 4 BASIN AND CROSS SECTION MAP**

**PBSI**

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## **APPENDIX E**

Data CD

